



Bull Run TREATMENT

Filtration

Subject:	Stormwater Drainage Report
PWB P roject #s:	W02229
Date:	January 15, 2024
To:	David Peters, Program Director Portland Water Bureau
From:	Rafael Gaeta, PE Principal Engineer Emerio Design
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Emerio Project Number: 0596-003

City of Portland Permit Numbers: TBD

I hereby certify that this Stormwater Management Report for this project has been prepared by me or under my supervision and meets minimum standards of the Multnomah County Design and Construction Manual (MCDCM) and normal standards of engineering practice. I hereby acknowledge and agree that the jurisdiction does not and will not assume liability for the sufficiency, suitability, or performance of drainage facilities designed by me.

This report was prepared in support of the City of Portland Water Bureau's Bull Run Filtration Facility Site Project land use applications in Multnomah County and reflects the current status of the project design, which is approximately 100% complete as of the date of this report. This design is subject to change and has been prepared for the specific purpose of addressing conformance of the project to the Multnomah County land use requirements as expressed in the Multnomah County Code.

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List of Abbreviations

BES	Bureau of Environmental Services
CAD	Computer-Aided Design
CN	Curve Number
COP	City of Portland
CSSWF	Columbia South Shore Wellfield
EPA	Environmental Protection Agency
GIS	Geographical Information System
MCDCM	Multnomah County Design and Construction Manual
NCRS	Natural Resources Conservation Service
NOAA	National Oceanic and Atmospheric Administration
0&M	Operations and Maintenance
OHA	Oregon Health Authority
PAC	Presumptive Approach Calculator
PDF	Portable document format
POD	Point of Discharge
PWB	Portland Water Bureau
RWP	Raw Water Pipe
SBUH	Santa Barbara Urban Hydrograph
SDFDM	City of Portland 2020 Sewer and Drainage Facilities Design Manual
SWMM	City of Portland 2020 Stormwater Management Manual
TR	Technical Report

References

Multnomah County, Design and Construction Manual

City of Portland, 2020 Stormwater Management Manual

City of Portland, 2020 Sewer and Drainage Facilities Design Manual

City of Portland, 2020 Source Control Manual

Oregon Department of Environmental Quality, Section 401 Water Quality Certification Post-Construction Stormwater Management Plan Submission Guidelines, January 2021.

Oregon 2018/2020 Integrated Report

2018/2020 Water Quality Report and List of Water Quality Limited Waters, dated April 2020 prepared by the Oregon Department of Environmental Quality.

Urban Hydrology for Small Watersheds TR-55, dated June 1986.

1.0 Introduction

1.1 Project Description

The water supply for the City of Portland includes two sources: the Bull Run Watershed and the Columbia South Shore Wellfield (CSSWF). In addition to the City of Portland, the Portland Water Bureau (PWB) provides potable water to 19 wholesale customers. The Bull Run Watershed, located east of Portland in the Mount Hood National Forest, is the primary source of water. This 102-square-mile protected watershed is managed by the U.S. Forest Service in cooperation with the PWB. Two dam structures within this watershed create two surface water reservoirs with a combined storage capacity of 16.5 billion gallons. This water is transported from the lower dam near the headworks site to the Portland Metro area via three large-diameter pipelines.

The Bull Run supply is currently an unfiltered water supply and has consistently met the filtration avoidance criteria under the Surface Water Treatment Rule for source water quality, watershed management, and disinfection. Before distribution, the supply is treated with free chlorine for primary disinfection, ammonia to form chloramines as a residual disinfectant, and sodium hydroxide for corrosion control. The PWB supplements the Bull Run source as needed with groundwater withdrawn from the CSSWF. The CSSWF is primarily used as an emergency backup. It is typically used during turbidity events in the Bull Run Watershed and for summer supply augmentation.

In 2006, the U.S. Environmental Protection Agency (EPA) required water utilities to treat for the pathogen Cryptosporidium. From 2012 to 2017, the Oregon Health Authority (OHA) issued a variance from this rule subject to a set of conditions that included exhibiting an exceptionally low concentration of Cryptosporidium in ongoing raw water sampling. However, on May 19, 2017, OHA issued an order revoking Portland's treatment variance. In August 2017, the Portland City Council voted to build the Bull Run Filtration Facility to meet the treatment requirements for Cryptosporidium. On December 18, 2017, OHA and the PWB signed a bilateral compliance agreement establishing a compliance schedule for meeting the EPA requirements.

The Bull Run Water Treatment Project was established to meet the compliance schedule and is a multi-year drinking water infrastructure program consisting of a new drinking water filtration facility and associated new raw and finished water pipelines. This report is developed for the Filtration Facility. The facility will have a capacity of 135 mgd. The facility location is a 95-acre site owned by the PWB, east of Gresham, Oregon.

1.2 Purpose of this Report

The purpose of this report is to evaluate the existing and proposed stormwater conditions for the Bull Run Filtration Facility including Carpenter Lane and the southern emergency access road. This report includes an analysis and discussion on the following:

- City of Portland's Infiltration and Discharge Hierarchy
- Site Specific Infiltration Testing and Rates
- Stormwater Quality Treatment
- Stormwater Flow Control
- Stormwater Conveyance
- Analysis of surface stormwater for both Run-On and Run-Off conditions
- Groundwater management including groundwater collected from sub-slab drainage pipes
- Stormwater management of Carpenter Lane
- Stormwater management of the emergency access road

1.3 Project Location

The proposed project is located south of SE Carpenter Lane, Multnomah County, Oregon. Currently, the existing site is a nursery crop production facility.

1.4 Property Zoning

This project area is zoned for Exclusive Farm and Forest Use.

1.5 Agency Stormwater Criteria

This project lies within two jurisdictions: Multnomah County and Clackamas County.

Multnomah County is the governing agency for the Bull Run Filtration Facility and Carpenter Lane. Both stormwater flow control and stormwater quality treatment are required and shall be designed in accordance with the Multnomah County Design and Construction Manual (MCDCM), Section 5. Drainage. The MCDCM follows the City of Portland 2020 Stormwater Management Manual (SWMM) for stormwater quality treatment (per MCDCM Section 5.1.3. Water Quality Design Standards). Stormwater flow control is designed per MCDCM Section 5.1.2. Water Quantity Design Standards which match the SWMM Section 2.5.3.2. Designing for Flow Control Requirements.

The emergency access road that runs from the southeast corner of the Facility site to SE Bluff Road is located within Clackamas County. Both stormwater treatment and stormwater quantity are required and are designed in accordance with the Stormwater Standards Clackamas County Service District No.1, July 1, 2013.

1.6 Site Specific Stormwater Design Requirements and Assumptions

Stormwater management requirements include infiltration, water quality treatment, and flow control. The following section includes design assumptions for the Bull Run Filtration Facility Improvements. Onsite stormwater management is required to the maximum extent possible depending on site-specific conditions. The following is a discussion of the site-specific parameters and design assumptions.

Hydromodification is the alteration of the natural flow of water, timing, frequency, and volume of runoff from the land surface as a result of urbanization. Hydromodification results in an increase in runoff energy and frequency in the receiving water bodies. This increase in energy leads to the degradation of the natural stream and wetland systems through incision, bank erosion, and sedimentation. The flow control requirements in this stormwater management report are designed to minimize these impacts in accordance with Section 1.3.5. Level 2 Separated Storm System Requirements of the City of Portland SWMM.

1.6.1 Infiltration and Discharge Hierarchy

The City of Portland has three Levels of hierarchy for the disposal and conveyance of stormwater. The city ranks the use of these systems for stormwater management in Section 1.3.3. Infiltration and Discharge Hierarchy and Table 1-2. Summary of Infiltration and Discharge Hierarchy Stormwater Management Requirements of the SWMM, as follows:

Level 1 - Full Onsite infiltration

Level 2 – Offsite Discharge to the Separate Stormwater System

Level 3 - Combined sewers that convey water to the wastewater treatment plant

Infiltration Rates

The Final Geotechnical Data Report Technical Memorandum Dated July 15, 2022 (prepared by McMillan Jacobs Associates), Attachment K Preliminary Geotechnical Data Report (prepared by Rhino One) discusses the field infiltration testing related to stormwater design. Per section 2.4 of the Preliminary Data Report prepared by Rhino One the infiltration rates are low, and infiltration is not feasible at this site. Table 1. Geotechnical Measured Infiltration Rates show extremely low infiltration rates. The measured infiltration rates range from 0.012 in/hr to 0.86 in/hr as noted in the boring logs and Table 4 of the Geotechnical Data Report prepared by Rhino One.

Table 1. Geotechnical Measured Infiltration Rates					
Boring Number	Infiltration Rate (in/hr)	Nearest Stormwater Pond			
WTP-B-06	0.3	D			
WTP-B-10	0.11	D			
WTP-B-11	0.31	D			
WTP-B-12	0.012	D			
WTP-TP-02	0.86	A			
WTP-TP-04	0.29	A			
WTP-TP-06	0.18	С			
WTP-TP-08	0.36	С			
WTP-TP-09	0.41	С			
WTP-TP-10	0.13	n/a			
WTP-TP-11	0.32	n/a			

Discharge Hierarchy

As the onsite infiltration measured rate is extremely low, it has been deemed not practical for this project.

This project is using Hierarchy Level 2 - Offsite Discharges to the Separated Stormwater System, Johnson Creek. The project is also assuming that there is no infiltration within the stormwater facilities due to the extremely low infiltration rates shown in the geotechnical data and engineering reports. The geotechnical evaluation did not recommend the use of infiltration-based facilities and is deemed not practical for this project as a stormwater management strategy.

1.6.2 Stormwater Facility Selection, Sizing, and Design

The City of Portland allows the use of three design approaches for stormwater facilities being:

- Simplified Approach
- Presumptive Approach
- Performance Approach

This project will use both the Presumptive Approach and the Performance Approach. The Presumptive Approach is used for sizing the stormwater planters and basins, while the Performance Approach is used for sizing the flow control ponds and grassy swales.

1.6.3 Stormwater Quality Treatment

Per Table 1-2. Summary of Infiltration and Discharge Hierarchy Stormwater Management Requirements of the SWMM this project is Hierarchy Level 2 in the discharge hierarchy as discussed above in Section 1.6.1. Therefore, the following stormwater quality treatment standards apply.

Pollution reduction required:

- Achieve 70% TSS removal from the runoff resulting from 90% of the average annual rainfall.
- Treat 80% of the average annual rainfall.
- Use a pollution reduction facility that will reduce pollutants of concern in watersheds with a TMDL or on DEQ's 303(d) list of impaired waters.

Per the Oregon 2018/2020 Integrated Report and the 2018/2020 Water Quality Report and List of Water Quality Limited Waters, dated April 2020, Johnson Creek has TMDL and 303(d) listed.

TMDLs:

- Bacteria
- Temperature
- DDT 4, 4'; DDD4, 4'; DDE 4,4'

303(d):

- Dissolved Oxygen
- Iron
- Endosulfan
- Chlordane
- Polychlorinated Biphenyls (PCBs)
- Polycyclic Aromatic Hydrocarbons (PAHs)

From the City of Portland SWMM Table 2-8. Summary of How Requirements are Met, both the Presumptive and Performance Approaches are allowed to be used with Hierarchy Level 2.

This project is proposing to use multiple facilities to achieve the water quality treatment requirement by installing Planters, Basins, Filter Strips, Grassy Swales, and an Ecoroof. These facilities are allowed per City of Portland SWMM Table 2-9. Summary of Generally Allowed Stormwater Facilities.

The proposed planters and basins are designed using the Presumptive Approach Calculator (PAC) and meet the requirements of Section 2.5.2. Presumptive Approach. Due to limited space between the buildings, paved access roads, and underground piping, these facilities will be designed for stormwater quality treatment only.

Basin 11A is sized using HydroCAD under the Performance Approach, rather than the PAC Calculator due to its unique geometry as a tiered facility with three levels divided by check dams, but with no longitudinal slope. This geometry is input into HydroCAD and analyzed using the site-specific water quality design storm described below.

Per DEQ Section 401 Water Quality Certification Post Construction Stormwater Management Plan Section E.1.1, the water quality storm event depth is the site's 2-year, 24-hour precipitation. Per Table 3 below the rainfall depth is 2.8 inches. The water quality design storm factor is 0.5 for this site, resulting in a water quality design storm depth of 1.4 inches. This is less than the rainfall depth that is used in the City of Portland's PAC, which is 1.61 per section A.2.1 Water Quality Treatment Volume goal in the SWMM.

The grassy swales are a rate-based facility and will be designed to meet the requirements of City of Portland SWMM Table 2-12. Water Quality Storm and Section 3.2.5.5. Grassy Swales.

Vegetated filter strips will be designed to meet the requirements of City of Portland SWMM Table 3-6.

The ecoroof will be designed to meet the requirements of City of Portland SWMM Section 3.2.1.1.

The proposed development will contribute runoff to Johnson Creek. As such, pollution reduction will be provided for all runoff from proposed impervious areas by routing the runoff through either basin, flow-through planter, or grassy swale facilities. The basins and flow-through planters will filter out the relevant pollutants via percolation of runoff through the vegetation, growing medium, and gravel within the facilities. The grassy swales will filter out the relevant pollutants by routing runoff through the densely vegetated surfaces of the facilities. The proposed vegetation will remove pollutants as runoff slowly flows to the downstream ends of the facilities.

1.6.4 Stormwater Flow Control

This project is proposing to use five Dry Detention Ponds, a sloped basin, and an Ecoroof to achieve the stormwater flow control requirements. These facilities are allowed per the City of Portland SWMM Table 2-9. Summary of Generally Allowed Stormwater Facilities.

The proposed ponds are designed using the Santa Barbara Urban Hydrograph Method (SBUH) with an NRCS Type IA synthetic rainfall distribution. The calculations were executed with the computer program HydroCAD. This method was used to generate site runoff hydrographs, determine peak flows, and perform pond routing analysis.

The ponds are designed using the Performance Approach meeting the requirements of the City of Portland SWMM Section 2.5.3. Performance Approach and Sub-section 2.5.3.2. Designing for Flow Control Requirements, per Table 2 below:

Table 2. Flow Control Requirements*							
Design Storm Event	24-hr Rainfall Depth (inches)**	Requirements by the Receiving System					
2 Year	2.8	Limit 1/2 the 2-year post-development peak flow to 1/2 the 2-year pre-development peak flow					
5 Year	3.4	Do not exceed pre-development peak flows					
10 Year	3.8	Do not exceed pre-development peak flows					
25 Year	4.5	Do not exceed pre-development peak flows					

*Table 2 is a partial table taken from the SWMM Table 2-13.

**Rainfall depths are taken from Table 3 below.

Rainfall Data

The 24-hour rainfall depths used in the hydrographs to determine the peak stormwater runoff rates for this analysis were obtained from NOAA ATLAS 2, Volume X, Isopluvial Maps Figures 25 to 30 summarized in Table 3 below.

Table 3. 24-hour Rainfall Depths					
Design Storm Event	24-hr Rainfall Depth (inches)				
2 Year	2.8				
5 Year	3.4				
10 Year	3.8				
25 Year	4.5				
50 Year	5.0				
100 Year	5.5				

These rainfall depths are based on the physical location of the project. As such, they do not match the City of Portland rainfall depths listed in Table A-9. 24-Hour Rainfall Depths at Portland Airport of the SWMM.

Existing Hydrologic Soil Groups

The existing soils on-site are as follows (Per NRCS Soil Maps in Attachment B):

- Cazadero Silty Clay Loam, Hydrologic Soil Group C
- Wollent Silt Loam, Hydrologic Soil Group C/D
- Borges Silty Clay Loam, Hydrologic Soil Group D
- Haplumbrepts, Hydrologic Soil Group B

The curve numbers discussed below are based on this hydrologic soil group.

Curve Numbers

The existing site landscape character is agricultural and has served as a nursery / tree farm for decades. Tree starts are planted tightly in rows and harvested as very young (estimated at 1/2-inch caliper/6-foot height average) bare root stock. The crop rows are oriented to drain as quickly as possible that results in surface runoff to the low points of the site. Much of the site does not have developed groundcover vegetation and has a significant amount of exposed soil.

The proposed landscape will include permanent native grassland seeding with some locations being populated by trees and understory plants appropriate for the surrounding context. This area will be mown infrequently, maybe two times a year as a management strategy. Inside the secure facility, the landscape will be more of a campus style development with maintained and irrigated landscape beds integrated with the proposed buildings and parking areas to meet development codes and provide a hospitable environment. The proposed landscape outside the secure area to the west, south and east of the facility will include the same permanent native grassland seeding with scattered groupings of native oaks and pines with associated understory plants. Some portions of this area will be replanted similar to adjacent forested areas which will include large native evergreen trees and native understory plantings.

Vegetated stormwater facilities will be integrated into the landscape design inside the secure area and outside it to connect to the lowest points of the site where stormwater eventually exits. The vegetated facility types include stormwater planters and basins, flow control ponds and planted swales to convey runoff to the low points in a controlled manner. Stormwater facilities will be planted in a manner that matches the visual character of the immediate surrounding landscape while introducing more significant permanent vegetation.

Pre-developed Curve Numbers

The existing site primarily consists of hydrologic soil group (HSG) of C.

Using the TR-55 Urban Hydrology for Small Watersheds, Table 2-2b. Runoff Curve Numbers for Cultivated Agricultural Lands, cover type of Row Crops, having a treatment of straight rows and crop residue cover in good condition, a curve number of 82 has been selected for the entire site. This land coverage is consistent across the entire site. This curve number is being used in the hydrologic flow calculations to determine the existing stormwater flows that occur on the site.

Developed Curve Numbers

During construction this site will experience significant disturbance, causing the existing soil profile to be altered and consolidated. Construction activities include mass grading and compaction, utility installation, building construction and roadway construction. This consolidation will also affect the existing soils infiltration rate. As a result of this disturbance, the developed curve numbers were selected using a HSG of D. This selection is based on TR-55 Urban Hydrology for Small Watersheds, Appendix A, Disturbed Soils Profiles.

Using the TR-55 Urban Hydrology for Small Watersheds, Table 2-2a. Runoff Curve Numbers for Urban Areas, with cover type of impervious paved parking lots, roofs, driveways, etc., a curve number of 98 has been selected. This CN has been applied for all impervious areas of the site.

Using the TR-55 Urban Hydrology for Small Watersheds, Table 2-2a. Runoff Curve Numbers for Urban Areas, Open Space (lawns, parks, golf courses, cemeteries, etc.,), good condition (grass cover > 75%), a curve number of 80 was selected. This CN has been applied to the landscape areas within drainage basins A, B, C1, C2, D, E and F as well as the cross-property flows from subbasins 1A and 2A. Refer to Figure 2 Post Construction Stormwater Drainage Basin Map Cross-Property and Figure 3 Site Stormwater Drainage Basin Map Flow Control for locations.

These curve numbers are being used in the hydrologic flow calculations to determine the developed conditions stormwater flows that occur on the site.

1.6.5 Stormwater Conveyance

An onsite system of pipes and ditches will convey collected onsite flows to and from proposed treatment and detention facilities before routing flows to site discharge points.

The proposed pipes and ditches are designed using the Santa Barbara Urban Hydrograph Method (SBUH) with an NRCS Type IA synthetic rainfall distribution. HydroCAD is used to determine tributary basin flows and to analyze the capacity of proposed ditch geometries. Pipe capacity analysis is completed in a separate spreadsheet.

Standards outlined in the City of Portland 2020 Sewer and Drainage Facilities Design Manual (SDFDM) section 6.4.1 state that piped flows must be conveyed without surcharge during the 10-year design storm and with a minimum 6 inches of freeboard within the system during the 25-year design storm. The proposed onsite piped system is designed to convey the 25-year design storm without surcharging. The 25-year design storm is also used to size all conveyance ditches per Table 6-2 in the 2020 SDFDM.

Segments of the conveyance system managing pumped underdrain flows from Areas 33, 35, 37, and the overflow basins will be sized and analyzed using different assumptions as the rest of the conveyance system. Pumped flow rates from groundwater entering Pond C were determined by others and was forced through the relevant conveyance pipes as a base flow, rather than flows from a standard tributary basin.

The primary pump station just south of Area 40 pumps flows to the flow spreader at proposed Point of Discharge #2. It is assumed that this pump discharges flows from a modified 2-year design storm, while the overflow basins provide storage for larger storm events while the underdrain system empties. The conveyance system downstream of the pump station is designed to convey the 2-year design storm for the 30-minute period where the design storm flows peak. The flow forced through these pipes by the pump is approximately 4 cfs as this flow occurs approximately 15 minutes before 2-year flows reach a peak. It is assumed that all runoff from the pumped areas enter the underdrain system. To supplement for the conveyance system managing a lower storm event from the pumped areas, the overflow basins will be analyzed for their capacity to contain the 25-year design storm from all pumped areas assuming the pump station is not running.

All curve number and rainfall data assumptions stated in section 1.6.4 of this report will apply to design methods regarding the proposed onsite conveyance system.

2.0 Existing and Pre-Developed Stormwater Drainage Conditions

2.1 Description of Existing Stormwater Drainage Conditions

This site is an existing nursery crop production facility and contains no existing storm drainage infrastructure. This site is bounded by dirt access roads and several dirt access roads are located within the project's limits. This site is bisected by a ridge line where the majority of the property generally slopes to the west or southwest. There is a small portion from the ridge line that flows southeasterly. (See Figure 1 – Existing Stormwater Drainage Basin Map)

Subbasin 1 is located at the northwest corner of the property wedged between SE Carpenter Lane and a private gravel access road. The existing stormwater drains westerly across the property line at an angle onto the adjacent farmland. The flows are partially by sheet flow and partially by shallow concentrated flow. The shallow concentrated flows are collected by an existing 8" PVC culvert that crosses the gravel road at a low point. The culvert drains to the west and provides stormwater to an existing ditch as well as an unnamed tributary of Johnson Creek.

Subbasin 2 is the central basin encompassing the majority of the site. The basin extends from the east side of the property (being an existing gravel access road) to the west side of the site. The existing stormwater drains to a low point on the west property line. The flows are partially by sheet flow and partially by shallow concentrated flow. This low point is the same 8" PVC culvert described above.

Subbasin 3 is located at the southwest corner of the property wedged between the west and south property lines. The west and south property lines consist of a dirt access road. The existing stormwater drains across both the west and south property lines at an angle eventually draining to the headwaters of Johnson Creek. Johnson Creek is approximately 100 feet southwest of the basin, and flows are partially by sheet flow and partially by shallow concentrated flow.

Subbasin 4 is located on the south side of the property. The south property line consists of a dirt access road where the existing stormwater drains across the access road at an angle eventually draining to the headwaters of Johnson Creek. Johnson Creek is approximately 100 feet southwest of the basin, and flows are partially by sheet flow and partially by shallow concentrated flow.

Subbasin 5 is located at the southeast corner of the property wedged between the east and south property lines. The east and south property lines consist of a gravel access road. The existing stormwater drains easterly

to a low point on the east property line where it crosses the existing gravel road onto an adjacent property. The stormwater continues to flow east where it disperses into an existing forested area.

The cross-property stormwater flows are discussed in Section 4 Cross-Property Drainage.

2.2 Existing Stormwater Points of Discharge

The existing stormwater runoff occurs via sheet flow and shallow concentrated and/or drainage ditches. There are three points of discharge from the site, identified in Figure 1.

Existing Point of Discharge #1 is located on the west side of the site. It is a low-lying area where the shallow concentrated flows are collected by two existing 8" PVC culverts that crosses the gravel road at a low point to an existing drainage ditch. The culvert drains to the west and provides stormwater to the existing ditch as well as an unnamed tributary of Johnson Creek. These existing pipes are full of debris and need to be cleaned prior to connecting to them. Existing subbasins 1 and 2 drain to this point of discharge.

Existing Point of Discharge #2 is located along southwest portion of the site, draining southwesterly to a vegetated area at the headwaters of Johnson Creek. Existing subbasins 3 and 4 drain to this point of discharge.

Existing Point of Discharge #3 is located on the east side of the site. The existing stormwater drains easterly to a low-lying area where it crosses the existing gravel road onto an adjacent property. The stormwater continues to flow east where it disperses into an existing forested area. Existing subbasin 5 drains to this point of discharge.

SE Carpenter Lane slopes westerly to SE Cottrell Road. There are no defined drainage ditches or stormwater infrastructure within the right-of-way of SE Carpenter Lane. The existing runoff sheet flows into the adjacent farm fields.

This site consists of five existing subbasins. See Attachment A, Figure 1 Existing Stormwater Drainage Basin Map.

3.0 Proposed Post Construction Stormwater Drainage

3.1 Description of Proposed Stormwater Drainage Conditions

The proposed improvements include the construction of buildings, above-ground tanks, underground tanks, loading areas, and storage areas. These newly generated impervious surfaces will drain to trap-type catch basins, a piped conveyance system, grass-lined drainage ditches, and detention ponds.

The mitigated portion of the site provides stormwater quality and stormwater flow control prior to discharging off-site and one of three Points of Discharge. Most of these areas drain via surface flow to water quality facilities or directly into a conveyance system which routes all treated or directly collected flows to five dry detention ponds. Some mitigated areas drain via surface flow directly into the ponds. See Attachment A, Figures 3, 4, and 5 for maps of mitigated basin areas regarding the flow control, water quality, and conveyance systems, respectively.

The non-mitigated areas are areas where cross-property flows occur post construction, along the west property line and a portion of the south property line. These flows are not managed by flow control or a water quality facility. These areas do not contain impervious surface and consist of landscaping. See Attachment A, Figure 2 Post Construction Stormwater Drainage Basin Map Cross-Property, which is discussed below in section 4 Cross-Property Drainage.

Existing drainage patterns along Carpenter Lane will be maintained after development. Per conversations with Multnomah County, no new storm drainage infrastructure will be added along Carpenter Lane to intercept and collect flows from this drainage pattern.

A flow splitter controls the amount of stormwater that drains to the Point of Discharge #1. Flows will enter the flow splitter from Pond D and from unmitigated areas along the northwest portion of the site before being routed to either Point of Discharge #1 or Point of Discharge #2, where all other mitigated flows exit the site. See Section 3.4 Site Points of Discharge Locations discussing total flows discharging off site.

The emergency gravel access road (Road B) extends from the southeast corner of the facility site south to SE Bluff Road. Road B is bisected by a ridge line, as can be seen in Figures 3, 4, and 5. The north section of Road B drains north and utilizes the interface of the shoulder and existing farmland as a channel. The north section eventually drains to a grassy swale adjacent to Road B where flows will be treated for water quality. This grassy swale drains flows southwesterly to Pond C for flow control.

The south section of Road B (south of the ridge line) drains south within a new drainage way that utilizes the interface of the shoulder and existing farmland as the channel. Flows drain to Pond E, which contains a grassy swale at the bottom of the proposed pond volume.

3.2 On-Site Stormwater Quality Treatment

Treatment of stormwater runoff is proposed for the site's building and roadway impervious areas. Treatment will be provided by vegetated planters, stormwater basins, grassy swales, vegetated filter strips, and an ecoroof (See Figure 4. Site Stormwater Drainage Basin Map Water Quality). Proposed planters and grassy swales are sized water quality treatment only, while stormwater basin E1 and the ecoroof will additionally provide flow control. Flows exiting the treatment-only facilities will be conveyed to one of the five on-site dry detention ponds, as discussed below in Section 3.3. The treatment areas are delineated based on building downspout layouts and roadway grading. The treatment basins shown in Attachment A, Figure 4 are assumed to be 100% impervious. Table 4 below is a summary of the water quality treatment facility and stormwater flows treated in each catchment area.

The vegetated planters and stormwater basins have been designed using the City of Portland's Presumptive Approach Calculator (PAC). Grassy swales were designed using separate spreadsheets under Performance Approach standards. Basin 11A was sized using HydroCAD, with the results of the model showing that the water quality storm does not overtop any facility overflow structures. Vegetated filter strips have been designed using the minimum geometries outlined in Table 3-6 of the City of Portland SWMM. PAC and swale spreadsheet calculations are in Attachment D.

	Table 4.	Proposed Stormwa	ater Quality	Facility Sum	nmary	
Facility #	Contributing Impervious Area (sf)	Facility Type(ft)	Bottom Area (sf)	Blended Soil Depth (in)	Rock Storage Depth (in)	Overflow Depth (in)
11A	26,106	Basin	39	18	12	12
11B	35,031	Planter	656	18	12	12
11C	7,918	Planter	257	18	12	12
12A	8,535	Planter	206	18	12	12
12B	20,925	Planter	324	18	12	12
40A	93,777	Ecoroof	0	0	0	0
65A	10,752	Planter	508	18	12	12
70A	36,357	Planter	513	18	12	16
70B	18,015	Planter	314	18	12	12
81A	58,265	Planter	1,344	18	12	12
F1	48,779	Sloped Basin	1,260	12	12	18
E1	17,024	Sloped Basin	1,202	12	12	18
Total	381,484					
		Grass	y Swales			
Facility #	Treatment Area (sf)	Treatment Flow (cfs)	Length (ft)	Slope (%)	Flow Depth (in)	Velocity (ft/sec)
Area 12C	51,507	0.20	137	0.5	4	0.20
Area 51	28,769	0.11	144	1	4	0.10
Road A	22,489	0.09	251	1	4	0.08
Road B1	48,943	0.19	507	1	4	0.18
Road B2	36,099	0.14	158	1	4	0.13
Road F	8,475	0.03	76	1	4	0.03
Total	196,282					
		Vegetated	d Filter Strips			
Facility #	Tr	eatment Area (SF)			Width (ft)	
1		11,277			20	
2		20,813		20		
3		19,281			20	
4		5,291		20		
5		10,340			20	
Total 67,002						

3.3 On-Site Stormwater Flow Control

Five Dry Detention Ponds (being A, B, C, D and E), one sloped basin, and an Ecoroof will achieve stormwater flow control requirements for this project. These facilities are allowed per the City of Portland SWMM Table 2-9. Summary of Generally Allowed Stormwater Facilities. See Attachment A, Figure 3 for a map of the detention basins. Tables 6-10 below are summaries of the dry detention ponds' performance. See Attachment E for plots of the pond flow control analysis.

Basin F, consisting of a section of Road E and adjacent landscaping drains to a sloped basin and will manage both water quality and flow control. The sloped basin will have check dams every 50 feet. The stormwater basin was designed using the PAC. A report of the PAC results can be found in Attachment D.

Sloped Basin E1 summary:

- 3 feet wide bottom
- 3:1 side slopes
- Check dams at 50' o.c.
- 12" topsoil depth
- 12" storage rock depth, 3' wide
- 1% running slope
- 400 feet in length

There is one ecoroof proposed in this project, located on Building 40. Per City of Portland's SWMM Table 2-9. Summary of Generally Allowed Stormwater Facilities the ecoroof can provide both stormwater quality treatment and stormwater flow control. Per Table 2-10. Simplified Approach Sizing and Design the sizing factor is 1, meaning the required area of the ecoroof is 1 times the impervious (or roof) area.

Table 5. Area and Composite Curve Number Summary								
Drainage Basin	Total Area (SF)	Impervious Area (SF)	Pervious Area (SF)	Ecoroof (SF)	CN	СNр	CNecoroof	Composite CN
А	349,468	176,142	173,326	-	98	80	61	89
В	294,211	160,056	134,155	-	98	80	61	90
С	754,337	82,047	672,290	-	98	80	61	82
D	485,197	135,356	256,064	-	98	80	61	85
E	580,537	36,209	544,328	-	98	80	61	81
F	103,464	17,069	86,395	-	98	80	61	83
G	93,777	-	-	93,777	98	80	61	61

Curve number selection is discussed in section 1.6.4.

3.3.1 Time of Concentration

Stormwater runoff travels through a watershed as sheet flow, shallow concentrated flow, open channel flow, or a combination of these flows. This project calculates the pre-developed time of concentration using sheet flow for the first 300 feet of travel and shallow concentrated flow for the remaining travel distance to one of the three existing points of discharge. The calculations are based on the methods and formulas described in TR-55-Chapter 3 Time of Concentration and Travel Time.

The minimum time of concentration for all the post development stormwater basins of 5 minutes was used in the calculations for determining the peak runoff rates, per City of Portland's SWMM Section 1.3.5 Level 2: Separated Storm System Requirements.

3.3.2 Dry Detention Ponds

This site has five dry detention ponds (A, B, C, D, and E) that are described below. The stormwater is treated for water quality by separate features and is discussed in Section 3.2. On-Site Stormwater Quality Treatment.

- Pond A is located on the north side of the site adjacent to the Maintenance Building.
- Pond B is located on the east side of the site adjacent to the Wash-Water Clarification structure.
- Pond C is located on the south side of the site, east of the Overflow Basins.
- Pond D is located on the west side of the site, west of Building 16 North Electrical Complex.
- Pond E is located at the southeast corner of the site.

The dry detention ponds are designed with 1 foot of freeboard. The flow out of each pond will be controlled by a flow control manhole. The flow control manhole will have an overflow riser for larger storm events to bypass the pond.

Table 6. Pond 'A' Performance Summary								
Storm Event (year)	Pre-Developed Flow Rate (cfs)	Developed Flow Rate (cfs)	Allowable Release Rate (cfs)	Actual Release Rate (cfs)	Pond Water Elev (ft)	Pond Storage (cf)		
2	1.24	3.52	0.62	0.61	709.36	13,290		
5	1.87	4.59	1.87	1.18	709.65	15,735		
10	2.31	5.33	2.31	1.56	709.85	17,390		
25	3.12	6.62	3.12	2.11	710.32	21,617		

Lower orifice 2.6" dia, IE = 696.43

Upper orifice 3" h X 16" w, IE= 709.40

*The allowable release rate is limited to 50 percent of the 2-year pre-Developed flow rate.

	Table 7. Pond 'B' Performance Summary								
Storm Event (year)	Pre-Developed Flow (cfs)	Developed Flow (cfs)	Allowable Release Rate (cfs)	Actual Release Rate (cfs)	Pond Water Elev (ft)	Pond Storage (cf)			
2	1.38	3.08	0.69	0.67	709.12	9,226			
5	2.07	3.98	2.07	1.34	709.48	11,166			
10	2.55	4.60	2.55	1.70	709.73	12,599			
25	3.43	5.70	3.43	2.22	710.29	15,967			

Lower orifice 3.5" dia, IE = 703.04

Upper orifice 3" h X 14" w, IE= 709.15

*The allowable release rate is limited to 50 percent of the 2-year pre-developed flow rate

		Table 8. Pond 'C' Performance Summary							
Storm Event (year)	Pre- Developed Flow (cfs)	Developed Flow (cfs)	Underdrain Pumps (cfs)	Total Inflow (cfs)	Allowable Release Rate (cfs)	Actual Release Rate (cfs)	Pond Water Elev (ft)	Pond Storage (cf)	
2	4.04	4.78	0.8	5.58	2.02*	1.86	704.82	15,943	
5	5.99	6.89	0.8	7.69	5.99	2.53	705.13	22,341	
10	7.37	8.39	0.8	9.19	7.37	2.92	705.34	26,741	
25	9.88	11.12	0.8	11.92	9.88	3.50	705.81	37,299	

Lower orifice 5.1'' dia, IE = 695.90

Upper orifice 3" h X 16" w, IE= 704.85

* The allowable release rate is limited to 50 percent of the 2-year pre-developed flow rate.

	Table 9. Pond 'D' Performance Summary								
Storm Event (year)	Pre-Developed Flow (cfs)	Developed Flow (cfs)	Allowable Release Rate (cfs)	Actual Release Rate (cfs)	Pond Water Elev (ft)	Pond Storage (cf)			
2	1.90	3.35	0.95*	0.94	694.57	8,181			
5	2.83	4.50	2.83	1.75	694.95	10,505			
10	3.49	5.31	3.49	2.18	695.33	12,303			
25	4.70	6.75	4.70	2.80	695.8	16,277			

Lower orifice 4.3'' dia, IE = 690.50

Upper orifice 3" h X 15" w, IE= 694.60

* The allowable release rate is limited to 50 percent of the 2-year pre-developed flow rate

Table 10. Pond 'E' Performance Summary								
Storm Event (year)	Pre- Developed Flow (cfs)	Developed Flow (cfs)	Allowable Release Rate (cfs)	Actual Release Rate (cfs)	Pond Water Elev (ft)	Pond Storage (cf)		
2	2.65	3.43	1.33*	1.30	713.96	8,316		
5	3.97	5.04	3.97	2.22	714.27	11,384		
10	4.90	6.18	4.90	2.71	714.50	13,759		
25	6.60	8.27	6.60	3.45	714.98	18,886		

Lower orifice 7.0" dia, IE = 711.00

Upper orifice 3" h X 16" w, IE= 713.95

* The allowable release rate is limited to 50 percent of the 2-year pre-developed flow rate.

3.4 Site Points of Discharge Locations

The site has three proposed Points of Discharge as described in Section 2.1 Description of Existing Stormwater Drainage Conditions above. See Figure 3 Site Stormwater Drainage Basin Map Flow Control for the following discussion.

The existing points of discharge will be maintained and utilized for this project as described below. The site's post developed stormwater flows are equal or less than the pre-developed stormwater flows, as shown in Table 11 below.

Most mitigated flows will route directly to Point of Discharge #2. Mitigated flows from Pond D and unmitigated flows from the northwest portion of the site will route through a flow splitter manhole, which routes discharges to both Point of Discharge #1 and Point of Discharge #2. An internal tee structure with a single orifice will control the routing of flows leaving the flow splitter. Most flows will route through the larger bypass pipe to Point of Discharge #2, while the rest will pass through the tee structure to Point of Discharge #1.

Proposed Point of Discharge #1 is located on the west side of the site. There are two existing 8" PVC culverts that cross the existing access road. Only one will be used for this project. This discharge location is not the primary discharge for the site. This discharge is to maintain and provide stormwater to the existing downstream storm system within the flow capacities of the existing culverts. This Point of Discharge receives stormwater from the site conveyance system by means of a flow splitter. The flow splitter will provide variable flow in the range of 0.47 to 0.66 cfs. See Table 11 for flow summary and Attachment G for plots of the flow splitter HydroCAD model.

The primary discharge location for this site is Proposed Point of Discharge #2, being Flow Spreader No. 1. It is located in the southwest corner of the site located between the sites Overflow Basins and the SEC Zone for water resources. The structure is a concrete channel with the downhill side lower than the uphill side. The structure has a bottom that will slope to a low point. This low point will have a drain with an 8" pipe to daylight, that will dewater the structure. The top of the facility is flat to provide uniform flow over the top of the structure, which flows to a layer of drain rock and a landscaped buffer area for erosion control. See Attachment H: Outfall Flow Spreader Calculations and Details. The structure is:

- 175 feet long
- Structure bottom running slope is 0.25%.
- Structure top is flat
- Design flow 16.82 cfs
- Flow depth is 0.10 feet
- Velocity of approximately 0.94 fps

Point of Discharge #2 has a greatly increased flow rate in the post-developed condition. However, with total onsite flows between all discharge points being reduced due to onsite detention, and the velocity of flows exiting the top of the flow spreader being less than 1 fps, increased erosion, or damage to downstream bodies of water are not anticipated.

The Proposed Point of Discharge #3 is located on the east side of the site adjacent to Pond E, utilizing the existing low area that exists. The pond with discharge on to the finished grade on the east side of the new emergency access road using a small bubbler consisting of a catch basin surrounded by a layer of drain rock. The mitigated stormwater runoff will continue to flow east in its current direction of travel. Point of Discharge #1 receives stormwater from the on-site flow splitter.

	Table 11. Point o	of Discharge Flo	w Summary				
	Flows	s to Flow Splitter					
Storm Event (year)	Pond Actual Release Rate (cfs)	Intercepted Unmitigated Flows (cfs)	Cross- property Flow (cfs)	Total Flow at Flow Splitter (cfs)			
2	4.24	0.90	1.26	6.40			
5	7.27	1.37	1.95	10.59			
10	9.02	1.71	2.46	13.19			
25	11.42	2.32	3.39	17.13			
	Point of Disch	narge #1					
Storm Event (year)	Pre-Developed Flow (cfs)	Flow From Flow Splitter to POD (cfs)	Total Flow POD (cfs)				
2	7.34	0.47	0.47				
5	11.01	0.54	0.54				
10	13.62	0.60	0.60				
25	18.41	0.66	0.66				
	Point of Disch	narge #2					
Storm Event (year)	Pre-Developed Flow (cfs)	Flow From Flow Splitter to POD (cfs)	Total Flow POD (cfs)				
2	4.60	0.91	9.33				
5	6.86	1.58	10.90				
10	8.46	2.23	13.09				
25	11.38	3.23	16.82				
Point of Discharge #3							
Storm Event (year)	Pre-Developed Flow (cfs)	Pond Actual Release Rate (cfs)	Total Flow POD #3 (cfs)				
2	2.65	1.30	1.30				
5	3.97	2.22	2.22				
10	4.90	2.71	2.71				
25	6.60	3.45	3.45	•			

3.5 Conveyance System

Onsite developed runoff will be collected y flowing overland to proposed catch basins, water quality treatment facilities, and detention facilities. After collection, flows will be routed to and from the various water quality and detention facilities and to the three points of discharge by a conveyance system made up of pipes and conveyance ditches. The proposed onsite storm conveyance system is a combination of 8", 10", 12", 15", 18", & 24" diameter storm pipes designed to convey the 25-year design storm without surcharge and conveyance ditch segments designed to convey the 25-year design storm under open channel flow conditions with adequate

freeboard. Maps of the tributary onsite basin areas and proposed conveyance system can be found in Attachment A, Figures 5 and 6, respectively.

Using the methods and assumptions outlined in section 1.6.5 of this report, both the piped conveyance system and conveyance ditch segments were analyzed for conveyance capacity. Along a series of pipes that experience the same flows, only the pipe with the lowest capacity was analyzed to determine the capacity of the series. The results of the system analysis show that all proposed pipes are adequately sized to convey the 25-year design storm under open channel flow conditions without any surcharging throughout the system. The analysis also shows that all conveyance ditches can adequately convey the 25-year design storm while maintaining more than the minimum required 0.5 ft of freeboard. Pipe and ditch segments managing pumped flows were determined to have sufficient capacity to convey the modified 2-year design storm from all pumped areas. Additionally, the Overflow basins were determined to have sufficient storage capacity for the 25-year design storm with a peak 25-year storage elevation of 685.19' and top of basin elevations at 693.00'. Table 12 below contains the results of the ditch analysis. See Attachment G for HydroCAD plots of the tributary basins and ditch segments, and the piped system calculation spreadsheet.

	Table 12. Conveyance Ditch Performance Summary								
Facility #	Bottom Width (ft)	Longitudinal Slope (ft/ft)	Flow Rate (cfs)	Flow Depth (ft)	Freeboard (ft)				
Ditch 1	2	0.019	1.86	0.25	0.5				
Ditch 2	3	0.022	1.42	0.17	0.83				
Ditch 3	2	0.008	2.46	0.37	0.63				
Ditch 4	0	0.038	5.82	0.73	1.02				
Ditch 5	3	0.005	4.50	0.49	0.51				
Ditch 6	2	0.015	3.65	0.38	0.62				

4.0 Cross-Property Drainage

The purpose of this section is to address how Oregon has adopted the civil law doctrine of drainage. Under this doctrine adjoining landowners are entitled to have the normal course of natural drainage maintained. The lower landowner must accept water which naturally comes to his land from above, but he is entitled not to have the normal drainage changed or substantially increased. The lower landowner may not obstruct the run-off from the upper land if the upper landowner is properly discharging the water.

4.1 Existing Surface Water Cross-Property Drainage

See Section 2.1 Description of Existing Stormwater Drainage Conditions.

4.2 Proposed Surface Water Cross-Property Drainage

Post-construction, the site will consist of four subbasins that will discharge offsite unmitigated. See Figure 2 Post Construction Stormwater Drainage Basin Map Cross-Property. This section does not discuss the discharge points, see Section 3.4 Site Discharge Locations.

The development of this site is reducing the stormwater flows crossing property lines by reducing the amount of area that crosses property lines, reducing the runoff curve number, or cutting off the drainage by installing drainage ditches along the property lines.

Subbasin 1A has been disconnected from cross-property flows due to the new drainage ditch along the west property line that extends from Carpenter Lane to the Point of Discharge No 1. These flows have been included in the point of discharge analysis in Section 3.4 Site Discharge Locations.

Subbasin 2A has been reduced in size due to the site development. It flows toward the Point of Discharge No. 1.

Subbasin 3A has been disconnected from cross-property flows due to the new drainage ditch along the west property line. The northern portion the subbasin 3 is captured by a drainage ditch, which is connected to the sites storm drainage system. The southern portion of the subbasin continues to flow across the property lines but has been significantly reduced. These flows have been included in the point of discharge analysis in Section 3.4 Site Discharge Locations.

Subbasin 4A has been reduced in size due to the site development. The subbasin continues to flow across the property lines but has been significantly reduced.

4.3 Existing and Proposed Cross-Property Hydrologic Analysis

The analysis was completed using the SBUH hydrological model with an NRCS Type 1A synthetic rainfall distribution. The 24-hour rainfall depths used for the analysis shown in Table 3 above were used in the calculations. Table 13 below is a summary of the pre-developed flows versus the post construction flows (or unmitigated flows) from newly landscaped areas. It shows that the cross-property flows have been reduced.

The pre-developed unmitigated flows consist of the upstream farmland, (See Figure 1 – Existing Stormwater Drainage Basin Map). The post construction flows (or unmitigated flows) are flows from the newly constructed landscape areas as can be seen in Figure 2 Post Construction Stormwater Drainage Basin Map Cross-Property. A curve number of 82 was used for the pre-developed flows (with no impervious surfaces included in the calculations) and a curve number of 80 was used for the post construction altered flows. The curve numbers are discussed above in section 1.6.4.

Table 13. Cross-Property Storm Drainage Flow Summary								
Subbasin	Design Storm (year)	Drainage Area (sf)	Pre-Developed Flow Farm Use (cfs)	Post Construction Landscaped Areas (cfs)				
1A	2	100,222	0.42	0.39				
	5		0.63	0.59				
	10		0.77	0.73				
	25		1.05	0.99				
2A	2	57,233	0.30	0.24				
	5	-	0.45	0.37				
	10	-	0.56	0.57				
	25	-	0.75	0.64				
3A	2	33,843	0.20	0.18				
	5		0.29	0.27				
	10		0.35	0.33				
	25		0.47	0.43				
4A	2	440,630	1.67	1.37				
	5	-	2.51	2.07				
	10	-	3.10	2.58				
	25	-	4.19	3.51				

5.0 Groundwater Management and Subdrain System

5.1 Description of Proposed Post-Construction Groundwater Discharge

This project is proposing to collect groundwater using a sub-drain system under Buildings 30 and 40. The groundwater is included in the sizing of Pond C, being 300 gpm (0.8 cfs) for managing flow control. This groundwater in combination with the site's mitigated surface stormwater flows will discharge to the site's flow spreader, Point of Discharge #2.

No known contamination exists on site, and construction site management activities and post-construction stormwater management will implement stormwater facilities conforming to SWMM stormwater facility selection, sizing, and design criteria.

Table 13 summarizes the approximate distance from the pond bottom elevation to the approximate groundwater depth as shown in the Final Geotechnical Data Report Technical Memorandum Dated July 15, 2022 (prepared by McMillan Jacobs Associates), Attachment K Preliminary Geotechnical Data Report (prepared by Rhino One). Groundwater is not anticipated to affect the ponds capacity.

	Table 14. Pond Bottom to Groundwater Depth								
Pond	Approx. Exist. Grade at Pond (ft)	Measured Water Level BGS (ft) and Boring # **	Approx. Groundwater Elev (ft)*	Pond Bottom Elev (ft)*	Approx. Depth to Groundwater (ft)*				
А	716	55.45 WTP-B-04	661	707.5	46.5				
В	718	24.66 WTP-B-01	693	707	14				
С	722	45.34 WTP-B-05	677	704	27				
D	698	55.45 WTP-B-04	643	693	50				
E	712	7.67 WTP-B-02	704	713	9				

* Elevations have been rounded to the nearest foot and are approximate. The elevations and depths were estimated based on the nearest boring.

** Final Geotechnical Data Report Technical Memorandum Dated July 15, 2022 (prepared by McMillan Jacobs Associates), Attachment K Preliminary Geotechnical Data Report (prepared by Rhino One) Table 7 Groundwater Level Observations Summary. The measured groundwater depths were measured on April 15, 2021, the shallowest measure and listed in Table 7.

6.0 Summary

The Multnomah County Design and Construction Manual refers to the City of Portland Stormwater Management Manual for stormwater quantity and quality control and conveyance design standards. The design of the proposed site satisfies both stormwater quality treatment, stormwater flow control, and stormwater conveyance standards as outlined in the City of Portland SWMM.

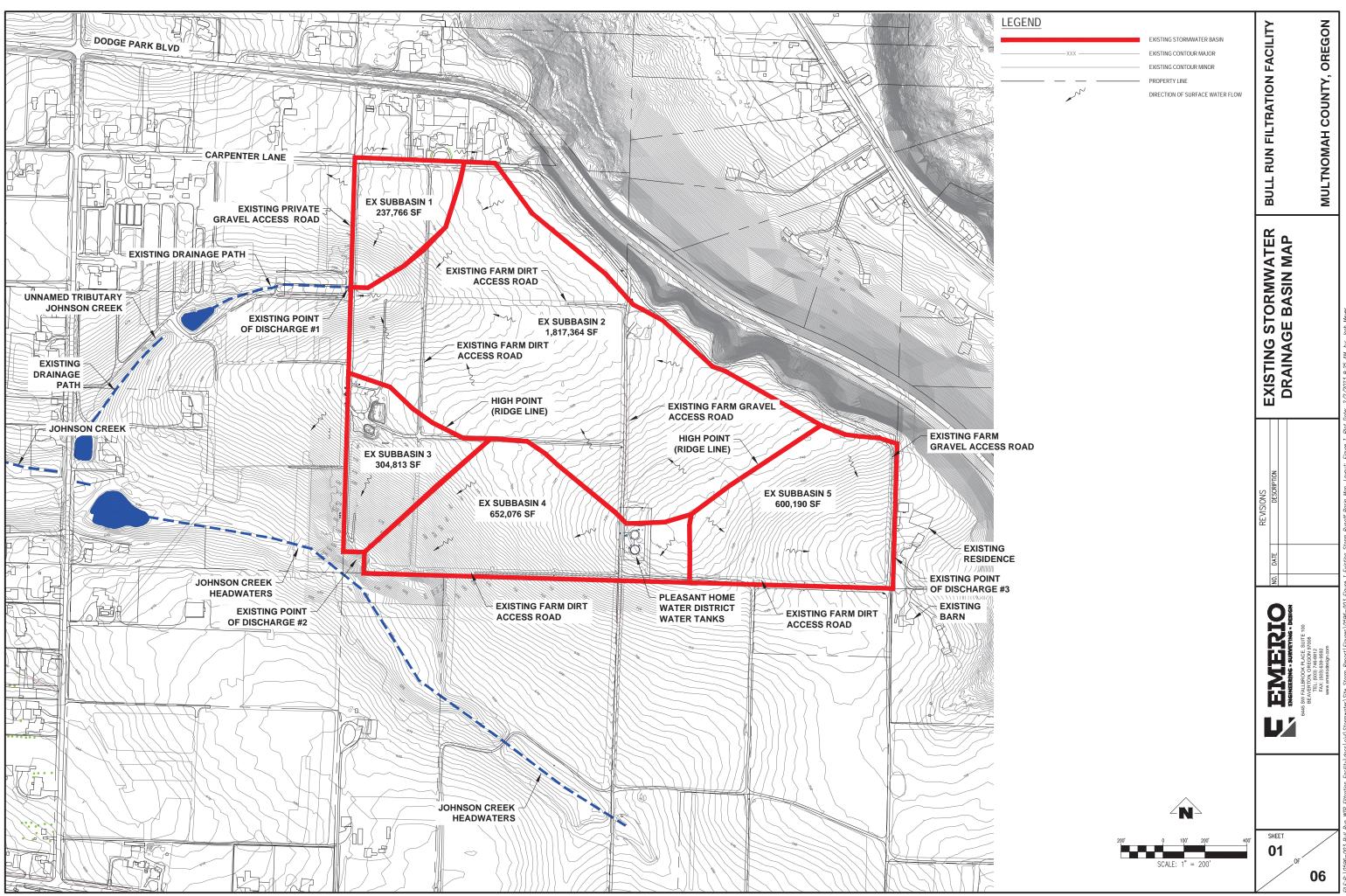
The flow control implemented in this project exceeds what is required by the SWMM, which requires the postdevelopment runoff rates to be less than or equal to the pre-development runoff rates for the 2-year through 25-year storm events. This project is providing flow control to the extent that the 2-year storm event is being detained while releasing one-half the total flow from the pre-developed 2-year storm event.

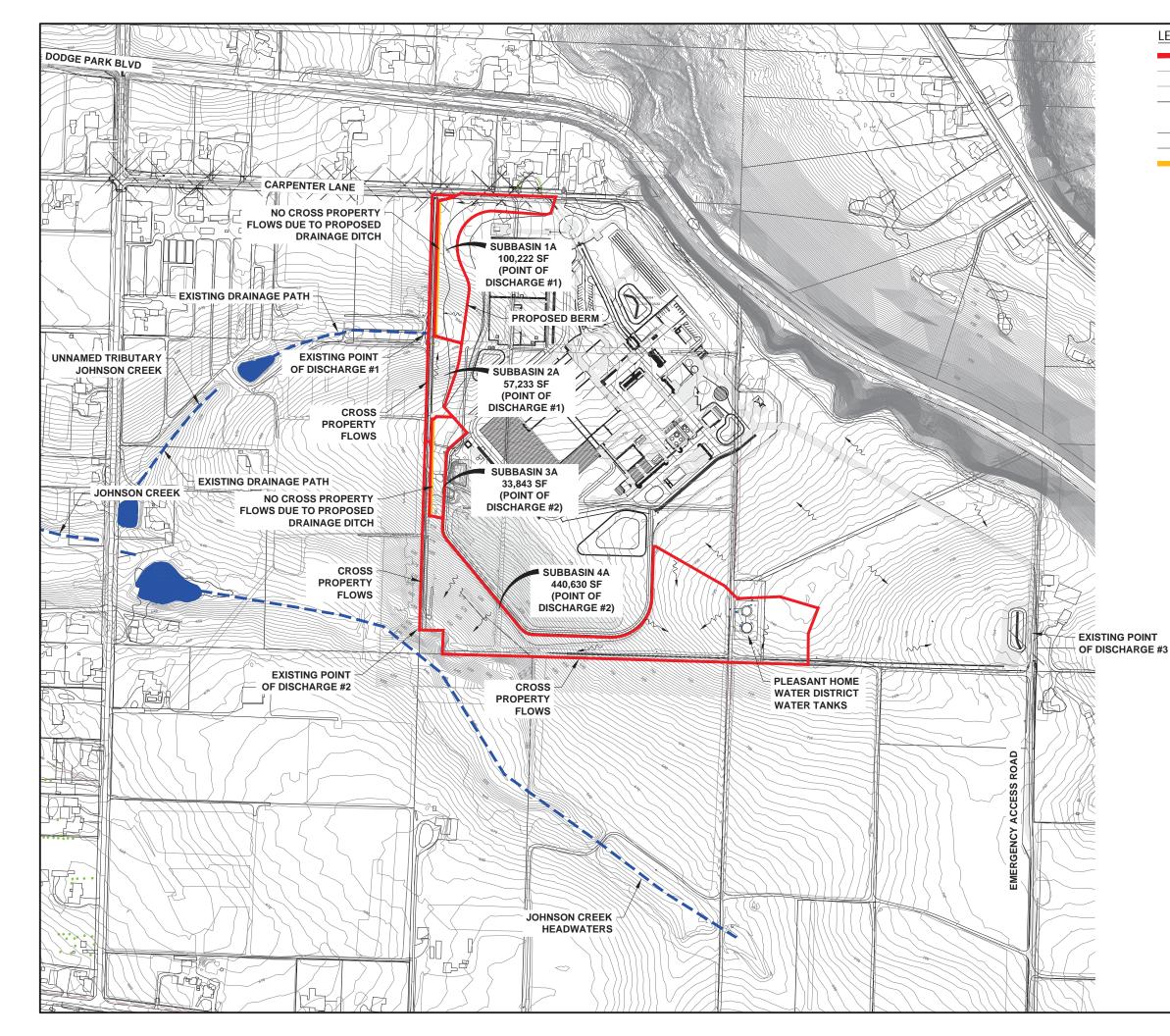
The site's post-construction improvements meet local and state stormwater quality treatment and flow control requirements, minimizing hydromodification in accordance with Portland SWMM Section 1.3.5.

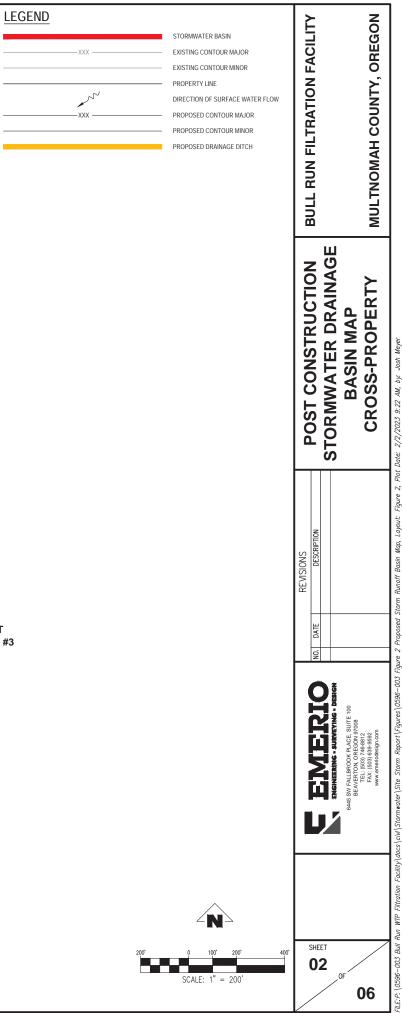
The conveyance system implemented in this project exceeds what is required by the SDFDM, which requires that the 10-year storm event pass through the system without surcharging.

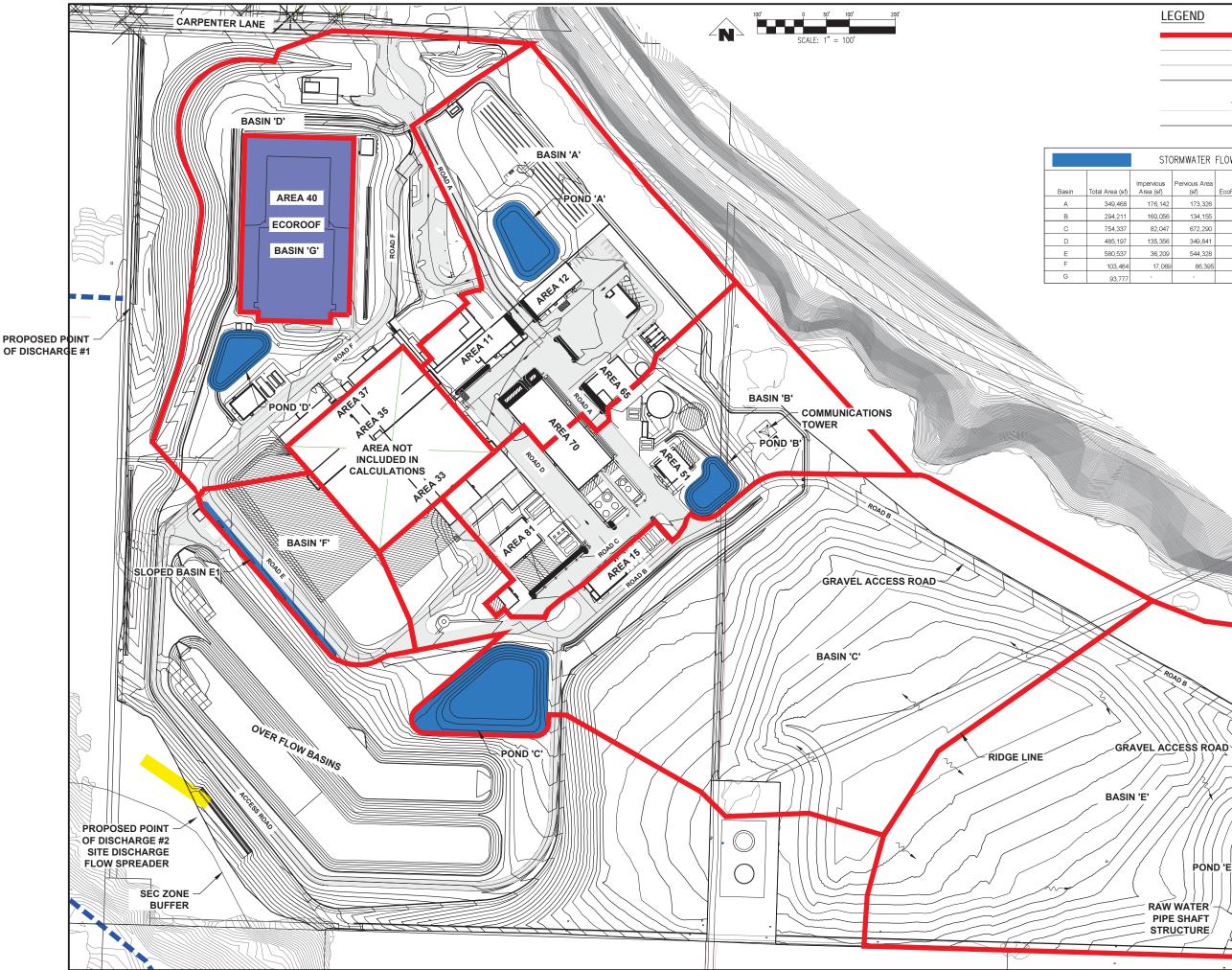
Attachment A: Stormwater Drainage Basin Maps

- Figure 1 Existing Stormwater Drainage Basin Map
- Figure 2 Post Construction Stormwater Drainage Basin Map Cross-Property
- Figure 3 Site Stormwater Drainage Basin Map Flow Control
- Figure 4 Site Stormwater Drainage Basin Map Water Quality
- Figure 5 Site Stormwater Drainage Basin Map Conveyance
- Figure 6 Conveyance System Map









LEGEND



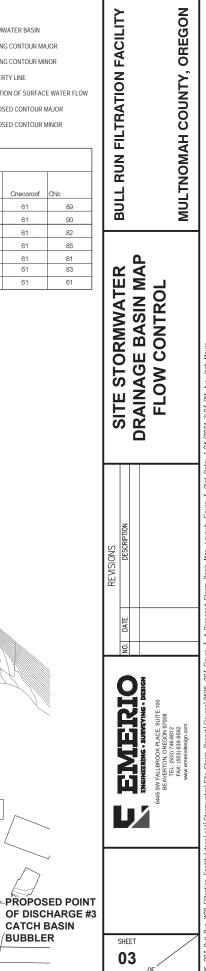
POND 'E

VATER BASI EXISTING CONTOUR MAJOR EXISTING CONTOUR MINOR PROPERTY LINE DIRECTION OF SURFACE WATER FLOW PROPOSED CONTOUR MAJOR PROPOSED CONTOUR MINOR

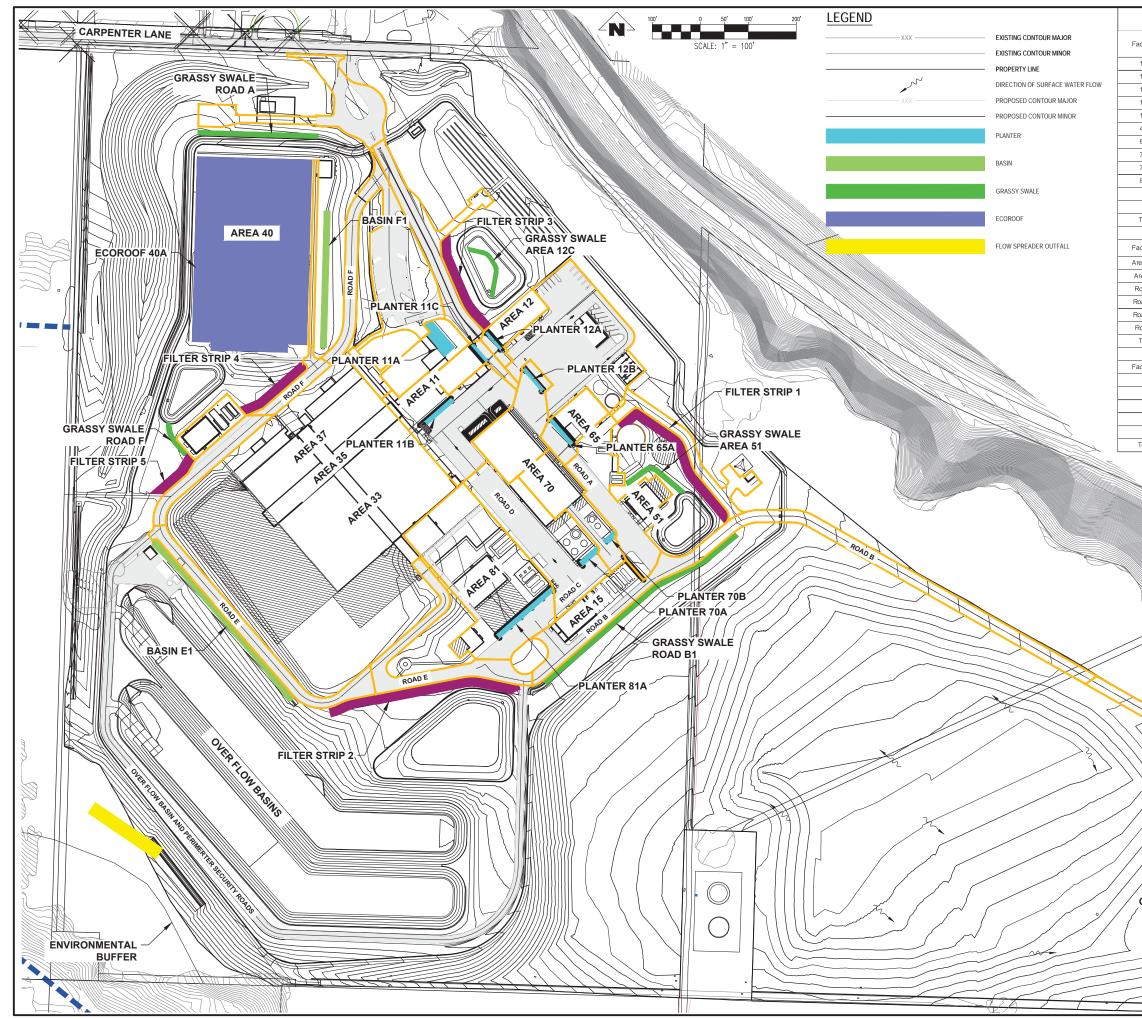
CATCH BASIN

BUBBLER

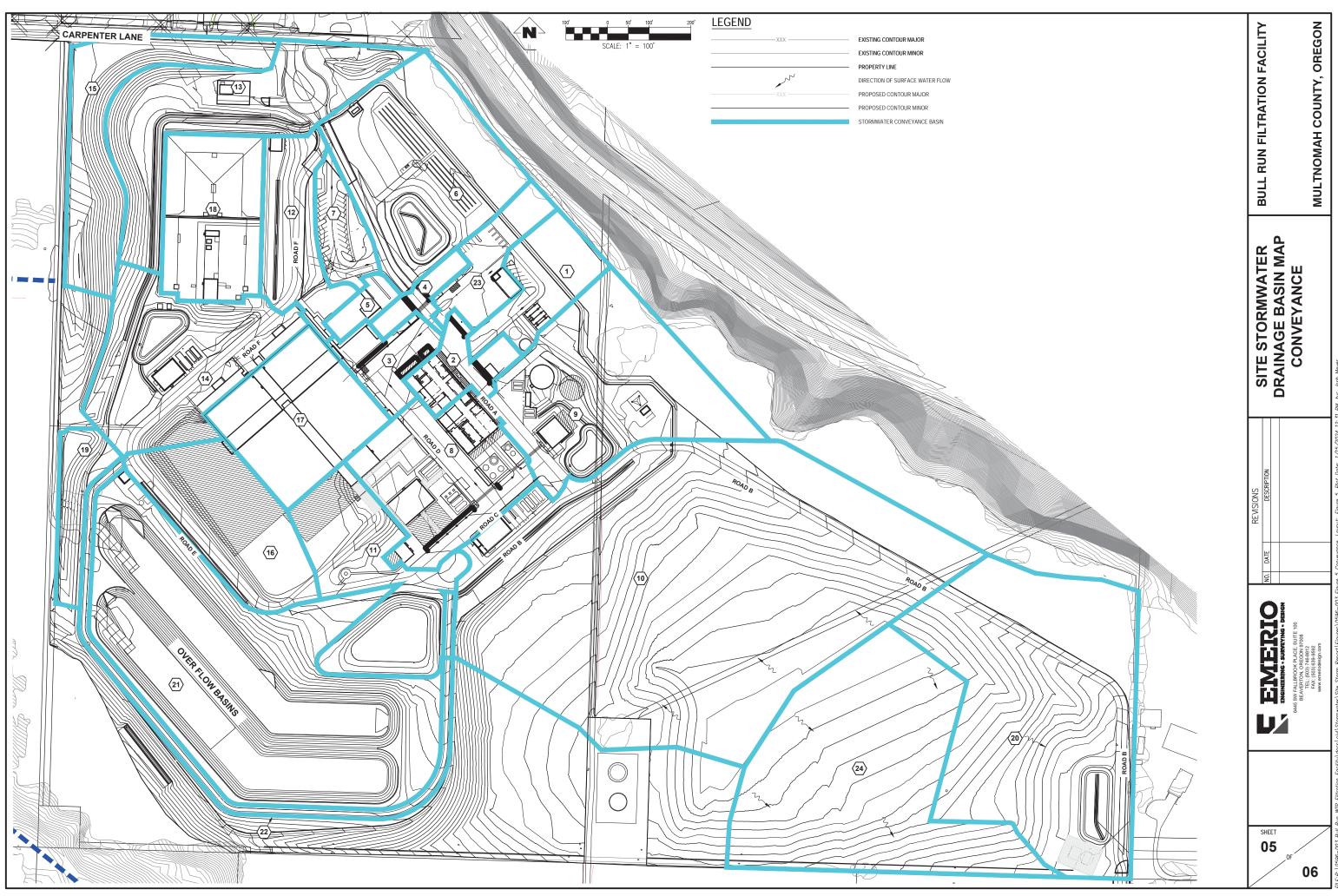
STORMWATER FLOW CONTROL POND DATA									
npervious Area (sf)	Pervious Area (sf)	EcoRoof (sf)	CNi	CNp	Cnecoroof	CNc			
176, 142	173,326	-	98	80	61	89			
160,056	134, 155	-	98	80	61	90			
82,047	672,290	-	98	80	61	82			
135,356	349,841	-	98	80	61	85			
36,209	544,328	-	98	80	61	81			
17,069	86,395	-	98	80	61	83			
-	-	93,777	98	80	61	61			

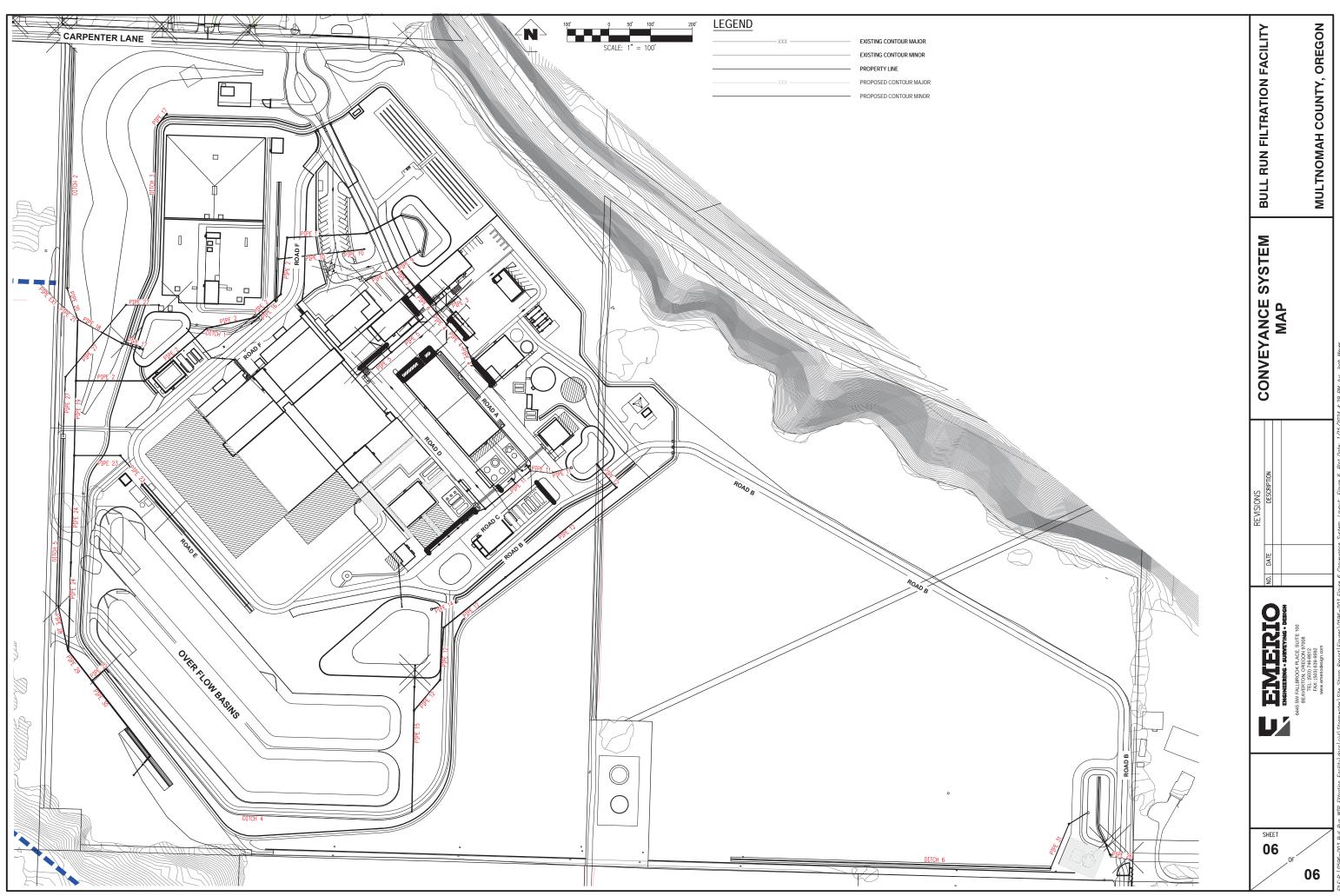


06



Bottom Area (sf) 39 656 257 206 324 0 508 513 314 1,344 1,260 1,202 1,20		Facility Type(t) I Basin I Planter I Sloped Basin I Sloped Basin I Sloped Basin I Flow(cfs) I 0.00 I 0.111 I 0.09 I 0.19 I 0.14 I 0.03 I	Contributing Impervous Area (sf) Facility Type(ft) 26,106 Basin 1 35,031 Planter 1 7,918 Planter 1 8,535 Planter 1 20,925 Planter 1 93,777 Ecoroof 1 10,752 Planter 1 36,357 Planter 1 381,484 O 1 7 C24 Sloped Basin 31,507 0.20 2 28,769 0.11 2 22,489 0.09 4 48,943 0.19 3 36,099 0.14 4 8,475 0.03 1 9,281 1,277
Blended Soil Depth (in) 18 18 18 18 0 18 18 18 18 18 18 18 18 18 18	(sf) Depth (in) 39 18 656 18 257 18 206 18 324 18 324 18 0 0 508 18 513 18 1,344 18 1,202 12 1,202 12 rassy Swales 1 Length (ft) Slope (%) 137 0.5 144 1 251 1 507 1 158 1 76 1 158 1	Type(ft)(sf)Depth (in)Basin3918Planter65618Planter25718Planter20618Planter32418Ecoroof00Planter50818Planter51318Planter31418Planter31418Planter1,34418Sloped Basin1,20212Sloped Basin1,20212TreatmentLength (ft)Slope (%)Flow (cfs)Length (ft)Slope (%)0.1014410.037610.1415810.037610.1415810.15Itler (Ftt)Itler (Ftt)Vegetter Filter Struct111,2771120,81315,29110,3401	Impensions Pachicy Bounder Son Area (sh) Type(th) (sf) Depth (in) 26,106 Basin 39 18 35,031 Planter 265 18 7,918 Planter 257 18 8,535 Planter 206 18 20,925 Planter 324 18 38,777 Ecoroof 0 0 10,752 Planter 513 18 36,357 Planter 513 18 36,357 Planter 1,344 18 48,779 Sloped Basin 1,200 12 17,024 Sloped Basin 1,202 12 148,444 1 12 14 24,869 0.99 251 1 48,943 0.19 507 1 36,099 0.14 158 1 196,282 1 1 192,81 1 1
	Bottom Area (sf) 39 257 206 324 0 508 513 314 1,344 1,260 1,202 1,	Facility Type(ft) Bottom Area (sf) Basin 39 Planter 666 Planter 205 Planter 206 Planter 324 Ecoroof 0 Planter 508 Planter 513 Planter 1,344 Sloped Basin 1,202 Sloped Basin 1,202 Sloped Basin 1,202 Treatment Length (ft) 0.20 137 0.11 144 0.09 251 0.11 144 0.09 251 0.11 144 0.09 251 0.11 144 0.09 251 0.13 76 0.14 158 0.15 76 11,277 20,813 20,813 19,281 10,340 5,291	Impervous Area (sf) Pachity Type(t) Bolliny (sf) 26,106 Basin 39 35,031 Planter 656 7,918 Planter 257 8,535 Planter 208 20,925 Planter 324 93,777 Ecoroof 0 10,752 Planter 508 36,357 Planter 513 18,015 Planter 513 18,015 Planter 1,344 48,779 Sloped Basin 1,260 17,024 Sloped Basin 1,202 381,484 D D Crassy Swales Treatment Felow (cfs) Length (ft) 51,507 0.20 137 28,769 0.11 144 22,489 0.09 251 48,943 0.19 507 36,099 0.14 158 8,475 0.03 76 196,282 Vegetated Filter Strip <tr< td=""></tr<>





Attachment B: NCRS Hydrologic Soil Group Information



USDA Natural Resources Conservation Service Web Soil Survey National Cooperative Soil Survey

	MAP L	EGEND)	MAP INFORMATION	
Area of Ir	iterest (AOI)	101	Spoil Area	The soil surveys that comprise your AOI were mapped at	
	Area of Interest (AOI)	0	Stony Spot	1:20,000.	
Soils		å	Very Stony Spot	Warning: Soil Map may not be valid at this scale.	
	Soil Map Unit Polygons	Ť	Wet Spot	Enlargement of maps beyond the scale of mapping can cause	
\sim	Soil Map Unit Lines	Δ	Other	misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of	
	Soil Map Unit Points	-	Special Line Features	contrasting soils that could have been shown at a more detaile	
Special	Point Features	· · · ·	·	scale.	
ဖ	Blowout	Water Fe	atures Streams and Canals	Please rely on the bar scale on each map sheet for map	
×	Borrow Pit	~		measurements.	
英	Clay Spot	Transpor	Rails	Source of Map: Natural Resources Conservation Service	
\diamond	Closed Depression		Interstate Highways	Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)	
X	Gravel Pit		US Routes	Maps from the Web Soil Survey are based on the Web Mercat	
**	Gravelly Spot		Major Roads	projection, which preserves direction and shape but distorts	
Ø	Landfill	~	Local Roads	distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more	
Ă.	Lava Flow			accurate calculations of distance or area are required.	
علم	Marsh or swamp	Backgrou	Aerial Photography	This product is generated from the USDA-NRCS certified data	
~	Mine or Quarry			of the version date(s) listed below.	
Ô	Miscellaneous Water			Soil Survey Area: Clackamas County Area, Oregon Survey Area Data: Version 18, Oct 27, 2021	
ŏ	Perennial Water			Soil Survey Area: Multhomah County Area, Oregon	
Š	Rock Outcrop			Survey Area Data: Version 20, Oct 27, 2021	
÷	Saline Spot			Your area of interest (AOI) includes more than one soil survey	
т **	Sandy Spot			area. These survey areas may have been mapped at different scales, with a different land use in mind, at different times, or a	
 a	Severely Eroded Spot			different levels of detail. This may result in map unit symbols, s	
0	Sinkhole			properties, and interpretations that do not completely agree across soil survey area boundaries.	
Ŷ	Slide or Slip			Soil map units are labeled (as space allows) for map scales	
\$ 	Sodic Spot			1:50,000 or larger.	
Ø				Date(s) aerial images were photographed: Jul 2, 2015—Sep 2016	

MAP LEGEND

MAP INFORMATION

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

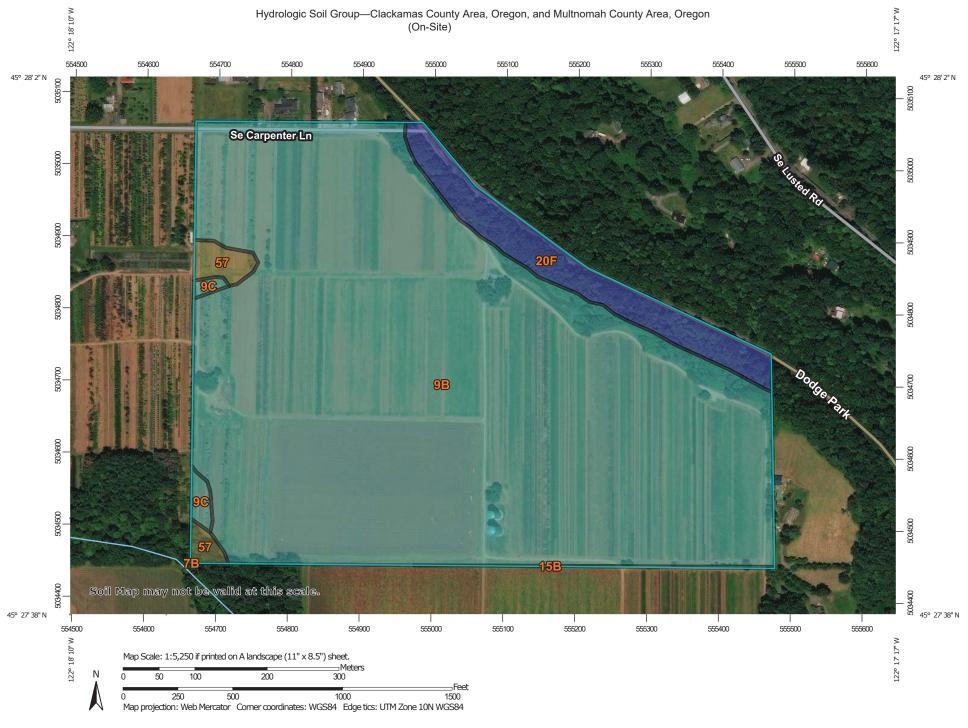


Map Unit Legend

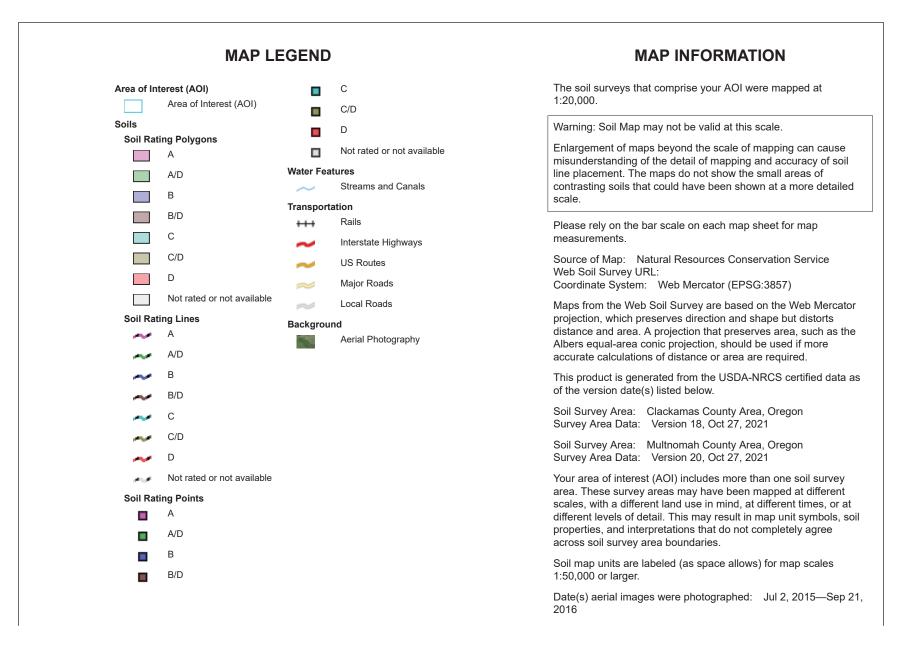
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
7B	Borges silty clay loam, 0 to 8 percent slopes	0.0	0.0%
15B	Cazadero silty clay loam, 0 to 7 percent slopes	0.1	0.1%
Subtotals for Soil Survey Area	l	0.1	0.1%
Totals for Area of Interest		100.0	100.0%

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
9B	Cazadero silty clay loam, 0 to 8 percent slopes	90.8	90.8%
9C	Cazadero silty clay loam, 8 to 15 percent slopes	0.7	0.7%
20F	Haplumbrepts, very steep	6.9	6.9%
57	Wollent silt loam	1.5	1.5%
Subtotals for Soil Survey	Area	99.9	99.9%
Totals for Area of Interes	t	100.0	100.0%





USDA Natural Resources Conservation Service Web Soil Survey National Cooperative Soil Survey





MAP LEGEND

MAP INFORMATION

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.



Hydrologic Soil Group

	1	1		
Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
7B	Borges silty clay loam, 0 to 8 percent slopes	D	0.0	0.0%
15B	Cazadero silty clay loam, 0 to 7 percent slopes	С	0.1	0.1%
Subtotals for Soil Surve	ey Area		0.1	0.1%
Totals for Area of Interest		100.0	100.0%	

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
9B	Cazadero silty clay loam, 0 to 8 percent slopes	С	90.8	90.8%
9C	Cazadero silty clay loam, 8 to 15 percent slopes	С	0.7	0.7%
20F	Haplumbrepts, very steep	В	6.9	6.9%
57	Wollent silt loam	C/D	1.5	1.5%
Subtotals for Soil Surv	ey Area		99.9	99.9%
Totals for Area of Intere	Totals for Area of Interest		100.0	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

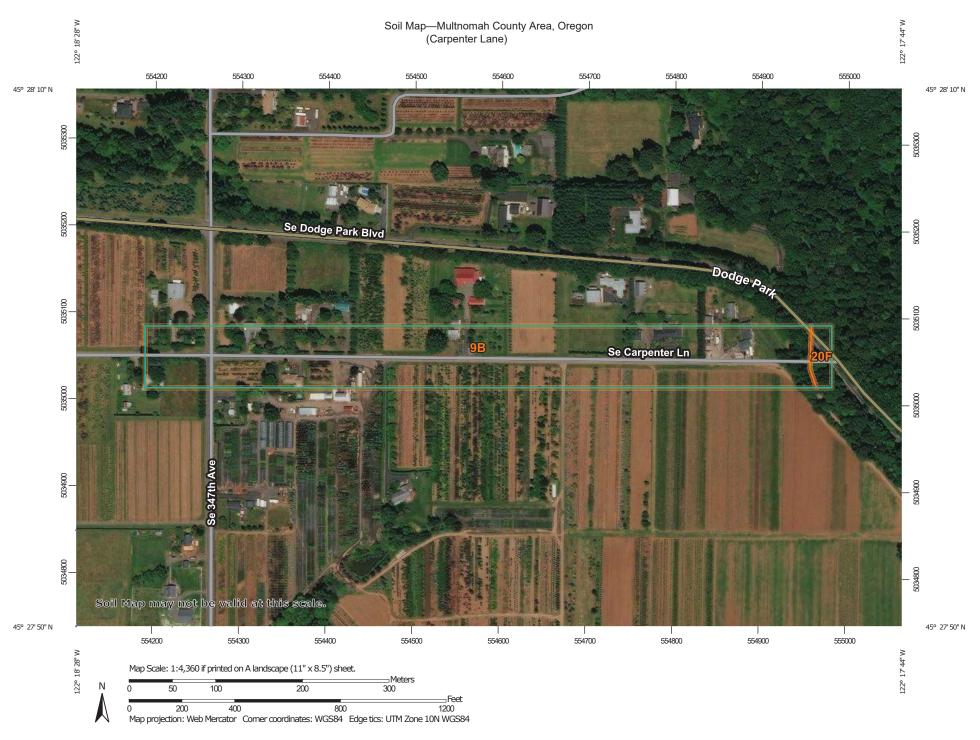
Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher





USDA Natural Resources Conservation Service

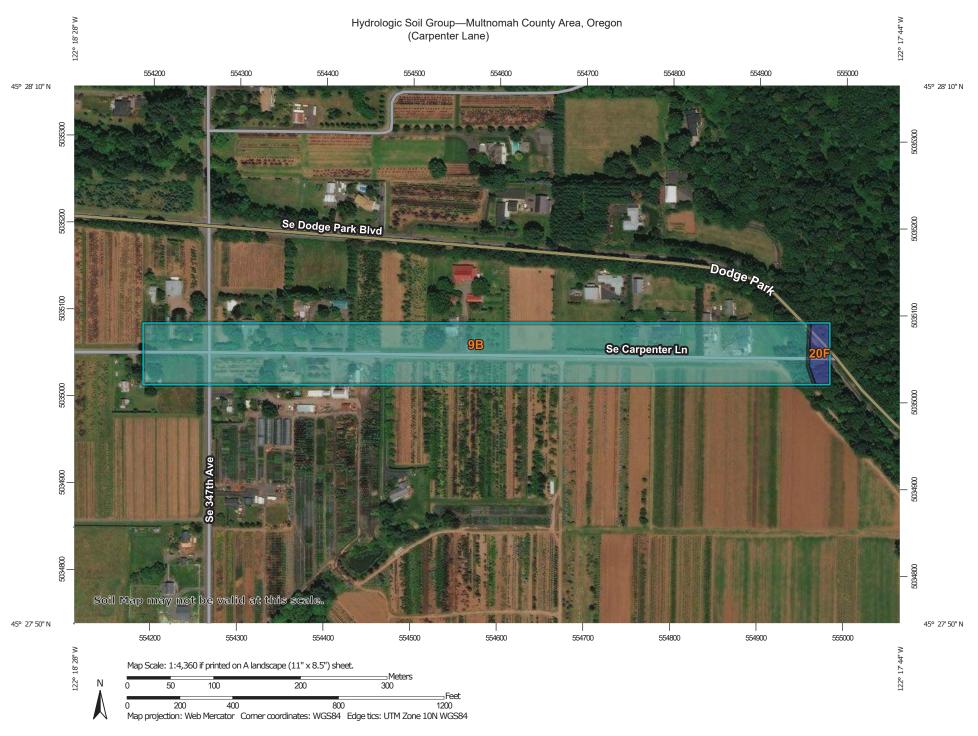
	MAP LEGEND	MAP INFORMATION
Area of Interest (AOI)	erest (AOI)	The soil surveys that comprise your AOI were mapped at 1:20,000.
	d Stony S	pot
Soils Soil Map I	nit Polygons	Warning: Soil Map may not be valid at this scale.
Soil Map U	😗 Wet Sp	et Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil
Soil Map U	∧ Other	line placement. The maps do not show the small areas of
Special Point Featu	special Special	Line Features contrasting soils that could have been shown at a more detailed scale.
(b) Blowout	Water Features	
Borrow Pit		s and Canals Please rely on the bar scale on each map sheet for map measurements.
💥 Clay Spot	Transportation Rails	Source of Map: Natural Resources Conservation Service
Closed De	oression	te Highways Coordinate System: Web Mercator (EPSG:3857)
Gravel Pit	US Rou	
Gravelly S		projection, which preserves direction and shape but distorts
🔕 Landfill	Local R	distance and area. A projection that preserves area, such as the
Lava Flow	Background	accurate calculations of distance or area are required.
Marsh or s	-	hotography This product is generated from the USDA-NRCS certified data a of the version date(s) listed below.
Mine or Q	arry	
Miscellane	ous Water	Soil Survey Area: Multnomah County Area, Oregon Survey Area Data: Version 20, Oct 27, 2021
Perennial	Vater	Soil map units are labeled (as space allows) for map scales
V Rock Outo	rop	1:50,000 or larger.
🕂 🛛 Saline Spo	t	Date(s) aerial images were photographed: Jul 2, 2015—Sep 2 2016
Sandy Spo	t	The orthophoto or other base map on which the soil lines were
Severely E	roded Spot	compiled and digitized probably differs from the background
Sinkhole		imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.
Slide or Sl	р	
💋 Sodic Spo		



Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
9B	Cazadero silty clay loam, 0 to 8 percent slopes	13.6	97.0%
20F	Haplumbrepts, very steep	0.4	3.0%
Totals for Area of Interest		14.0	100.0%

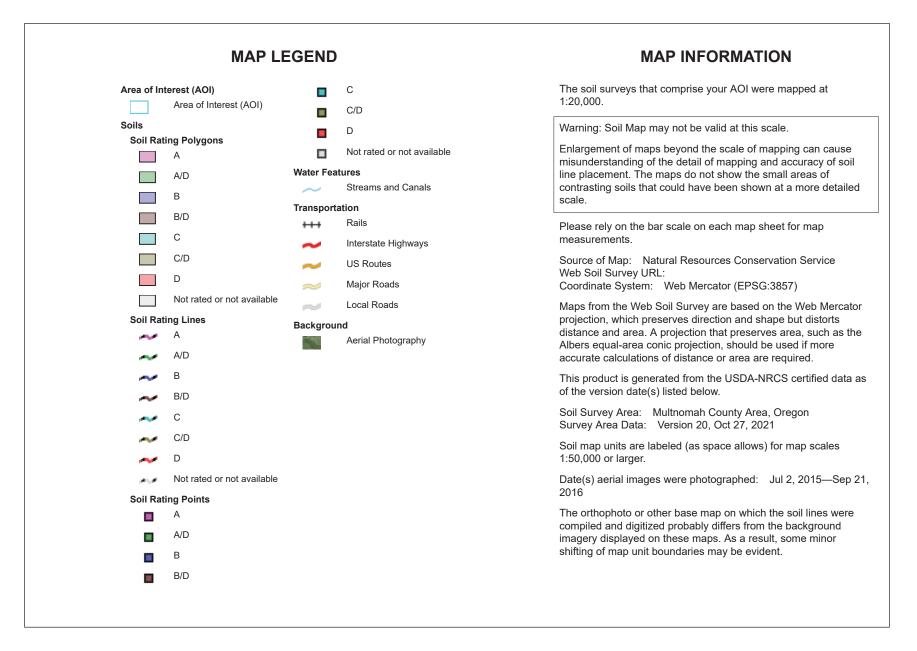




USDA Natural Resources

Conservation Service

Web Soil Survey National Cooperative Soil Survey 2/12/2022 Page 1 of 4



Hydrologic Soil Group

	-	1	1	
Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
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20F	Haplumbrepts, very steep	В	0.4	3.0%
Totals for Area of Intere	est		14.0	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

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USDA

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher





Web Soil Survey National Cooperative Soil Survey

N	AP LEGEND	MAP INFORMATION
Area of Interest (AOI)	Spoil Area	The soil surveys that comprise your AOI were mapped at
Area of Interest	AOI) 👔 Stony Spot	1:20,000.
Soils	Very Stony Spot	Warning: Soil Map may not be valid at this scale.
Soil Map Unit Po	ygons 🖤 Wet Spot	Enlargement of maps beyond the scale of mapping can cause
soil Map Unit Lir	es 🖌 Other	misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of
Soil Map Unit Po	nts Special Line Features	contrasting soils that could have been shown at a more detaile
Special Point Features	Water Features	scale.
Blowout	Streams and Canals	Please rely on the bar scale on each map sheet for map
Borrow Pit	Transportation	measurements.
💥 🛛 Clay Spot	Rails	Source of Map: Natural Resources Conservation Service
Closed Depression		Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)
Gravel Pit	US Routes	Maps from the Web Soil Survey are based on the Web Mercate
Gravelly Spot	Major Roads	projection, which preserves direction and shape but distorts
🚳 Landfill	Local Roads	distance and area. A projection that preserves area, such as th Albers equal-area conic projection, should be used if more
Lava Flow	Background	accurate calculations of distance or area are required.
Marsh or swamp	Aerial Photography	This product is generated from the USDA-NRCS certified data
Mine or Quarry	-	of the version date(s) listed below.
Miscellaneous V	ater	Soil Survey Area: Clackamas County Area, Oregon Survey Area Data: Version 18, Oct 27, 2021
Perennial Water		Soil Survey Area: Multnomah County Area, Oregon
Rock Outcrop		Survey Area Data: Version 20, Oct 27, 2021
Saline Spot		Your area of interest (AOI) includes more than one soil survey
Sandy Spot		area. These survey areas may have been mapped at different scales, with a different land use in mind, at different times, or a
Severely Erodec	Snot	different levels of detail. This may result in map unit symbols, s
		properties, and interpretations that do not completely agree across soil survey area boundaries.
*		Soil map units are labeled (as space allows) for map scales
100		1:50,000 or larger.
ø Sodic Spot		Date(s) aerial images were photographed: Jul 2, 2015—Sep 2016

MAP LEGEND

MAP INFORMATION

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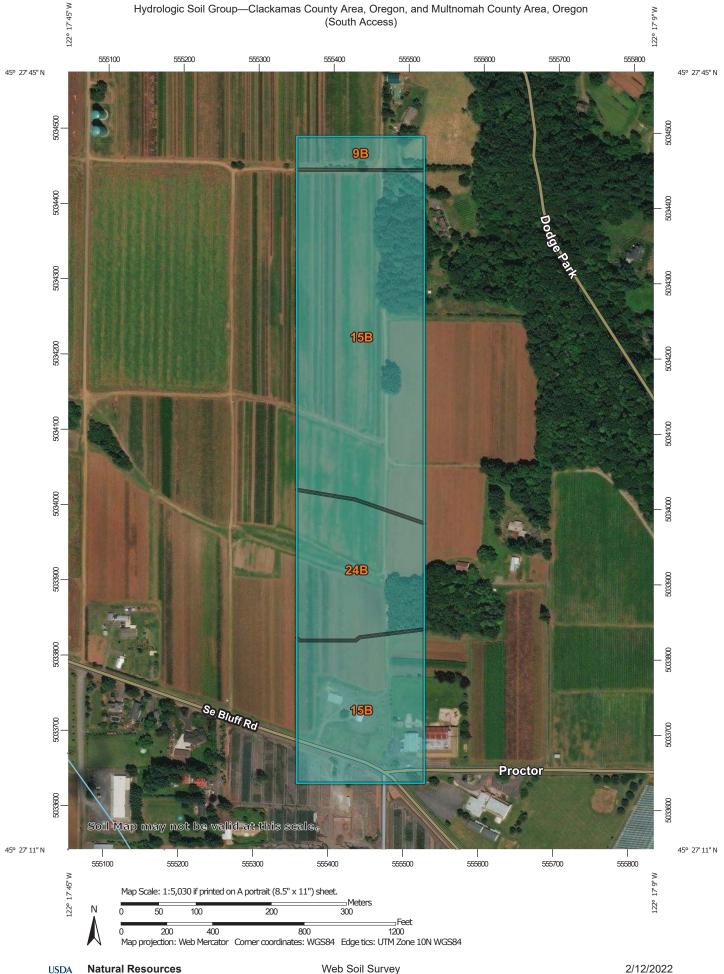


Map Unit Legend

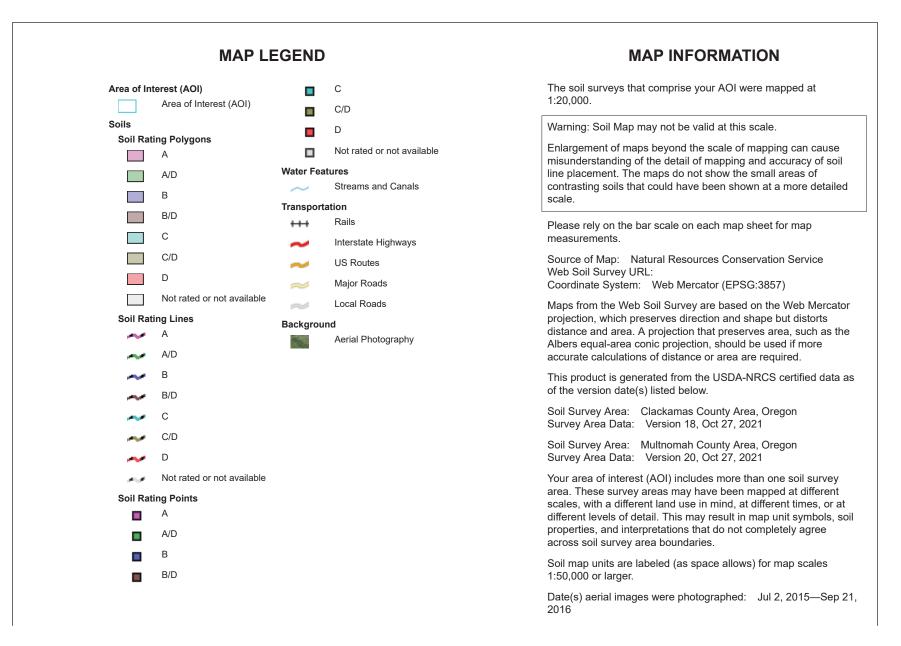
	1		
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
15B	Cazadero silty clay loam, 0 to 7 percent slopes	26.9	74.2%
24B	Cottrell silty clay loam, 2 to 8 percent slopes	7.5	20.6%
Subtotals for Soil Survey Area	1	34.4	94.8%
Totals for Area of Interest		36.3	100.0%

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
9B	Cazadero silty clay loam, 0 to 8 percent slopes	1.9	5.2%
Subtotals for Soil Survey Area	1	1.9	5.2%
Totals for Area of Interest		36.3	100.0%





Web Soil Survey National Cooperative Soil Survey



MAP LEGEND

MAP INFORMATION

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Hydrologic Soil Group

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Subtotals for Soil Surve	ey Area		34.4	94.8%
Totals for Area of Interest		36.3	100.0%	

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
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Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher



Table 2-2aRunoff curve numbers for urban areas 1/

Cover description			Curve nu -hydrologic	umbers for soil group	
-	verage percent		• 0	0.1	
	pervious area $\frac{2}{2}$	А	В	С	D
Fully developed urban areas (vegetation established)					
Dpen space (lawns, parks, golf courses, cemeteries, etc.) ³ /:					
Poor condition (grass cover < 50%)		68	79	86	89
Fair condition (grass cover 50% to 75%)		49	69	79	84
Good condition (grass cover > 75%)		39	61	74	80
Impervious areas:	••	00	01		00
Paved parking lots, roofs, driveways, etc.					
(excluding right-of-way)		98	98	98	98
Streets and roads:	••	00	00	00	50
Paved; curbs and storm sewers (excluding					
right-of-way)		98	98	98	98
Paved; open ditches (including right-of-way)		83	89	92	93
Gravel (including right-of-way)		76	85	89	91
Dirt (including right-of-way)		70 72	82	87	89
Western desert urban areas:	••	12	02	01	00
Natural desert landscaping (pervious areas only) 4/		63	77	85	88
Artificial desert landscaping (impervious areas only) =		00		00	00
desert shrub with 1- to 2-inch sand or gravel mulch					
and basin borders)		96	96	96	96
Urban districts:	••	50	50	50	50
Commercial and business	85	89	92	94	95
Industrial		81	88	91	93
Residential districts by average lot size:	12	01	00	01	55
1/8 acre or less (town houses)	65	77	85	90	92
1/4 acre		61	75	83	87
1/3 acre		57	72	81	86
1/2 acre		54	70	80	85
1 acre		51	68	79	84
2 acres		46	65	77	82
	. 14	10	00		02
Developing urban areas					
Newly graded areas					
(pervious areas only, no vegetation) ^{5/}		77	86	91	94
Idle lands (CN's are determined using cover types					

similar to those in table 2-2c). ¹ Average runoff condition, and $I_a = 0.2S$.

² The average percent impervious area shown was used to develop the composite CN's. Other assumptions are as follows: impervious areas are directly connected to the drainage system, impervious areas have a CN of 98, and pervious areas are considered equivalent to open space in good hydrologic condition. CN's for other combinations of conditions may be computed using figure 2-3 or 2-4.

³ CN's shown are equivalent to those of pasture. Composite CN's may be computed for other combinations of open space

⁴ Composite CN's for natural desert landscaping should be computed using figures 2-3 or 2-4 based on the impervious area percentage (CN = 98) and the pervious area CN. The pervious area CN's are assumed equivalent to desert shrub in poor hydrologic condition.

⁵ Composite CN's to use for the design of temporary measures during grading and construction should be computed using figure 2-3 or 2-4 based on the degree of development (impervious area percentage) and the CN's for the newly graded pervious areas.

cover type.

Table 2-2bRunoff curve numbers for cultivated agricultural lands 1/2

	Cover description		Curve numbers for hydrologic soil group			
		Hydrologic		119 01 01 0 81 0 5	on Sroup	
Cover type	Treatment 2/	condition 3/	А	В	С	D
Fallow	Bare soil	_	77	86	91	94
	Crop residue cover (CR)	Poor	76	85	90	93
		Good	74	83	88	90
Row crops	Straight row (SR)	Poor	72	81	88	91
		Good	67	78	85	89
	SR + CR	Poor	71	80	87	90
		Good	64	75	<mark>82</mark>	85
	Contoured (C)	Poor	70	79	84	88
		Good	65	75	82	86
	C + CR	Poor	69	78	83	87
		Good	64	74	81	85
	Contoured & terraced (C&T)	Poor	66	74	80	82
		Good	62	71	78	81
	C&T+ CR	Poor	65	73	79	81
		Good	61	70	77	80
Small grain	SR	Poor	65	76	84	88
		Good	63	75	83	87
	SR + CR	Poor	64	75	83	86
		Good	60	72	80	84
	С	Poor	63	74	82	85
		Good	61	73	81	84
	C + CR	Poor	62	73	81	84
		Good	60	72	80	83
	C&T	Poor	61	72	79	82
		Good	59	70	78	81
	C&T+ CR	Poor	60	71	78	81
		Good	58	69	77	80
Close-seeded	SR	Poor	66	77	85	89
or broadcast		Good	58	72	81	85
legumes or	С	Poor	64	75	83	85
rotation		Good	55	69	78	83
meadow	C&T	Poor	63	73	80	83
		Good	51	67	76	80

 $^{\rm 1}$ Average runoff condition, and $\rm I_a{=}0.2S$

² Crop residue cover applies only if residue is on at least 5% of the surface throughout the year.

³ Hydraulic condition is based on combination factors that affect infiltration and runoff, including (a) density and canopy of vegetative areas, (b) amount of year-round cover, (c) amount of grass or close-seeded legumes, (d) percent of residue cover on the land surface (good \geq 20%), and (e) degree of surface roughness.

Poor: Factors impair infiltration and tend to increase runoff.

Good: Factors encourage average and better than average infiltration and tend to decrease runoff.

Table 2-2cRunoff curve numbers for other agricultural lands

Cover description		Curve numbers for hydrologic soil group					
Cover type	Hydrologic condition	А	В	C	D		
Pasture, grassland, or range—continuous	Poor	68	79	86	89		
forage for grazing. ^{2/}	Fair	49	69	79	84		
	Good	39	61	74	80		
Meadow—continuous grass, protected from grazing and generally mowed for hay.	—	30	58	71	78		
Brush—brush-weed-grass mixture with brush	Poor	48	67	77	83		
the major element. 3/	Fair	35	56	70	77		
	Good	30 4/	48	65	73		
Woods—grass combination (orchard	Poor	57	73	82	86		
or tree farm). 5/	Fair	43	65	76	82		
	Good	32	58	72	<mark>79</mark>		
Woods. 6/	Poor	45	66	77	83		
	Fair	36	60	73	79		
	Good	30 4/	55	70	77		
Farmsteads—buildings, lanes, driveways, and surrounding lots.	—	59	74	82	86		

¹ Average runoff condition, and $I_a = 0.2S$.

² *Poor:* <50%) ground cover or heavily grazed with no mulch.

Fair: 50 to 75% ground cover and not heavily grazed.

Good: > 75% ground cover and lightly or only occasionally grazed.

Poor: <50% ground cover.

3

Fair: 50 to 75% ground cover.

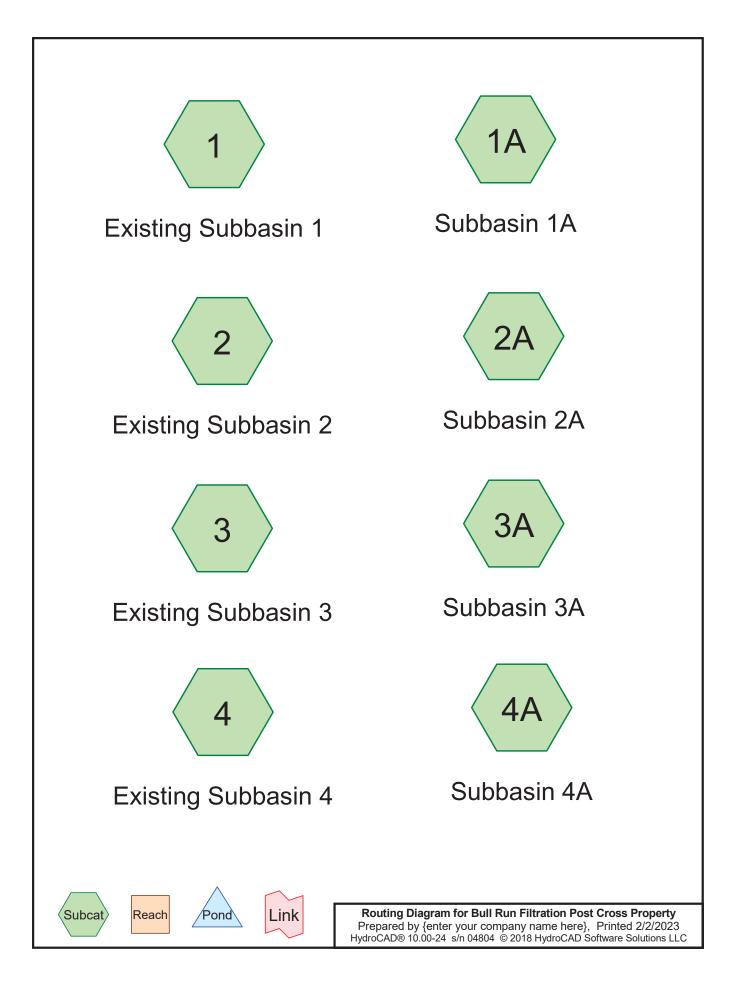
Good: >75% ground cover.

 4 $\,$ Actual curve number is less than 30; use CN = 30 for runoff computations.

⁵ CN's shown were computed for areas with 50% woods and 50% grass (pasture) cover. Other combinations of conditions may be computed from the CN's for woods and pasture.

⁶ *Poor:* Forest litter, small trees, and brush are destroyed by heavy grazing or regular burning. *Fair:* Woods are grazed but not burned, and some forest litter covers the soil. *Good:* Woods are protected from grazing, and litter and brush adequately cover the soil.

Attachment C: Hydrologic Analysis of Pre- and Post-Developed Conditions Cross-Property Flows



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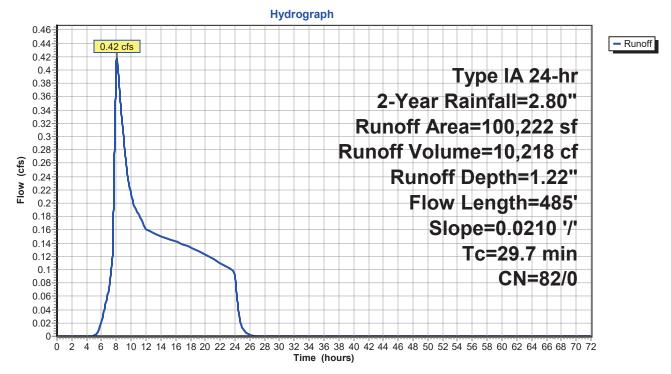
Summary for Subcatchment 1: Existing Subbasin 1

Runoff = 0.42 cfs @ 8.06 hrs, Volume= 10,218 cf, Depth= 1.22"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

	Area (sf)	CN [Description		
	100,222	82 F	Row crops,	SR + CR,	Good, HSG C
	100,222 82 100.00% Pervious Area			ervious Are	a
To (min)		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.3	300	0.0210	0.18		Sheet Flow,
2.4	185	0.0210	1.30		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
29.7	′	Total			

Subcatchment 1: Existing Subbasin 1



Bull Run Filtration Post Cross Property

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Summary for Subcatchment 1A: Subbasin 1A

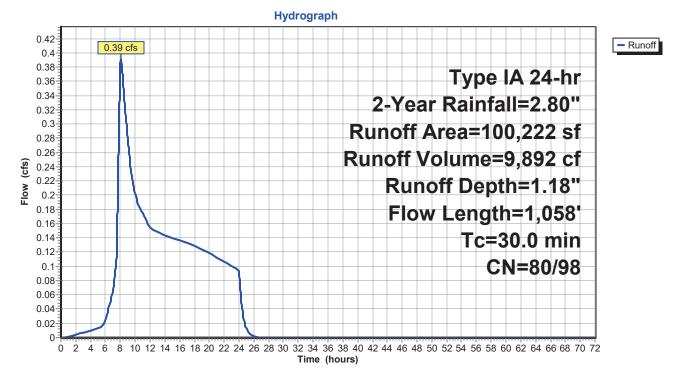
Runoff = 0.39 cfs @ 8.07 hrs, Volume= 9,892 cf, Depth= 1.18"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

	A	rea (sf)	CN	Description		
		94,598	80	>75% Gras	s cover, Go	ood, HSG D
*		5,624	98	Impervious		
	1	00,222	81	Weighted A	verage	
		94,598	80	94.39% Pei	rvious Area	
		5,624	98	5.61% Impe	ervious Area	a
	_					
	Tc	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	24.3	300	0.0220	0.21		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.80"
	2.9	182	0.0220	1.04		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	2.8	576	0.0222	3.38	20.28	Channel Flow,
						Area= 6.0 sf Perim= 9.0' r= 0.67'
_						n= 0.050 Scattered brush, heavy weeds
	20.0	1 050	Total			

30.0 1,058 Total

Subcatchment 1A: Subbasin 1A



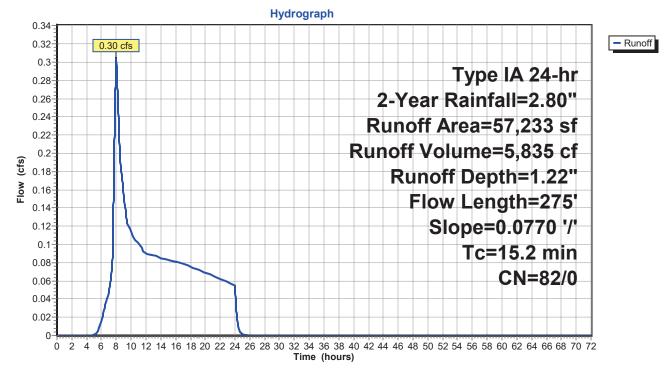
Summary for Subcatchment 2: Existing Subbasin 2

Runoff = 0.30 cfs @ 8.00 hrs, Volume= 5,835 cf, Depth= 1.22"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

Area (sf)	CN	Description						
57,233	82 Row crops, SR + CR, Good, HSG C							
57,233 82 100.00% Pervious Area								
Tc Length (min) (feet)	Slope (ft/ft)		Capacity (cfs)	Description				
15.2 275	0.0770	0.30		Sheet Flow, Cultivated: Residue>20%	n= 0.170	P2= 2.80"		

Subcatchment 2: Existing Subbasin 2



Bull Run Filtration Post Cross Property

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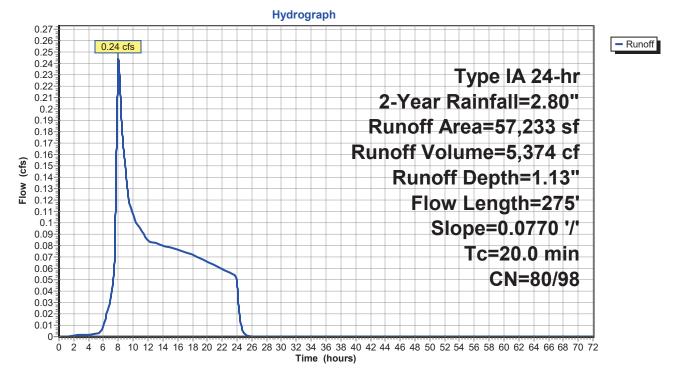
Summary for Subcatchment 2A: Subbasin 2A

Runoff = 0.24 cfs @ 8.01 hrs, Volume= 5,374 cf, Depth= 1.13"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

_	A	rea (sf)	CN	Description					
		56,269	80	>75% Gras	75% Grass cover, Good, HSG D				
*		964	98	Impervious	mpervious				
		57,233	80	Weighted A	verage				
		56,269	80	98.32% Pe	rvious Area				
		964	98	1.68% Impe	ervious Area	а			
	Тс	Length	Slop	,	Capacity	Description			
_	(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)				
	20.0	275	0.077	0 0.23		Sheet Flow,			
						Grass: Dense	n= 0.240	P2= 2.80"	

Subcatchment 2A: Subbasin 2A



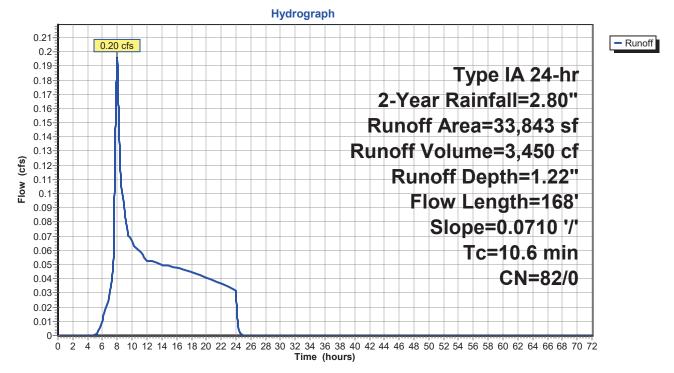
Summary for Subcatchment 3: Existing Subbasin 3

Runoff = 0.20 cfs @ 8.00 hrs, Volume= 3,450 cf, Depth= 1.22"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

Area (sf)	CN	Description						
33,843	82	32 Row crops, SR + CR, Good, HSG C						
33,843	3 82 100.00% Pervious Area							
Tc Length (min) (feet)	Slop (ft/f		Capacity (cfs)	Description				
10.6 168	0.071	0 0.27		Sheet Flow, Cultivated: Residue>20%	n= 0.170	P2= 2.80"		

Subcatchment 3: Existing Subbasin 3



Bull Run Filtration Post Cross Property

Type IA 24-hr 2-Year Rainfall=2.80" Printed 2/2/2023

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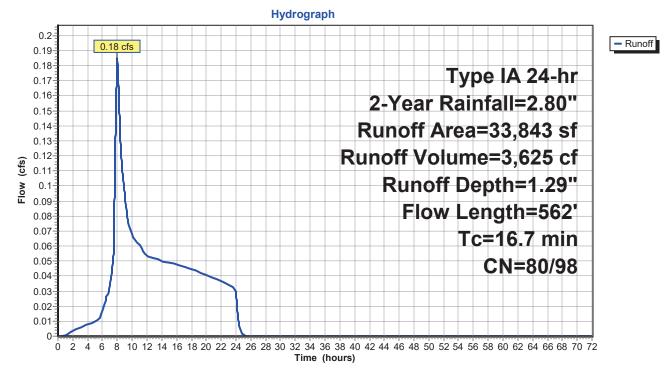
Summary for Subcatchment 3A: Subbasin 3A

3,625 cf, Depth= 1.29" Runoff = 0.18 cfs @ 8.01 hrs, Volume=

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

	A	rea (sf)	CN [Description					
		29,613	80 >	0 >75% Grass cover, Good, HSG D					
*		4,230	98 I	Impervious Area					
		33,843	82 \	Neighted A	verage				
		29,613	80 8	37.50% Per	vious Area				
		4,230	98 ´	12.50% Imp	pervious Ar	ea			
	Тс	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	12.2	151	0.0790	0.21		Sheet Flow,			
						Grass: Dense n= 0.240 P2= 2.80"			
	4.5	411	0.0046	1.54	9.23	Channel Flow,			
						Area= 6.0 sf Perim= 9.0' r= 0.67'			
_						n= 0.050 Scattered brush, heavy weeds			
	16.7	562	Total						

Subcatchment 3A: Subbasin 3A



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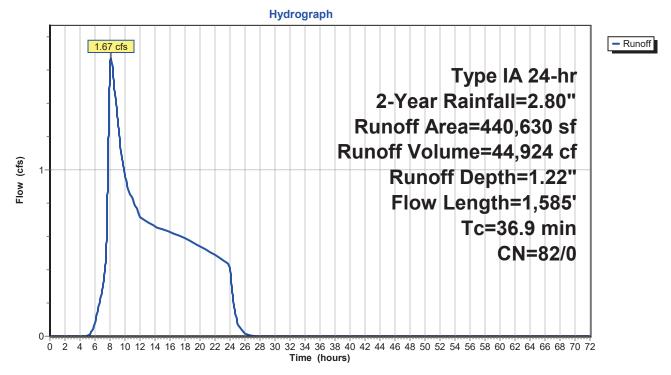
Summary for Subcatchment 4: Existing Subbasin 4

Runoff = 1.67 cfs @ 8.14 hrs, Volume= 44,924 cf, Depth= 1.22"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

_	Area (sf) CN Description					
	440,630 82 Row crops, SR + CR, G					Good, HSG C
	440,630 82 100.00% Pervious A			100.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description
-	27.9	300	0.0200	0.18		Sheet Flow,
	9.0	1,285	0.0700	2.38		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
-	36.9	1,585	Total			

Subcatchment 4: Existing Subbasin 4



Bull Run Filtration Post Cross Property

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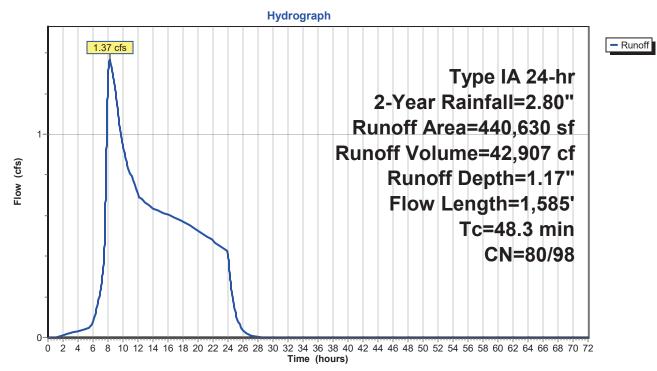
Summary for Subcatchment 4A: Subbasin 4A

Runoff = 1.37 cfs @ 8.24 hrs, Volume= 42,907 cf, Depth= 1.17"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

_	A	rea (sf)	CN [Description					
	4	20,676	80 >	>75% Grass cover, Good, HSG D					
*		19,954	98 I	Impervious					
440,630 81 Weighted Average					verage				
420,676 80 95.47% Pervious Area					vious Area				
19,954 98 4.53% Impervious Area			.53% Impe	ervious Area	а				
	Тс	Length	Slope		Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	36.7	300	0.0200	0.14		Sheet Flow,			
						Grass: Dense n= 0.240 P2= 2.80"			
	11.6	1,285	0.0700	1.85		Shallow Concentrated Flow,			
_						Short Grass Pasture Kv= 7.0 fps			
	48.3	1,585	Total						

Subcatchment 4A: Subbasin 4A



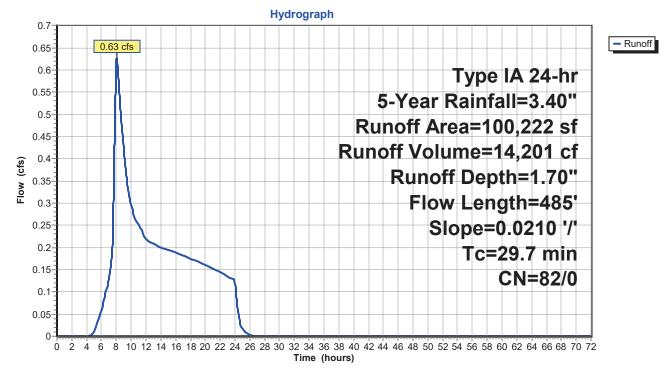
Summary for Subcatchment 1: Existing Subbasin 1

Runoff = 0.63 cfs @ 8.03 hrs, Volume= 14,201 cf, Depth= 1.70"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

_	Area (sf) CN Description					
	1	00,222	82 F	Row crops,	SR + CR,	Good, HSG C
-	1	00,222	82 1	100.00% Pe	ervious Are	a
	Tc Length (min) (feet)		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	27.3	300	0.0210	0.18		Sheet Flow,
	2.4	185	0.0210	1.30		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
	29.7	485	Total			

Subcatchment 1: Existing Subbasin 1



Bull Run Filtration Post Cross Property

Summary for Subcatchment 1A: Subbasin 1A

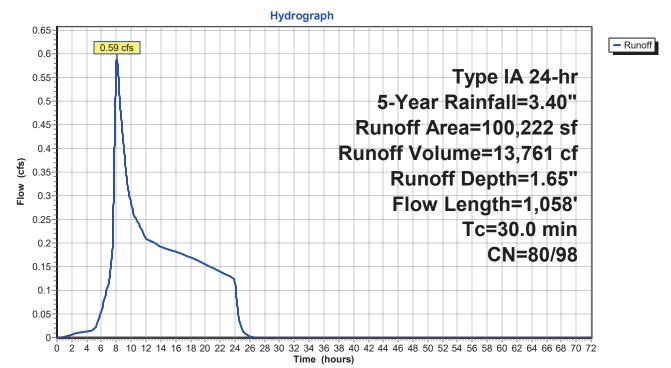
Runoff = 0.59 cfs @ 8.04 hrs, Volume= 13,761 cf, Depth= 1.65"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

	A	rea (sf)	CN	Description		
		94,598	80	>75% Gras	s cover, Go	ood, HSG D
*		5,624	98	Impervious		
	1	00,222	81	Weighted A	verage	
		94,598	80	94.39% Pei	rvious Area	
		5,624	98	5.61% Impe	ervious Area	a
	Tc	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
	24.3	300	0.0220	0.21		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.80"
	2.9	182	0.0220) 1.04		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	2.8	576	0.0222	2 3.38	20.28	Channel Flow,
						Area= 6.0 sf Perim= 9.0' r= 0.67'
						n= 0.050 Scattered brush, heavy weeds
	20 0	1 050	Total			

30.0 1,058 Total

Subcatchment 1A: Subbasin 1A



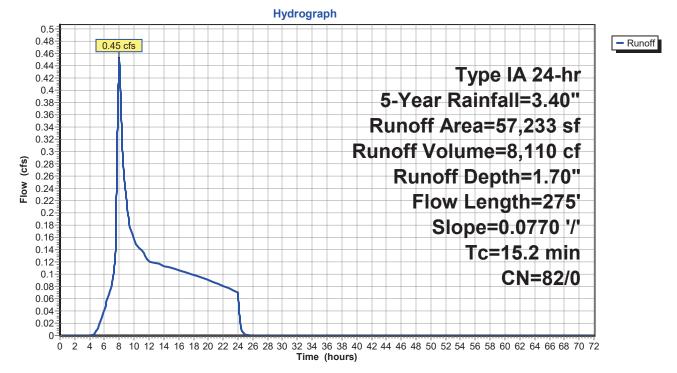
Summary for Subcatchment 2: Existing Subbasin 2

Runoff = 0.45 cfs @ 8.00 hrs, Volume= 8,110 cf, Depth= 1.70"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

Area (sf)	CN	Description						
57,233	82	82 Row crops, SR + CR, Good, HSG C						
57,233	82	82 100.00% Pervious Area						
Tc Length (min) (feet)	Slope (ft/ft		Capacity (cfs)	Description				
15.2 275	0.0770	0.30		Sheet Flow, Cultivated: Residue>20%	n= 0.170	P2= 2.80"		

Subcatchment 2: Existing Subbasin 2



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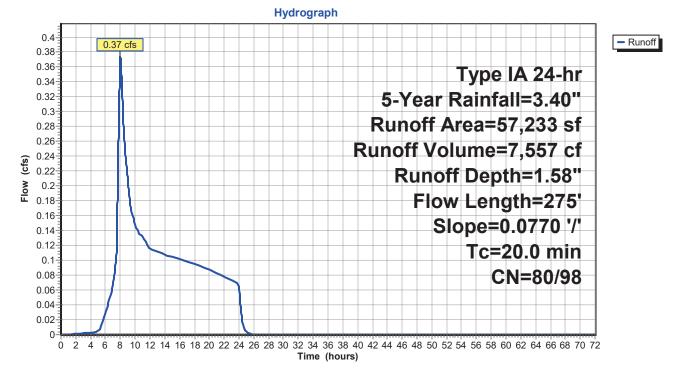
Summary for Subcatchment 2A: Subbasin 2A

Runoff = 0.37 cfs @ 8.01 hrs, Volume= 7,557 cf, Depth= 1.58"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

_	A	rea (sf)	CN	Desc	Description						
		56,269	80	>75%	75% Grass cover, Good, HSG D						
*		964	98	Impe	ervious						
		57,233	80	Weig	Weighted Average						
		56,269	80	98.3	2% Per						
		964	98	1.68	% Impe	ervious Area	а				
	Tc	Length	Slop		elocity	Capacity	Description				
_	(min)	(feet)	(ft/f	t) (1	ft/sec)	(cfs)					
	20.0	275	0.077	0	0.23		Sheet Flow,				
							Grass: Dense	n= 0.240	P2= 2.80"		





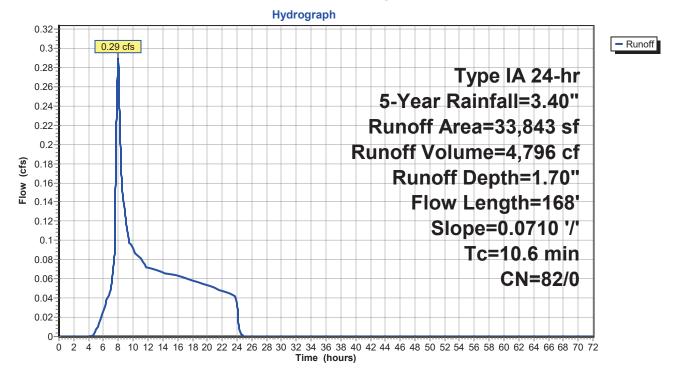
Summary for Subcatchment 3: Existing Subbasin 3

Runoff = 0.29 cfs @ 8.00 hrs, Volume= 4,796 cf, Depth= 1.70"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

A	rea (sf)	CN	Description						
	33,843	82	2 Row crops, SR + CR, Good, HSG C						
	33,843	82	82 100.00% Pervious Area						
Tc _(min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description				
10.6	168	0.0710	0.27		Sheet Flow, Cultivated: Residue>20%	n= 0.170	P2= 2.80"		

Subcatchment 3: Existing Subbasin 3



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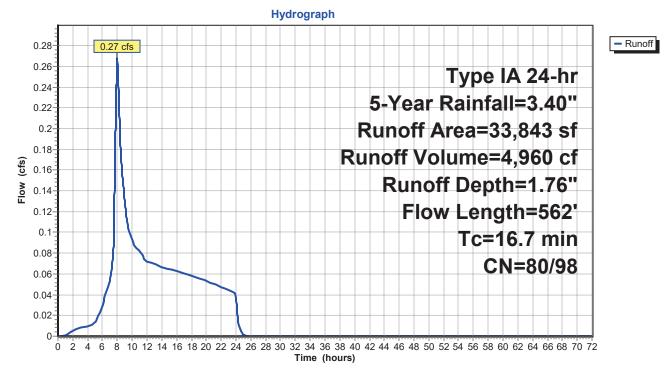
Summary for Subcatchment 3A: Subbasin 3A

Runoff = 0.27 cfs @ 8.00 hrs, Volume= 4,960 cf, Depth= 1.76"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

_	A	rea (sf)	CN	Description							
		29,613	80	>75% Grass cover, Good, HSG D							
*		4,230	98	98 Impervious Area							
		33,843 82 Weighted Average									
		29,613	80	87.50% Per	vious Area						
		4,230	98	12.50% Imp	pervious Ar	ea					
	Тс	Length	Slope		Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	12.2	151	0.0790	0.21		Sheet Flow,					
						Grass: Dense n= 0.240 P2= 2.80"					
	4.5	411	0.0046	1.54	9.23	Channel Flow,					
						Area= 6.0 sf Perim= 9.0' r= 0.67'					
_						n= 0.050 Scattered brush, heavy weeds					
	16.7	562	Total								

Subcatchment 3A: Subbasin 3A



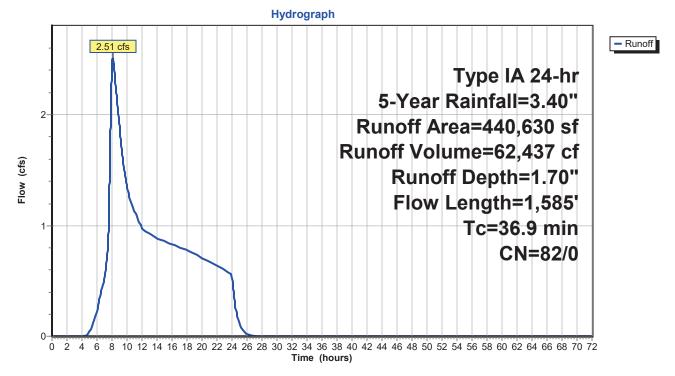
Summary for Subcatchment 4: Existing Subbasin 4

Runoff = 2.51 cfs @ 8.10 hrs, Volume= 62,437 cf, Depth= 1.70"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

_	A	rea (sf)	CN I	Description					
	4	40,630	82 I	Row crops,	SR + CR,	Good, HSG C			
	4	40,630	82 ⁻	100.00% Pervious Area					
	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description			
-	27.9	300	0.0200	0.18		Sheet Flow,			
	9.0	1,285	0.0700	2.38		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps			
	36.9	1,585	Total						

Subcatchment 4: Existing Subbasin 4



Type IA 24-hr 5-Year Rainfall=3.40" Printed 2/2/2023 Page 17

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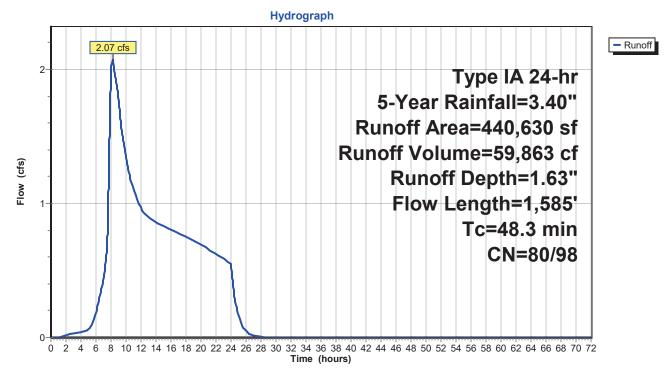
Summary for Subcatchment 4A: Subbasin 4A

8.20 hrs, Volume= 59,863 cf, Depth= 1.63" Runoff = 2.07 cfs @

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

_	A	rea (sf)	CN E	Description							
	4	20,676	80 >	75% Gras	5% Grass cover, Good, HSG D						
*		19,954	98 I	mpervious							
	4	40,630	81 V	Veighted A	verage						
	4	20,676	80 9	5.47% Per	vious Area						
		а									
	Tc	Length	Slope		Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	36.7	300	0.0200	0.14		Sheet Flow,					
						Grass: Dense n= 0.240 P2= 2.80"					
	11.6	1,285	0.0700	1.85		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	48.3	1,585	Total								

Subcatchment 4A: Subbasin 4A



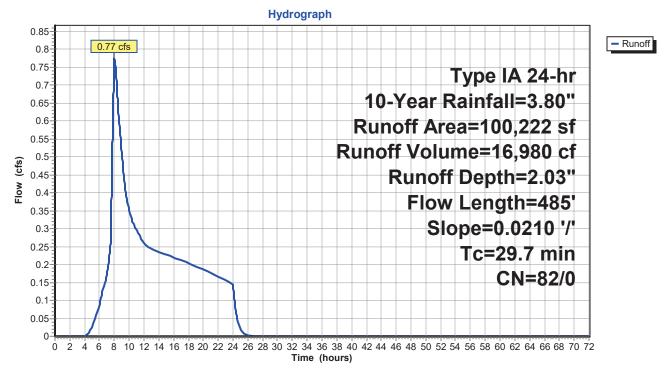
Summary for Subcatchment 1: Existing Subbasin 1

Runoff = 0.77 cfs @ 8.02 hrs, Volume= 16,980 cf, Depth= 2.03"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

_	A	rea (sf)	CN [Description		
	1	00,222	82 F	Row crops,	SR + CR,	Good, HSG C
	1	00,222	82 1	100.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	27.3	300	0.0210	0.18	· · ·	Sheet Flow,
	2.4	185	0.0210	1.30		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
_	29.7	485	Total			

Subcatchment 1: Existing Subbasin 1



 Type IA 24-hr
 10-Year Rainfall=3.80"

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Summary for Subcatchment 1A: Subbasin 1A

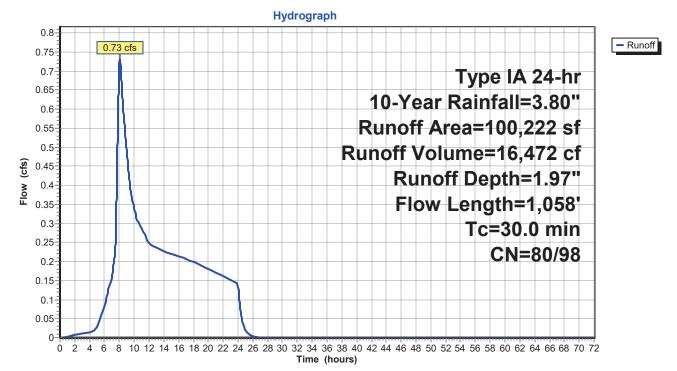
Runoff = 0.73 cfs @ 8.03 hrs, Volume= 16,472 cf, Depth= 1.97"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

	A	rea (sf)	CN	Description						
		94,598	80	>75% Gras	s cover, Go	ood, HSG D				
*		5,624	98 Impervious							
	1	00,222	81	Weighted A	verage					
		94,598	80	94.39% Pei	rvious Area					
		5,624	98	5.61% Impe	ervious Area	a				
	_									
	Tc	Length	Slope		Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	24.3	300	0.0220	0.21		Sheet Flow,				
						Grass: Short n= 0.150 P2= 2.80"				
	2.9	182	0.0220	1.04		Shallow Concentrated Flow,				
						Short Grass Pasture Kv= 7.0 fps				
	2.8	576	0.0222	3.38	20.28	Channel Flow,				
						Area= 6.0 sf Perim= 9.0' r= 0.67'				
_						n= 0.050 Scattered brush, heavy weeds				
	20.0	1 050	Total							

30.0 1,058 Total

Subcatchment 1A: Subbasin 1A



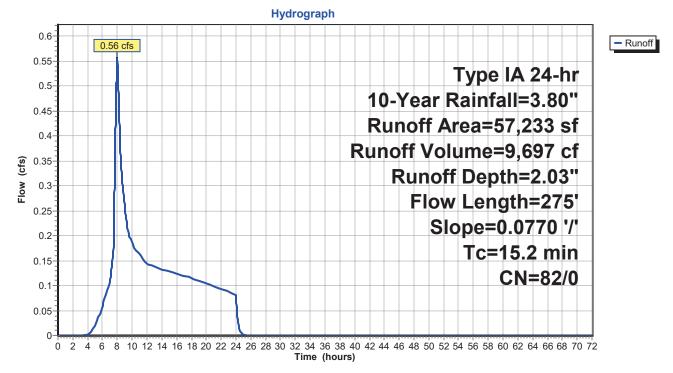
Summary for Subcatchment 2: Existing Subbasin 2

Runoff = 0.56 cfs @ 8.00 hrs, Volume= 9,697 cf, Depth= 2.03"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

Area (sf)	CN	Description							
57,233	82	2 Row crops, SR + CR, Good, HSG C							
57,233	82	82 100.00% Pervious Area							
Tc Length (min) (feet)	Slop (ft/ft		Capacity (cfs)	Description					
15.2 275	0.077	0 0.30		Sheet Flow, Cultivated: Residue>20%	n= 0.170	P2= 2.80"			

Subcatchment 2: Existing Subbasin 2



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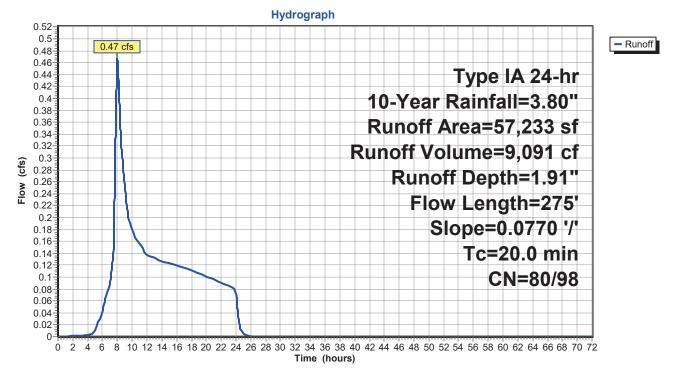
Summary for Subcatchment 2A: Subbasin 2A

Runoff = 0.47 cfs @ 8.01 hrs, Volume= 9,091 cf, Depth= 1.91"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

	A	rea (sf)	CN	Description							
		56,269	80	>75% Gras	75% Grass cover, Good, HSG D						
*		964	98	Impervious	Impervious						
		57,233	80	Weighted Average							
		56,269	80	98.32% Pe	rvious Area						
		964	98	1.68% Impe	ervious Area	а					
	Tc	Length	Slop		Capacity	Description					
_	(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)						
	20.0	275	0.077	0 0.23		Sheet Flow,					
						Grass: Dense	n= 0.240	P2= 2.80"			

Subcatchment 2A: Subbasin 2A



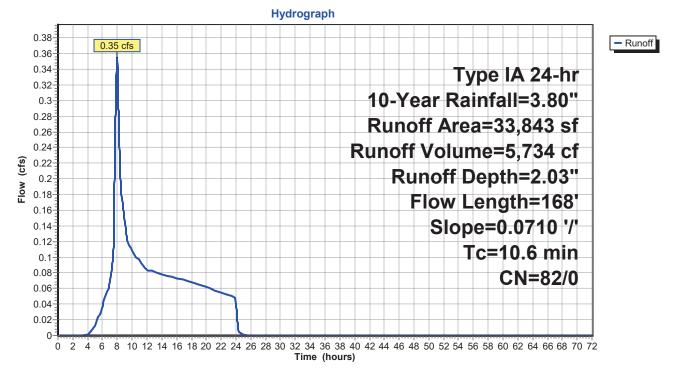
Summary for Subcatchment 3: Existing Subbasin 3

Runoff = 0.35 cfs @ 8.00 hrs, Volume= 5,734 cf, Depth= 2.03"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

Area (sf)	CN	Description							
33,843	82	82 Row crops, SR + CR, Good, HSG C							
33,843	82	82 100.00% Pervious Area							
Tc Length (min) (feet			Capacity (cfs)	Description					
10.6 168	0.071	0 0.27		Sheet Flow, Cultivated: Residue>20%	n= 0.170	P2= 2.80"			

Subcatchment 3: Existing Subbasin 3



 Type IA 24-hr
 10-Year Rainfall=3.80"

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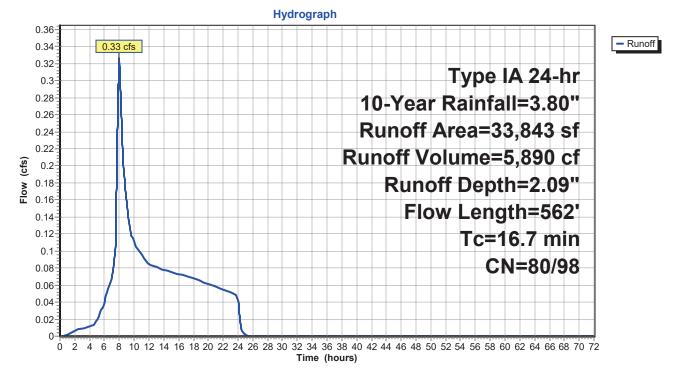
Summary for Subcatchment 3A: Subbasin 3A

Runoff = 0.33 cfs @ 8.00 hrs, Volume= 5,890 cf, Depth= 2.09"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

_	A	rea (sf)	CN	Description						
		29,613	80	>75% Grass cover, Good, HSG D						
*		4,230	98	8 Impervious Area						
		33,843	43 82 Weighted Average							
		29,613	80	87.50% Per	vious Area					
		4,230	98	12.50% Imp	pervious Ar	ea				
	Tc	Length	Slope		Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	12.2	151	0.0790	0.21		Sheet Flow,				
						Grass: Dense n= 0.240 P2= 2.80"				
	4.5	411	0.0046	1.54	9.23	Channel Flow,				
						Area= 6.0 sf Perim= 9.0' r= 0.67'				
_						n= 0.050 Scattered brush, heavy weeds				
	16.7	562	Total							

Subcatchment 3A: Subbasin 3A



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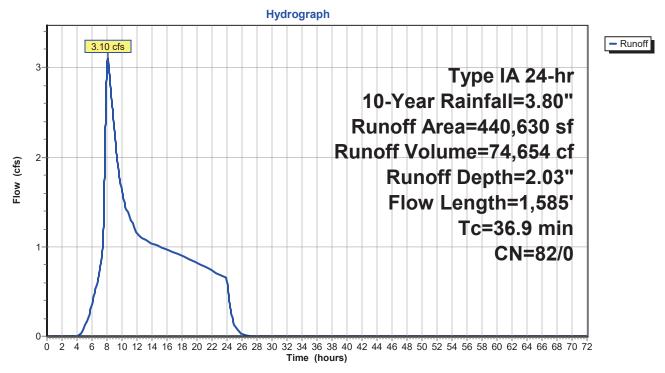
Summary for Subcatchment 4: Existing Subbasin 4

Runoff = 3.10 cfs @ 8.09 hrs, Volume= 74,654 cf, Depth= 2.03"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

_	A	rea (sf)	CN [Description						
	440,630 82 Row crops, SR + CR, Good, HSG C									
	4	40,630	82 1	100.00% Pe	ervious Are	a				
	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description				
-	27.9	300	0.0200	0.18		Sheet Flow,				
	9.0	1,285	0.0700	2.38		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps				
-	36.9	1,585	Total							

Subcatchment 4: Existing Subbasin 4



Type IA 24-hr 10-Year Rainfall=3.80" Printed 2/2/2023 Page 25

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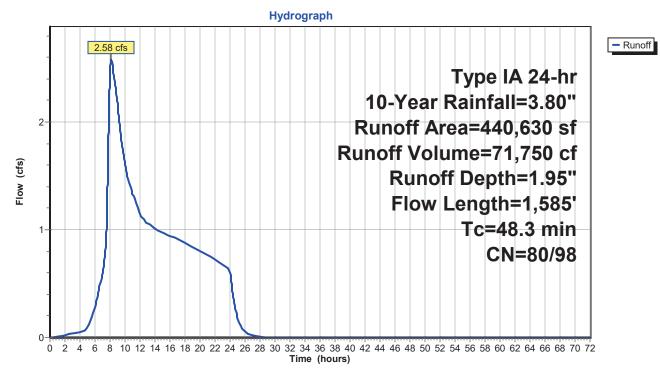
Summary for Subcatchment 4A: Subbasin 4A

8.19 hrs, Volume= 71,750 cf, Depth= 1.95" Runoff = 2.58 cfs @

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

_	A	rea (sf)	CN E	Description		
	4	20,676	80 >	75% Gras	s cover, Go	ood, HSG D
*		19,954	98 I	mpervious		
	4	40,630	81 V	Veighted A	verage	
	4	20,676	80 9	5.47% Per	vious Area	
		19,954	98 4	.53% Impe	ervious Area	а
	Тс	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	36.7	300	0.0200	0.14		Sheet Flow,
						Grass: Dense n= 0.240 P2= 2.80"
	11.6	1,285	0.0700	1.85		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	48.3	1,585	Total			

Subcatchment 4A: Subbasin 4A



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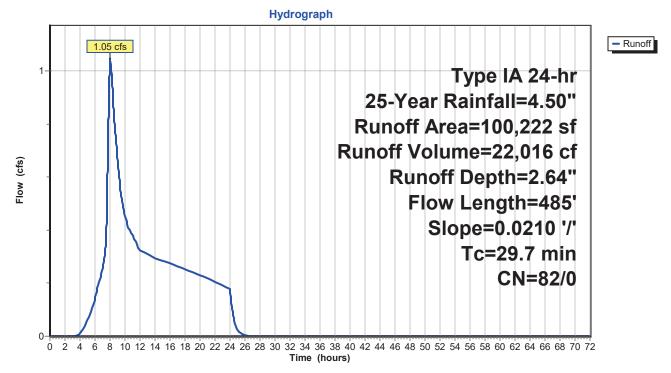
Summary for Subcatchment 1: Existing Subbasin 1

Runoff = 1.05 cfs @ 8.01 hrs, Volume= 22,016 cf, Depth= 2.64"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

_	A	rea (sf)	CN I	Description		
	1	00,222	82 I	Row crops,	SR + CR,	Good, HSG C
-	1	00,222	82 ⁻	100.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
-	27.3	300	0.0210	0.18		Sheet Flow,
_	2.4	185	0.0210	1.30		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
	29.7	485	Total			

Subcatchment 1: Existing Subbasin 1



 Type IA 24-hr
 25-Year Rainfall=4.50"

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Summary for Subcatchment 1A: Subbasin 1A

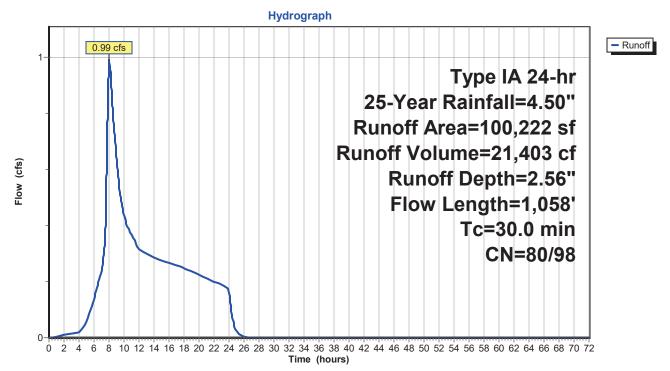
Runoff = 0.99 cfs @ 8.01 hrs, Volume= 21,403 cf, Depth= 2.56"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

	A	rea (sf)	CN	Description					
		94,598	80	>75% Gras	s cover, Go	ood, HSG D			
*		5,624	98	Impervious					
	1	00,222	81	Weighted A	verage				
		94,598	80	94.39% Pervious Area					
		5,624	98	5.61% Impe	ervious Area	a			
	Та	l e ve exte	Clana	Volocity	Conseitu	Description			
	Tc	Length	Slope		Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	24.3	300	0.0220	0.21		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.80"			
	2.9	182	0.0220	1.04		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	2.8	576	0.0222	3.38	20.28	Channel Flow,			
						Area= 6.0 sf Perim= 9.0' r= 0.67'			
_						n= 0.050 Scattered brush, heavy weeds			
	20.0	1 050	Total						

30.0 1,058 Total

Subcatchment 1A: Subbasin 1A



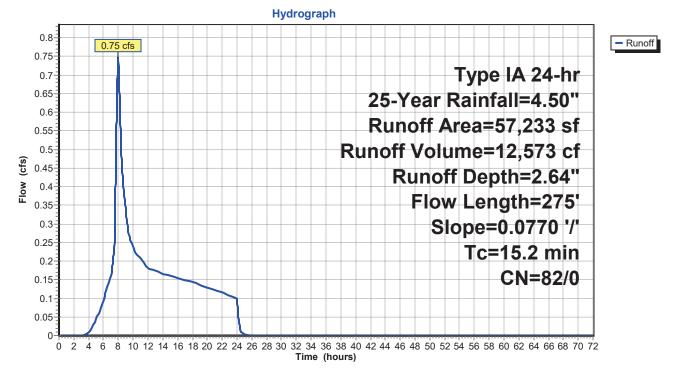
Summary for Subcatchment 2: Existing Subbasin 2

8.00 hrs, Volume= 12,573 cf, Depth= 2.64" Runoff = 0.75 cfs @

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

Area (sf)	CN	Description									
57,233	82	Row crops,	SR + CR, (Good, HSG C							
57,233	82	100.00% Pe	ervious Are	а							
Tc Length (min) (feet)	Slop (ft/ft		Capacity (cfs)	Description							
15.2 275	15.2 275 0.0770 0.30		Sheet Flow, Cultivated: Residue>20%	n= 0.170	P2= 2.80"						

Subcatchment 2: Existing Subbasin 2



Type IA 24-hr 25-Year Rainfall=4.50"

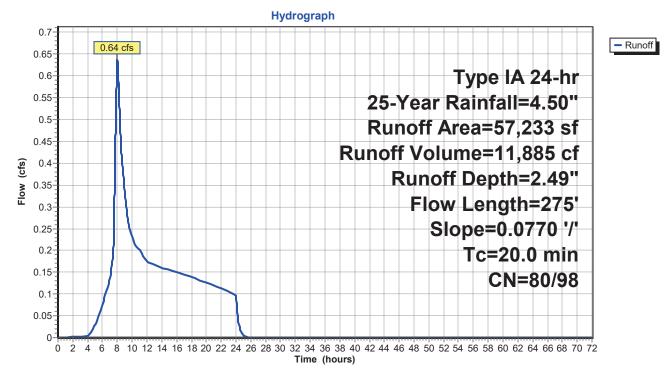
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Summary for Subcatchment 2A: Subbasin 2A

8.01 hrs, Volume= 11,885 cf, Depth= 2.49" Runoff = 0.64 cfs @

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

	A	rea (sf)	CN	Description					
		56,269	80	>75% Gras	s cover, Go	ood, HSG D			
*		964	98	Impervious					
		57,233	80	Weighted A	verage				
		56,269	80	98.32% Pe	rvious Area				
		964	98	1.68% Impe	ervious Area	а			
	Tc	Length	Slop		Capacity	Description			
_	(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)				
	20.0	275	0.077	0 0.23		Sheet Flow,			
						Grass: Dense	n= 0.240	P2= 2.80"	



Subcatchment 2A: Subbasin 2A

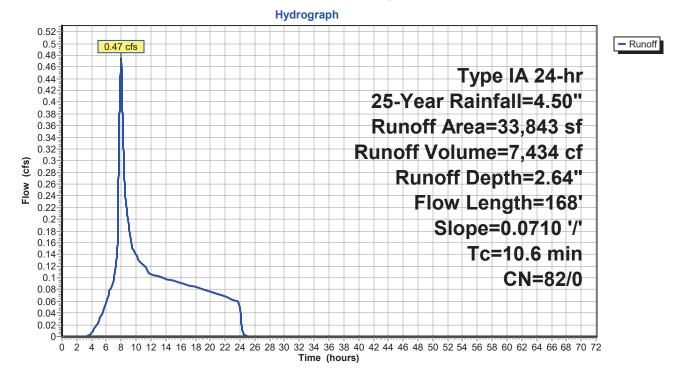
Summary for Subcatchment 3: Existing Subbasin 3

Runoff = 0.47 cfs @ 8.00 hrs, Volume= 7,434 cf, Depth= 2.64"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

Area (sf)	CN	Description									
33,843	82	2 Row crops, SR + CR, Good, HSG C									
33,843	82	100.00% Pe	ervious Are	а							
Tc Length (min) (feet)	Slope (ft/ft)		Capacity (cfs)	Description							
10.6 168	0.0710	0.27		Sheet Flow, Cultivated: Residue>20%	n= 0.170	P2= 2.80"					

Subcatchment 3: Existing Subbasin 3



 Type IA 24-hr
 25-Year Rainfall=4.50"

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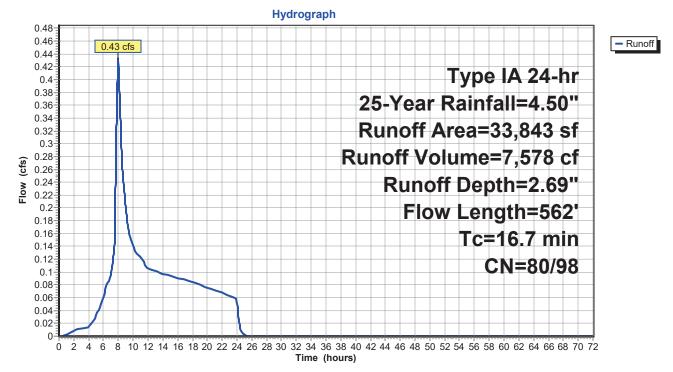
Summary for Subcatchment 3A: Subbasin 3A

Runoff = 0.43 cfs @ 8.00 hrs, Volume= 7,578 cf, Depth= 2.69"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

_	A	rea (sf)	CN I	Description									
		29,613	80 >	>75% Gras	s cover, Go	ood, HSG D							
*		4,230	98 I	mpervious	npervious Area								
		33,843	82 \	Neighted A	verage								
		29,613	80 8	37.50% Per	vious Area								
		4,230	98 ⁻	12.50% Imp	pervious Are	ea							
	Тс	Length	Slope	•	Capacity	Description							
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)								
	12.2	151	0.0790	0.21		Sheet Flow,							
						Grass: Dense n= 0.240 P2= 2.80"							
	4.5	411	0.0046	1.54	9.23	Channel Flow,							
						Area= 6.0 sf Perim= 9.0' r= 0.67'							
_						n= 0.050 Scattered brush, heavy weeds							
	16.7	562	Total										

Subcatchment 3A: Subbasin 3A



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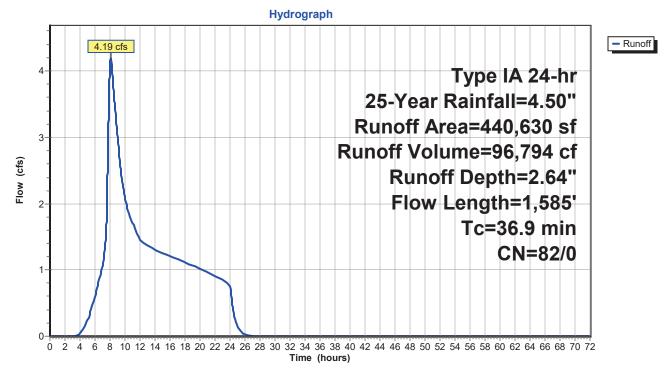
Summary for Subcatchment 4: Existing Subbasin 4

Runoff = 4.19 cfs @ 8.07 hrs, Volume= 96,794 cf, Depth= 2.64"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

A	rea (sf)	CN [Description		
4	40,630	82 F	Row crops,	SR + CR,	Good, HSG C
4	40,630	82 1	100.00% Pe	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.9	300	0.0200	0.18		Sheet Flow,
9.0	1,285	0.0700	2.38		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
36.9	1,585	Total			

Subcatchment 4: Existing Subbasin 4



Type IA 24-hr 25-Year Rainfall=4.50" Printed 2/2/2023 Page 33

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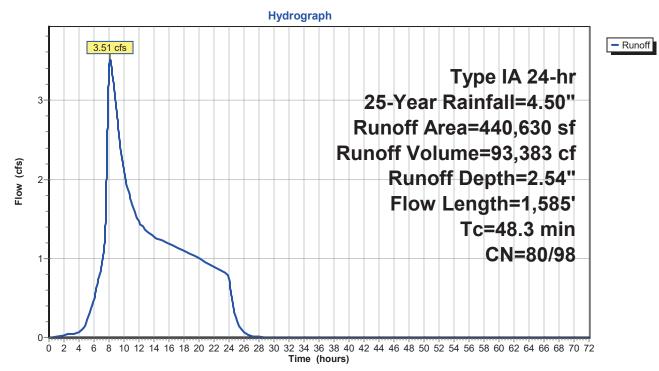
Summary for Subcatchment 4A: Subbasin 4A

8.16 hrs, Volume= 93,383 cf, Depth= 2.54" Runoff = 3.51 cfs @

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

_	A	rea (sf)	CN [Description		
	4	20,676	80 >	75% Gras	s cover, Go	ood, HSG D
*		19,954	98 I	mpervious		
	4	40,630	81 V	Veighted A	verage	
	4	20,676	80 9	5.47% Per	vious Area	
		19,954	98 4	.53% Impe	ervious Area	а
	Тс	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	36.7	300	0.0200	0.14		Sheet Flow,
						Grass: Dense n= 0.240 P2= 2.80"
	11.6	1,285	0.0700	1.85		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	48.3	1,585	Total			

Subcatchment 4A: Subbasin 4A



Attachment D: Water Quality Treatment Calculations

PAC Report Grassy Swale Calculations Basin 11A HydroCAD Plots

PAC Report

Project Details

Project Name	Permit No	Created
Bull Run Filtration Facility	TBD	1/5/2023 4:47:46 PM
Project Address	Designer	Last Modified
SE Carpenter Lane	Emerio Design	4/27/2023 11:36:11 PM
	Company Emerio Design	Report Generated 4/27/2023 4:56:28 PM

Project Summary

Catchment Name	Imper- vious Area (sq ft)	Native Soil Design Infilt- ration Rate (in/hr)	Level	Category	Config	Facility Area (excl. free board) (sq ft)	Facility Sizing Ratio (%)	PR Results	Infilt- ration Results	Flow Control Results
F1	96271	0	2B	SlopedFacility	D	2077.65	2.16	Pass	NA	Fail
81A	58265	0	2B	FlatPlanter	D	1344.00	2.31	Pass	NA	Fail
70B	18015	0	2B	FlatPlanter	D	315.00	1.75	Pass	NA	Fail
70A	36357	0	2B	FlatPlanter	D	522.00	1.44	Pass	NA	Fail
65A	10752	0	2B	FlatPlanter	D	508.00	4.72	Pass	NA	Fail
12B	20925	0	2B	FlatPlanter	D	324.00	1.55	Pass	NA	Fail
12A	8535	0	2B	FlatPlanter	D	206.00	2.41	Pass	NA	Fail
11C	7918	0	2B	FlatPlanter	D	257.00	3.25	Pass	NA	Fail
11B	35031	0	2B	FlatPlanter	D	656.00	1.87	Pass	NA	Fail
E1	103464	0	2B	SlopedFacility	D	2779.92	2.69	Pass	NA	Pass

Site Soils & Infiltration Testing	Infiltration Testing Procedure NA
	Tested Native Soil Infiltration Rate 0 in/hr
Correction Factor	CF test 2
Design Infiltration Rates	Native Soil 0 in/hr
	Imported Blended Soil 6 in/hr
Catchment Information	Hierarchy Level 2B
	Hierarchy Description
	Discharge to an overland storm drainage system, including streams, drainageways, and ditches, or to a storm-only pipe system that discharges to an overland storm drainage system.
	Pollution Reduction Requirement
	Filter the post-development stormwater runoff from the water quality storm event through the blended soil.
	Infiltration Requirement
	N/A
	Flow Control Requirement
	Limit the ½ the 2-yr, the 5-yr, and the 10-yr post- development peak flows to their respective pre- development peak flows. Unless the facility is a public facility (i.e., in the public right-of-way), also limit the 25-yr post-development peak flow to the 25-year pre- development peak flow.
	Impervious Area
	96271 sq ft 2.21 acre
	Pre-Development Time of Concentration (Tc pre)
	10 min
	Post-Development Time of Concentration (Tc post) 5 min
	Pre-Development Curve Number (CN pre) 82
	Post-Development Curve Number (CN post) 89

SBUH Results

PR 12 2 yr 5 yr 10 yr 10 yr 25 yr 1.8 1.6 1.4 1.2 Flow (cfs) 1.0 0.8 0.6 0.4 0.2 0-1000 ò 100 200 300 400 500 600 700 800 900 1100 1200 1300 1400 1470 Time (min)

	Pre - Development	t Rate and Volume	Post - Development Rate and Volume		
	Peak Rate (cfs) Total Volume (cf)		Peak Rate (cfs)	Total Volume (cf)	
PR	0.1303	3268	0.3763	5733.5	
1/2 2-Year	0.2123	3711.4	0.4023	5486	
5-Year	0.6451	10435.3	1.0982	14518.3	
10-Year	0.8826	13641.5	1.4003	18170.5	
25-Year	1.0815	16310.8	1.6455	21146.3	

	Overflow		Underdrain Outflow		Infiltration	
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)
PR	0	0	0.229	5733.5	0	0
1/2 2-Year	0	0	0.229	5486	0	0
5-Year	0.575	1177.5	0.229	13340.8	0	0
10-Year	1.108	3015	0.229	15155.5	0	0
25-Year	1.416	4833.7	0.229	16312.6	0	0

Sloped Facility

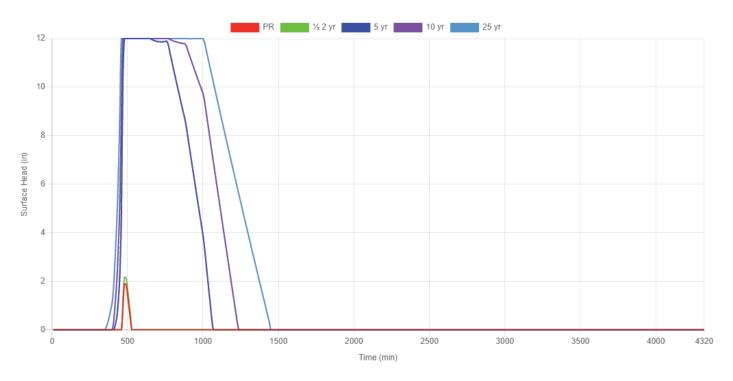
Site Soils & Infiltration Testing	Category Sloped Facility
	Shape Null
	Location Parcel
	Configuration D: Lined Facility with RS and Ud
	Above Grade Storage Data
	Total Depth of Blended Soil plus Rock 24 in
	Surface Storage Capacity at Overflow 1117.87 cu ft
	Design Infiltration Rate to Soil Underlying the Facility 0.000 cfs
	Design Infiltration Rate for Imported Blended Soil in the Facility 0.229 cfs
	Below Grade Storage Data
	Catchment is too small for flow control? No
	Rock Area 854.00 sq ft
	Rock Width 3.00 ft
	Rock Storage Depth 12.0 in
	Rock Porosity 0.3
	Percent of Facility Base that Allows Infiltration 0 %
	Underdrain Height 4 in
	Orifice (Y/N)? No
	Why no orifice

	Water-quality-only	facility				
Facility Facts	Total Facility Area (excluding freeboard) 2077.65 sq ft Sizing Ratio 2.16 % Segments Total Length 287.50 ft					
Pollution Reduction Results	Pollution Reduction Score Pass Overflow Volume 0.00 cf Surface Capacity Used 15.71 %					
Flow Control Results	Flow Control So Fail	STORMWATER FACILITY		PRE- DEVELOPMENT		
		OUTFLOW (CFS)		RUNOFF (CFS)		
	½ the 2 year	0.2292	<=	0.2123		
	5 year	0.8045	<=	0.6451		
	10 year	1.3368	<=	0.8826		
	25 year	1.6455	<=	1.0815		

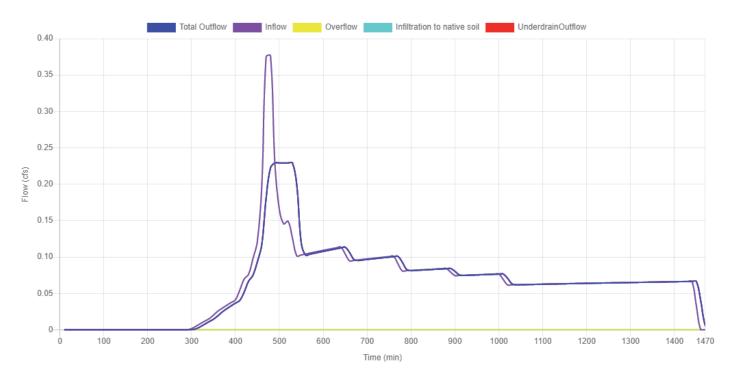
Sloped Facility Worksheet

Segment # Length (ft)	Check Dam	Slope, v/h (ft/ft)			Left Side Slope (_h:1v)	Down- gradient Depth (in)	Landscape Width (ft)	Adjusted Length (ft)	Up- gradient Depth (ft)	Surface Capacity Volume (cf)
0 57.00	0.5	0.01	3.00	3.0	3.0	12.0	9.0	57.00	0.43	223.57
1 57.00	0.5	0.01	3.00	3.0	3.0	12.0	9.0	57.00	0.43	223.57
2 57.00	0.5	0.01	3.00	3.0	3.0	12.0	9.0	57.00	0.43	223.57
3 57.00	0.5	0.01	3.00	3.0	3.0	12.0	9.0	57.00	0.43	223.57
4 57.00	0.5	0.01	3.00	3.0	3.0	12.0	9.0	57.00	0.43	223.57

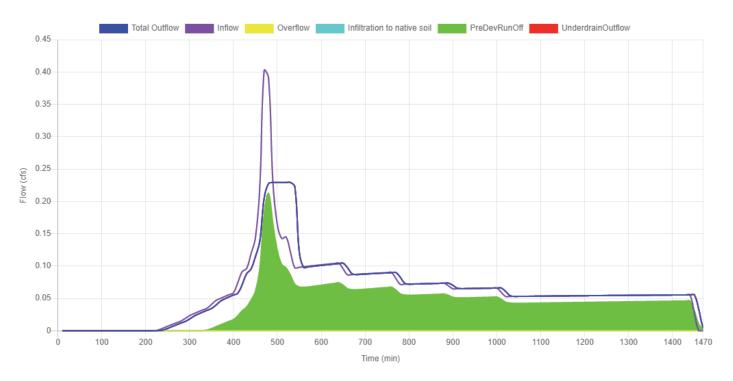
Surface Head



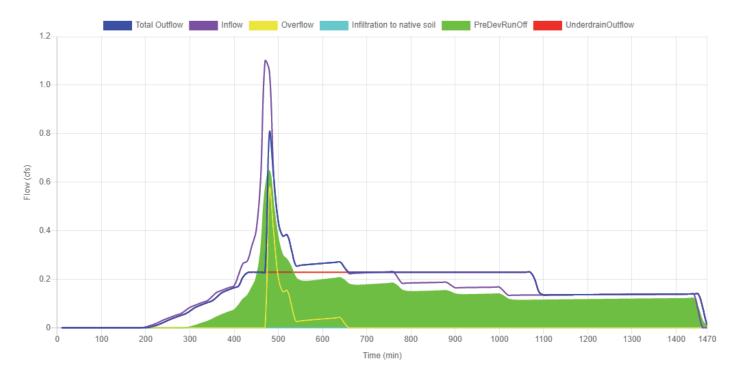
Water Quality



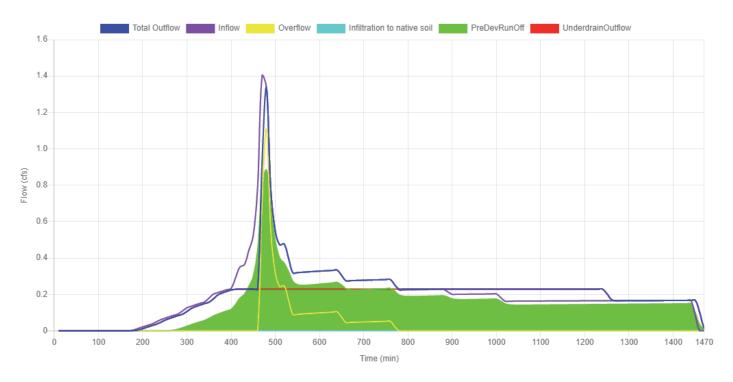
1/2 2-Year



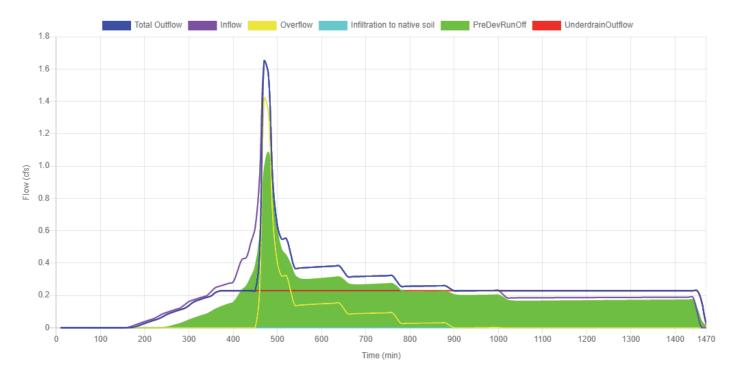
5-Year



10-Year



25-Year

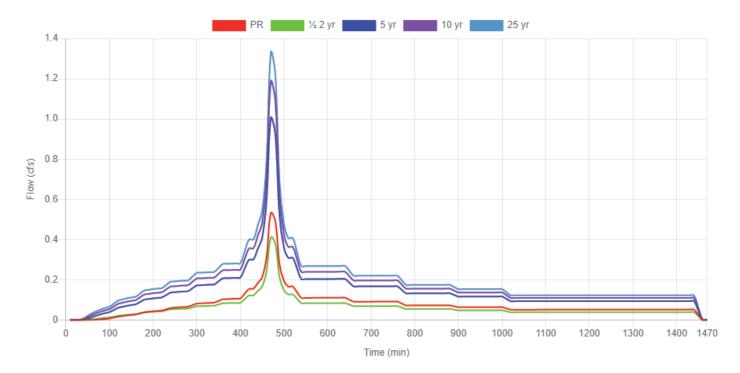


81A

Site Soils & Infiltration Testing	Infiltration Testing Procedure NA
	Tested Native Soil Infiltration Rate 0 in/hr
Correction Factor	CF test 2
Design Infiltration Rates	Native Soil 0 in/hr
	Imported Blended Soil 6 in/hr
Catchment Information	Hierarchy Level 2B
	Hierarchy Description
	Discharge to an overland storm drainage system, including streams, drainageways, and ditches, or to a storm-only pipe system that discharges to an overland storm drainage system.
	Pollution Reduction Requirement
	Filter the post-development stormwater runoff from the water quality storm event through the blended soil.
	Infiltration Requirement
	N/A
	Flow Control Requirement
	Limit the ½ the 2-yr, the 5-yr, and the 10-yr post- development peak flows to their respective pre- development peak flows. Unless the facility is a public facility (i.e., in the public right-of-way), also limit the 25-yr post-development peak flow to the 25-year pre- development peak flow.
	Impervious Area
	58265 sq ft 1.338 acre
	Pre-Development Time of Concentration (Tc pre)
	10 min
	Post-Development Time of Concentration (Tc post)
	5 min
	Pre-Development Curve Number (CN pre) 82
	Post-Development Curve Number (CN post) 98

SBUH Results

Post-Development Runoff



	Pre - Development	Rate and Volume	Post - Development Rate and Volume		
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	
PR	0.0788	1977.9	0.5322	6742.2	
1/2 2-Year	0.1285	2246.2	0.4113	5271.4	
5-Year	0.3904	6315.7	1.0047	12957.6	
10-Year	0.5342	8256.1	1.1859	15376.1	
25-Year	0.6545	9871.6	1.3304	17312.5	

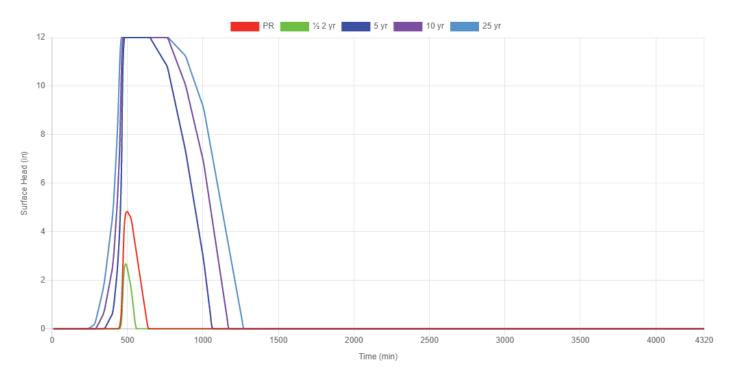
	Overflow		Underdrain Outflow		Infiltration	
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)
PR	0	0	0.187	6742.2	0	0
1/2 2-Year	0	0	0.187	5271.4	0	0
5-Year	0.523	918.8	0.187	12038.8	0	0
10-Year	0.908	2089.7	0.187	13286.4	0	0
25-Year	1.144	3197	0.187	14115.5	0	0

Flat Planter

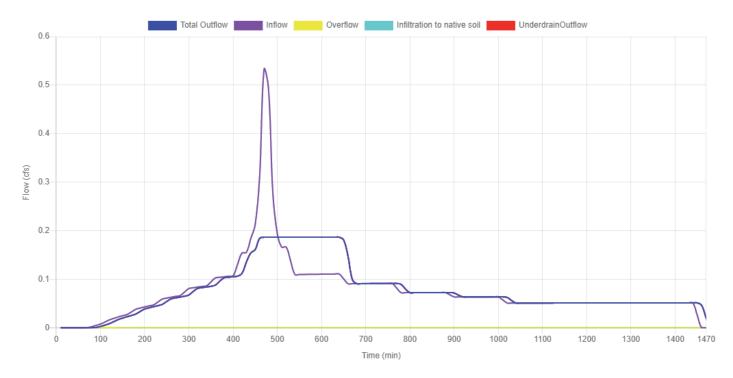
Site Soils & Infiltration Testing	Category
j	Flat Planter
	Shape
	Null
	Location
	Parcel
	Configuration
	D: Lined Facility with RS and Ud
	Above Grade Storage Data
	Bottom Area
	1344 sq ft
	Bottom Width
	10 ft
	Overflow Height 12 in
	Total Depth of Blended Soil plus Rock
	30 in
	Surface Storage Capacity at Overflow
	1344 cu ft
	Design Infiltration Rate to Soil Underlying the Facility 0.000 cfs
	Design Infiltration Rate for Imported Blended Soil in the Facility
	0.187 cfs
	Below Grade Storage Data
	Catchment is too small for flow control?
	No
	Rock Area 450.00 sq ft
	Rock Width
	3.00 ft
	Rock Storage Depth
	12.0 in
	Rock Porosity
	0.3
	Underdrain Height

	4 in					
	Percent of Facil 0 %	lity Base that Allow	vs Infil	tration		
	Orifice (Y/N)? No Why no orifice Water-quality-only facility					
Facility Facts	Total Facility Ar 1344.00 sq ft	ea (excluding free	board)		
	Sizing Ratio 2.31 %					
Pollution Reduction Results	Pass Overflow Volum 0.00 cf	Overflow Volume 0.00 cf Surface Capacity Used				
Flow Control Results	Flow Control Score Fail					
		STORMWATER FACILITY OUTFLOW (CFS)		PRE- DEVELOPMENT RUNOFF (CFS)		
	$\frac{1}{2}$ the 2 year	0.1867	<=	0.1285		
	5 year	0.7096	<=	0.3904		
	10 year	1.0946	<=	0.5342		
	25 year	1.3304	<=	0.6545		

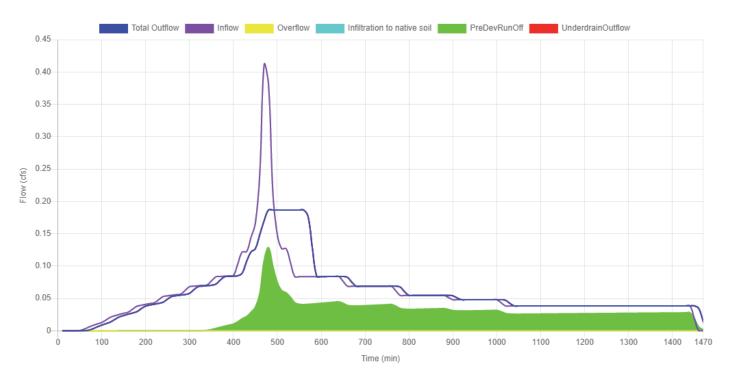
Surface Head

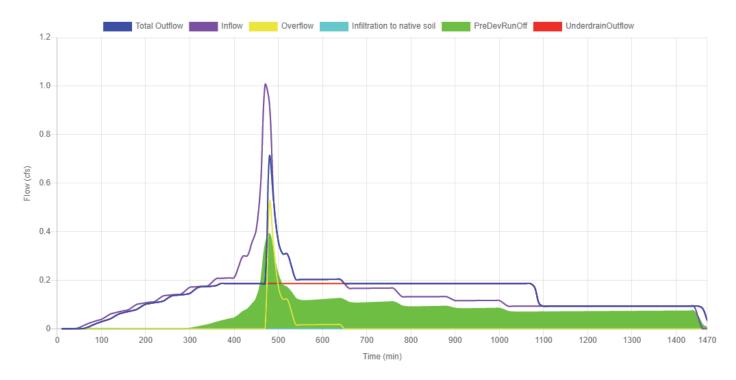


Water Quality

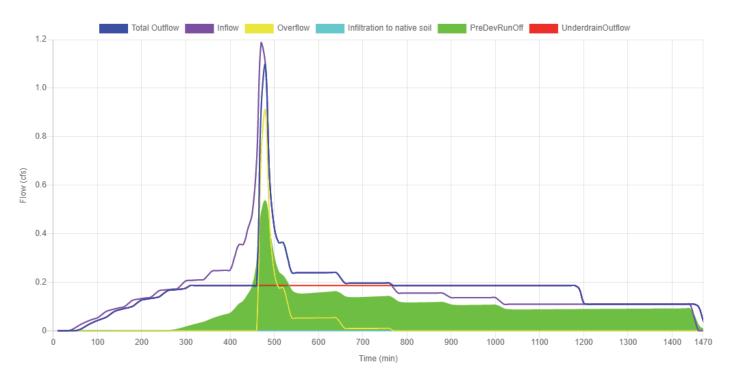


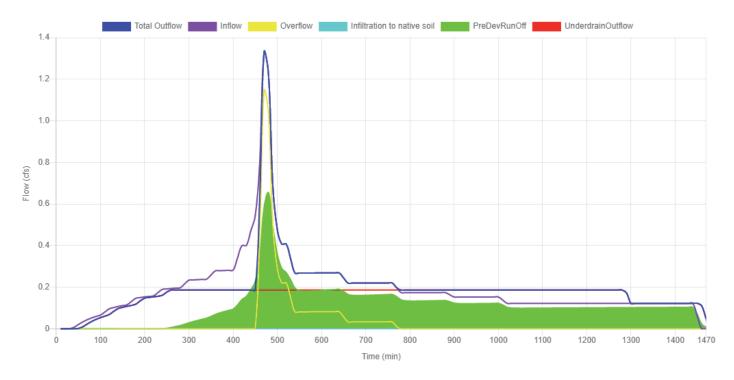
1/2 2-Year





10-Year





70B

Site Soils & Infiltration Testing	Infiltration Testing Procedure NA
	Tested Native Soil Infiltration Rate 0 in/hr
Correction Factor	CF test 2
Design Infiltration Rates	Native Soil 0 in/hr
	Imported Blended Soil 6 in/hr
Catchment Information	Hierarchy Level 2B
	Hierarchy Description
	Discharge to an overland storm drainage system, including streams, drainageways, and ditches, or to a storm-only pipe system that discharges to an overland storm drainage system.
	Pollution Reduction Requirement
	Filter the post-development stormwater runoff from the water quality storm event through the blended soil.
	Infiltration Requirement
	N/A
	Flow Control Requirement
	Limit the ½ the 2-yr, the 5-yr, and the 10-yr post- development peak flows to their respective pre- development peak flows. Unless the facility is a public facility (i.e., in the public right-of-way), also limit the 25-yr post-development peak flow to the 25-year pre- development peak flow.
	Impervious Area
	18015 sq ft 0.414 acre
	Pre-Development Time of Concentration (Tc pre)
	10 min
	Post-Development Time of Concentration (Tc post) 5 min
	Pre-Development Curve Number (CN pre) 82
	Post-Development Curve Number (CN post) 98

SBUH Results

PR 25 yr 5 yr 10 yr 25 yr 0.45 0.40 0.35 0.30 Flow (cfs) 0.25 0.20 0.15 0.10 0.05 0-100 200 300 400 500 600 700 800 900 1000 1100 1200 1300 1400 1470 Ó Time (min)

	Pre - Development Rate and Volume		Post - Development Rate and Volume		
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	
PR	0.0244	611.5	0.1646	2084.6	
1/2 2-Year	0.0397	694.5	0.1272	1629.9	
5-Year	0.1207	1952.7	0.3107	4006.4	
10-Year	0.1652	2552.7	0.3667	4754.1	
25-Year	0.2024	3052.2	0.4113	5352.9	

	Overflow		Underdrain Outflow		Infiltration	
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)
PR	0	0	0.044	2084.6	0	0
½ 2-Year	0	0	0.044	1629.9	0	0
5-Year	0.267	732	0.044	3274.4	0	0
10-Year	0.323	1213.7	0.044	3540.5	0	0
25-Year	0.368	1653.3	0.044	3699.6	0	0

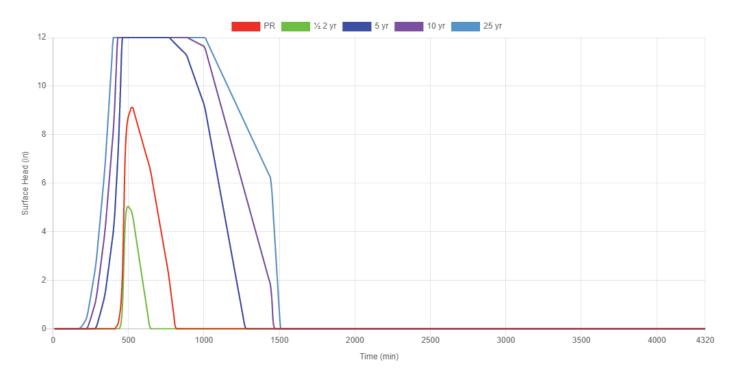
Post-Development Runoff

Flat Planter

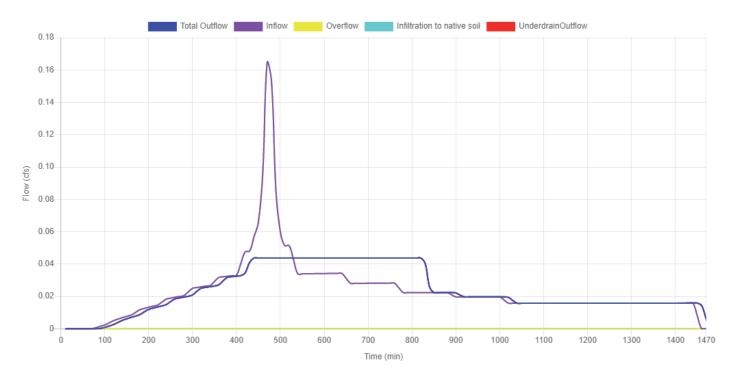
Site Soils & Infiltration Testing	Category
	Flat Planter
	Shape
	Null
	Location
	Parcel
	Configuration
	D: Lined Facility with RS and Ud
	Above Grade Storage Data
	Bottom Area
	315 sq ft
	Bottom Width
	11.00 ft
	Overflow Height 12 in
	Total Depth of Blended Soil plus Rock 30 in
	Surface Storage Capacity at Overflow
	315 cu ft
	Design Infiltration Rate to Soil Underlying the Facility 0.000 cfs
	Design Infiltration Rate for Imported Blended Soil in the
	Facility 0.044 cfs
	Below Grade Storage Data
	Catchment is too small for flow control?
	No
	Rock Area
	94.50 sq ft
	Rock Width
	3.00 ft
	Rock Storage Depth
	12.0 in
	Rock Porosity
	0.3
	Underdrain Height

	4 in				
	Percent of Facil 0 %	ity Base that Allow	vs Infil	tration	
	Orifice (Y/N)? No				
	Why no orifice Water-quality-only	facility			
Facility Facts	Total Facility Ar 315.00 sq ft	Total Facility Area (excluding freeboard) 315.00 sq ft)	
	Sizing Ratio 1.75 %				
Pollution Reduction Results	Pass Overflow Volum 0.00 cf	Overflow Volume 0.00 cf Surface Capacity Used			
Flow Control Results	Flow Control So Fail	Flow Control Score Fail			
		STORMWATER FACILITY OUTFLOW (CFS)		PRE- DEVELOPMENT RUNOFF (CFS)	
	$1/_2$ the 2 year	0.0438	<=	0.0397	
	5 year	0.3107	<=	0.1207	
	10 year	0.3667	<=	0.1652	
	25 year	0.4113	<=	0.2024	

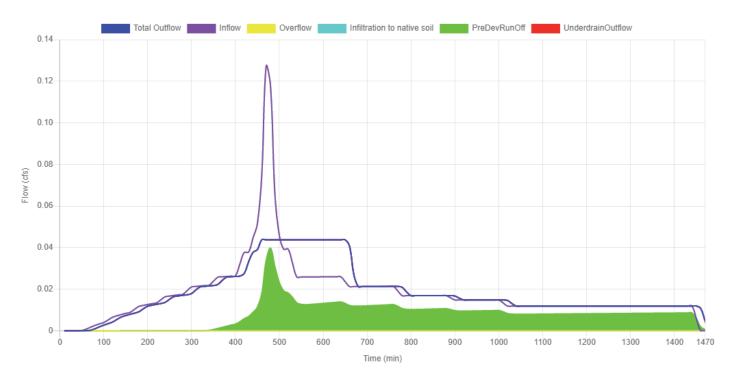
Surface Head

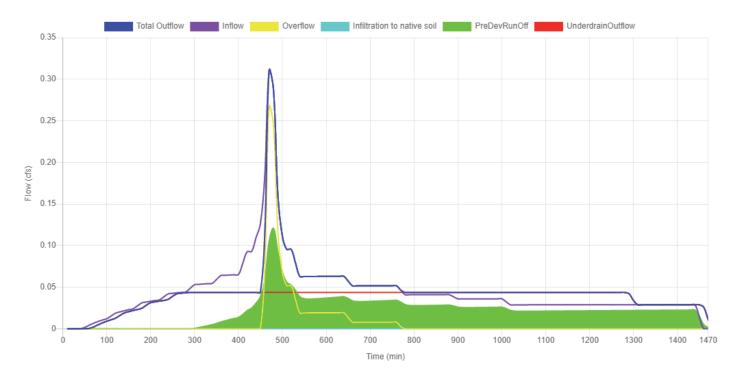


Water Quality

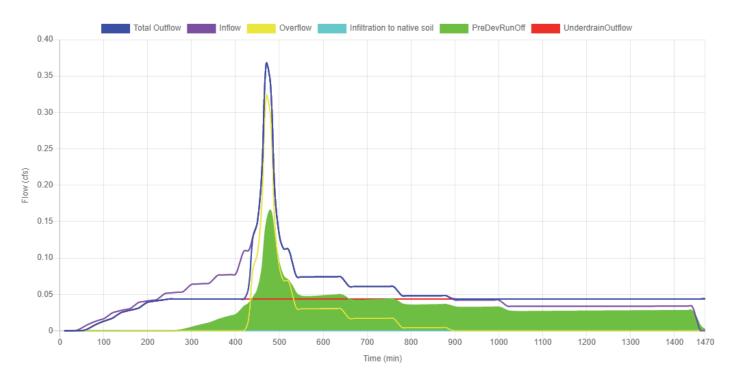


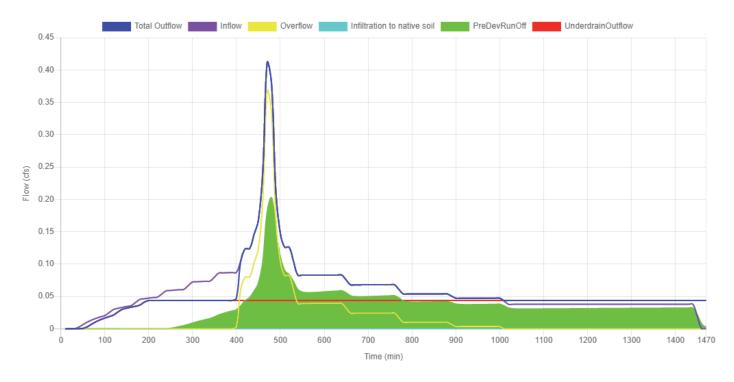
1/2 2-Year





10-Year





70A

Site Soils & Infiltration Testing	Infiltration Testing Procedure NA
	Tested Native Soil Infiltration Rate 0 in/hr
Correction Factor	CF test 2
Design Infiltration Rates	Native Soil 0 in/hr
	Imported Blended Soil 6 in/hr
Catchment Information	Hierarchy Level 2B
	Hierarchy Description
	Discharge to an overland storm drainage system, including streams, drainageways, and ditches, or to a storm-only pipe system that discharges to an overland storm drainage system.
	Pollution Reduction Requirement
	Filter the post-development stormwater runoff from the water quality storm event through the blended soil.
	Infiltration Requirement
	N/A
	Flow Control Requirement
	Limit the ½ the 2-yr, the 5-yr, and the 10-yr post- development peak flows to their respective pre- development peak flows. Unless the facility is a public facility (i.e., in the public right-of-way), also limit the 25-yr post-development peak flow to the 25-year pre- development peak flow.
	Impervious Area
	36357 sq ft 0.835 acre
	Pre-Development Time of Concentration (Tc pre)
	10 min
	Post-Development Time of Concentration (Tc post)
	5 min
	Pre-Development Curve Number (CN pre) 82
	Post-Development Curve Number (CN post) 98

SBUH Results

PR 1/2 2 yr 5 yr 10 yr 10 yr 25 yr 0.9 0.8 0.7 0.6 Flow (cfs) 0.5 0.4 0.3 0.2 0.1 0-100 ò 200 300 400 500 600 700 800 900 1000 1100 1200 1300 1400 1470 Time (min)

	Pre - Development Rate and Volume		Post - Development Rate and Volume		
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	
PR	0.0492	1234.2	0.3321	4207.1	
1/2 2-Year	0.0802	1401.6	0.2567	3289.3	
5-Year	0.2436	3940.9	0.6269	8085.5	
10-Year	0.3333	5151.8	0.74	9594.6	
25-Year	0.4084	6159.8	0.8302	10802.9	

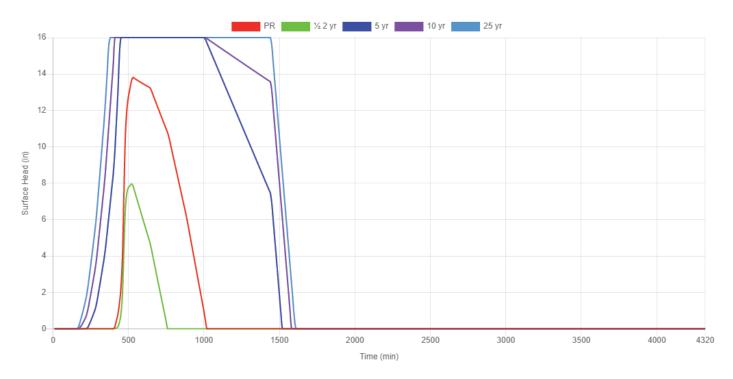
	Overflow		Underdrain Outflow		Infiltration	
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)
PR	0	0	0.072	4207.1	0	0
1/2 2-Year	0	0	0.072	3289.3	0	0
5-Year	0.554	2000.3	0.072	6085.2	0	0
10-Year	0.667	3142.4	0.072	6452.2	0	0
25-Year	0.758	4180	0.072	6622.9	0	0

Flat Planter

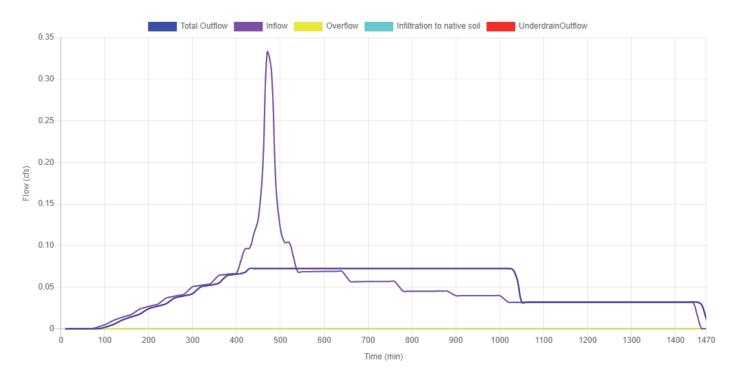
Site Soils & Infiltration Testing	Category
	Flat Planter
	Shape
	Null
	Location
	Parcel
	Configuration
	D: Lined Facility with RS and Ud
	Above Grade Storage Data
	Bottom Area
	522 sq ft
	Bottom Width
	11.00 ft
	Overflow Height
	16.0 in
	Total Depth of Blended Soil plus Rock 30 in
	Surface Storage Capacity at Overflow
	696 cu ft
	Design Infiltration Rate to Soil Underlying the Facility 0.000 cfs
	Design Infiltration Rate for Imported Blended Soil in the Facility
	0.073 cfs
	Below Grade Storage Data
	Catchment is too small for flow control?
	Rock Area
	150.00 sq ft
	Rock Width
	3.00 ft
	Rock Storage Depth
	12.0 in
	Rock Porosity
	0.3
	Underdrain Height

	4 in				
	Percent of Facil 0 %	ity Base that Allow	vs Infil	tration	
	Orifice (Y/N)? No				
	Why no orifice Water-quality-only	facility			
Facility Facts	Total Facility Ar 522.00 sq ft	ea (excluding free	board)	
	Sizing Ratio 1.44 %				
Pollution Reduction Results	Pollution Reduce Pass Overflow Volum				
	0.00 cf				
	Surface Capacity Used 86.33 %				
Flow Control Results	Flow Control Sc Fail	Flow Control Score Fail			
		STORMWATER FACILITY OUTFLOW (CFS)		PRE- DEVELOPMENT RUNOFF (CFS)	
	½ the 2 year	0.0725	<=	0.0802	
	5 year	0.6269	<=	0.2436	
	10 year	0.7400	<=	0.3333	
	25 year	0.8302	<=	0.4084	

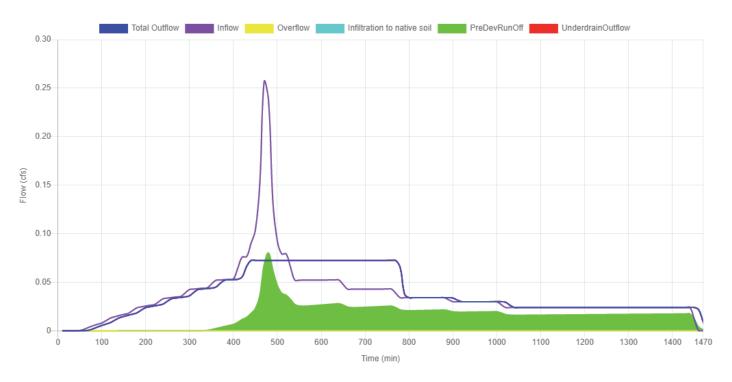
Surface Head

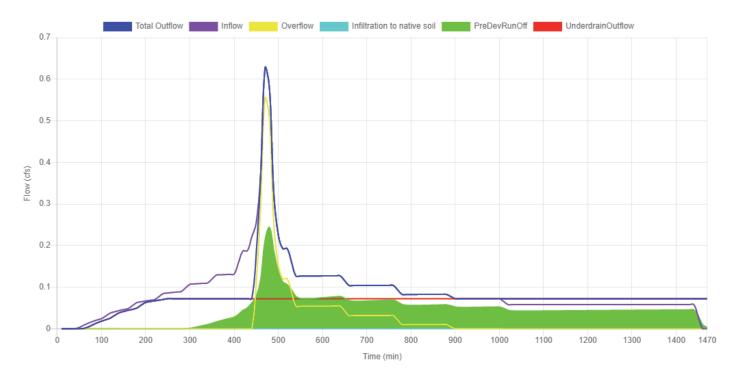


Water Quality

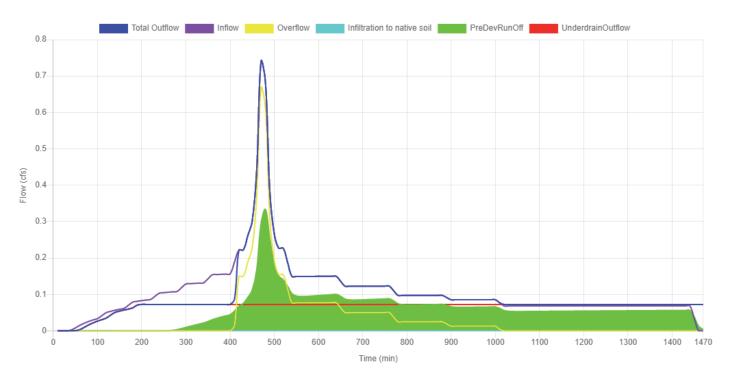


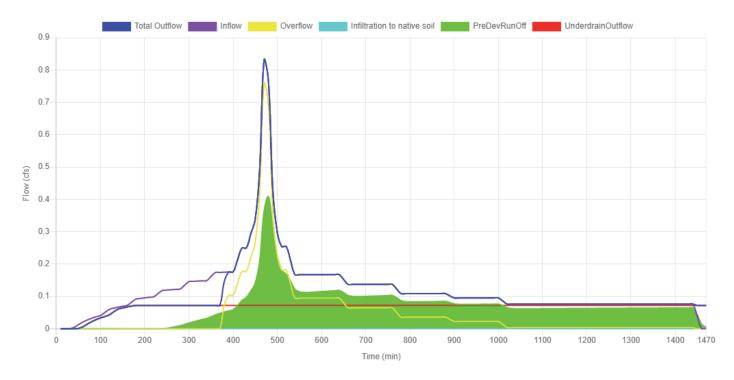
1/2 2-Year





10-Year



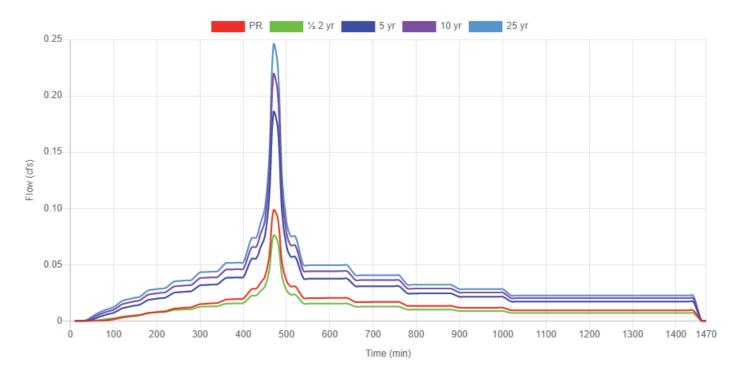


65A

Site Soils & Infiltration Testing	Infiltration Testing Procedure
	Tested Native Soil Infiltration Rate 0 in/hr
Correction Factor	CF test 2
Design Infiltration Rates	Native Soil 0 in/hr
	Imported Blended Soil 6 in/hr
Catchment Information	Hierarchy Level 2B
	Hierarchy Description
	Discharge to an overland storm drainage system, including streams, drainageways, and ditches, or to a storm-only pipe system that discharges to an overland storm drainage system.
	Pollution Reduction Requirement
	Filter the post-development stormwater runoff from the water quality storm event through the blended soil.
	Infiltration Requirement
	N/A
	Flow Control Requirement
	Limit the ½ the 2-yr, the 5-yr, and the 10-yr post- development peak flows to their respective pre- development peak flows. Unless the facility is a public facility (i.e., in the public right-of-way), also limit the 25-yr post-development peak flow to the 25-year pre- development peak flow.
	Impervious Area
	10752 sq ft 0.247 acre
	Pre-Development Time of Concentration (Tc pre)
	10 min
	Post-Development Time of Concentration (Tc post)
	5 min
	Pre-Development Curve Number (CN pre) 82
	Post-Development Curve Number (CN post) 98

SBUH Results

Post-Development Runoff



	Pre - Development Rate and Volume		Post - Development Rate and Volume		
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	
PR	0.0145	365	0.0982	1244.2	
1/2 2-Year	0.0237	414.5	0.0759	972.8	
5-Year	0.0721	1165.5	0.1854	2391.2	
10-Year	0.0986	1523.5	0.2188	2837.4	
25-Year	0.1208	1821.7	0.2455	3194.8	

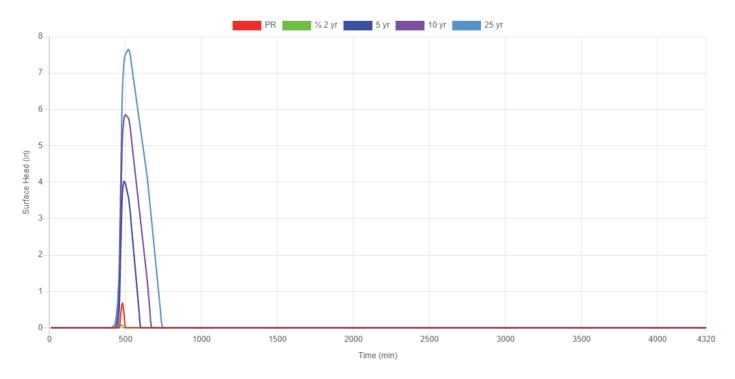
	Overflow		Underdrain Outflow		Infiltration	
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)
PR	0	0	0.071	1244.2	0	0
½ 2-Year	0	0	0.071	972.8	0	0
5-Year	0	0	0.071	2391.2	0	0
10-Year	0	0	0.071	2837.4	0	0
25-Year	0	0	0.071	3194.8	0	0

Flat Planter

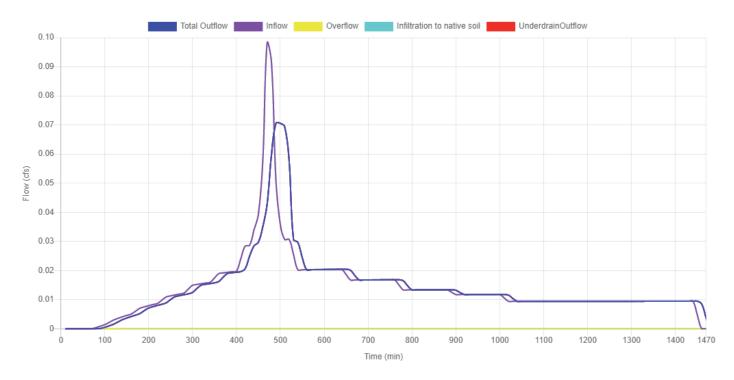
Site Soils & Infiltration Testing	Category
	Flat Planter
	Shape
	Null
	Location
	Parcel
	Configuration
	D: Lined Facility with RS and Ud
	Above Grade Storage Data
	Bottom Area
	508 sq ft
	Bottom Width 7.50 ft
	Overflow Height
	12 in
	Total Depth of Blended Soil plus Rock
	30 in
	Surface Storage Capacity at Overflow
	508 cu ft
	Design Infiltration Rate to Soil Underlying the Facility 0.000 cfs
	Design Infiltration Rate for Imported Blended Soil in the Facility
	0.071 cfs
	Below Grade Storage Data
	Catchment is too small for flow control?
	No
	Rock Area
	204.00 sq ft
	Rock Width
	3.00 ft
	Rock Storage Depth 12.0 in
	Rock Porosity 0.3
	Underdrain Height

	4 in					
	Percent of Facil 0 %	Percent of Facility Base that Allows Infiltration 0 %				
	Orifice (Y/N)? No					
	Why no orifice Water-quality-only facility					
Facility Facts	Total Facility Ar 508.00 sq ft	Total Facility Area (excluding freeboard) 508.00 sq ft				
	Sizing Ratio 4.72 %					
Pollution Reduction Results	Pass	Pollution Reduction Score Pass				
	Overflow Volume 0.00 cf					
	Surface Capacit 5.71 %	Surface Capacity Used 5.71 %				
Flow Control Results	Flow Control Score Fail					
		STORMWATER FACILITY OUTFLOW (CFS)		PRE- DEVELOPMENT RUNOFF (CFS)		
	½ the 2 year	0.0706	<=	0.0237		
	5 year	0.0706	<=	0.0721		
	10 year	0.0706	<=	0.0986		
	25 year	0.0706	<=	0.1208		

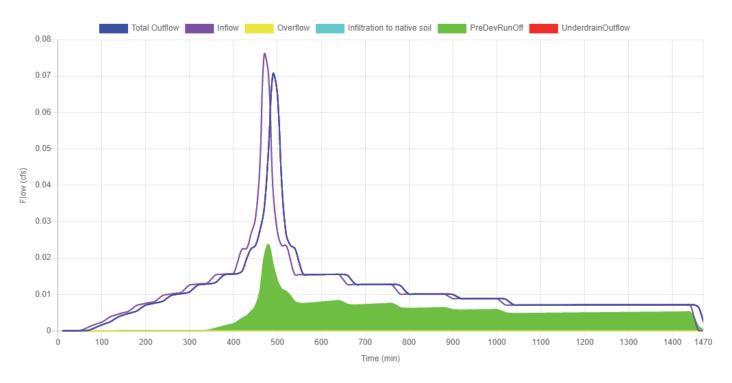
Surface Head

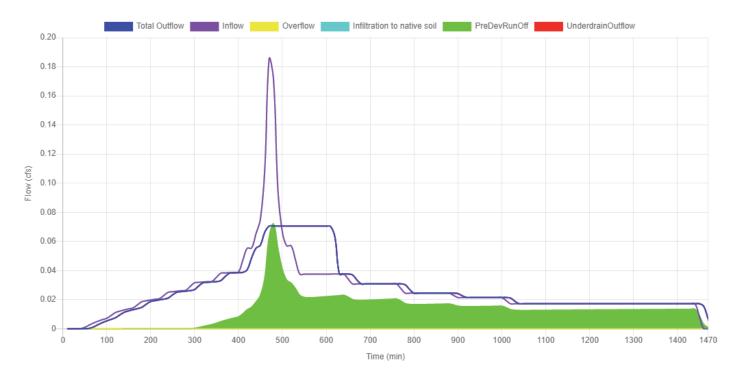


Water Quality

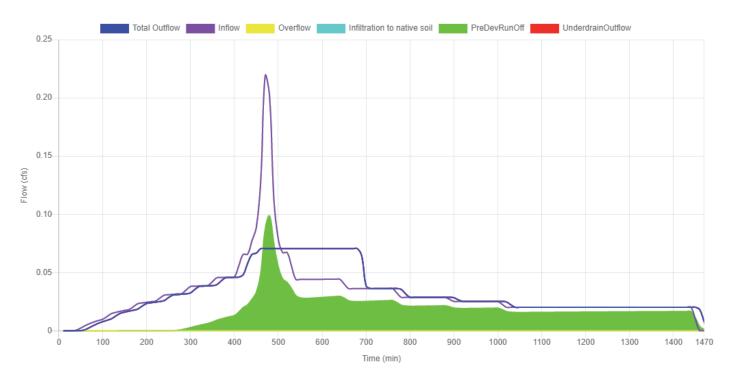


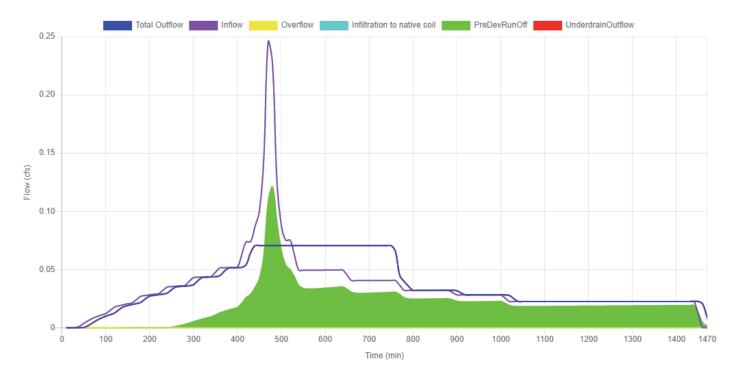
1/2 2-Year





10-Year





12B

Site Soils & Infiltration Testing	Infiltration Testing Procedure NA
	Tested Native Soil Infiltration Rate 0 in/hr
Correction Factor	CF test 2
Design Infiltration Rates	Native Soil 0 in/hr
	Imported Blended Soil 6 in/hr
Catchment Information	Hierarchy Level 2B
	Hierarchy Description
	Discharge to an overland storm drainage system, including streams, drainageways, and ditches, or to a storm-only pipe system that discharges to an overland storm drainage system.
	Pollution Reduction Requirement
	Filter the post-development stormwater runoff from the water quality storm event through the blended soil.
	Infiltration Requirement
	N/A
	Flow Control Requirement
	Limit the ½ the 2-yr, the 5-yr, and the 10-yr post- development peak flows to their respective pre- development peak flows. Unless the facility is a public facility (i.e., in the public right-of-way), also limit the 25-yr post-development peak flow to the 25-year pre- development peak flow.
	Impervious Area
	20925 sq ft 0.48 acre
	Pre-Development Time of Concentration (Tc pre)
	10 min
	Post-Development Time of Concentration (Tc post) 5 min
	Pre-Development Curve Number (CN pre) 82
	Post-Development Curve Number (CN post) 98

SBUH Results

PR 1/2 2 yr 5 yr 10 yr 10 yr 25 yr 0.50 0.45 0.40 0.35 0.30 Flow (cfs) 0.25 0.20 0.15 0.10 0.05 0-100 200 300 400 500 600 700 800 900 1000 1100 1200 1300 1400 1470 Ó Time (min)

Deet De		1 D
Post-De	velopmer	IT RUNOT

	Pre - Development Rate and Volume		Post - Development Rate and Volume		
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	
PR	0.0283	710.3	0.1911	2421.4	
1⁄2 2-Year	0.0462	806.7	0.1477	1893.1	
5-Year	0.1402	2268.2	0.3608	4653.5	
10-Year	0.1918	2965.1	0.4259	5522.1	
25-Year	0.2351	3545.2	0.4778	6217.5	

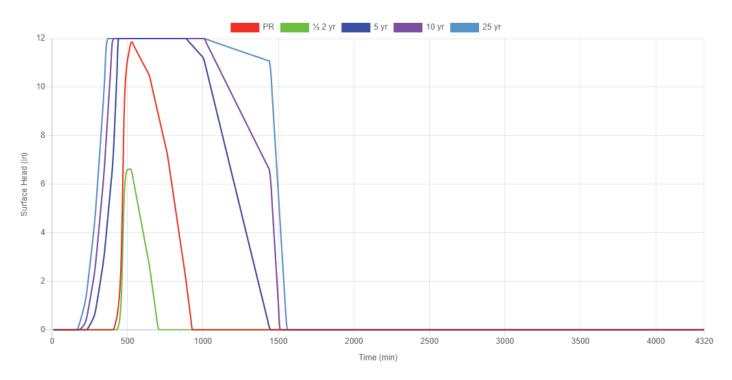
	Overflow		Underdrain Outflow		Infiltration	
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)
PR	0	0	0.045	2421.4	0	0
1/2 2-Year	0	0	0.045	1893.1	0	0
5-Year	0.316	1095.8	0.045	3557.7	0	0
10-Year	0.381	1720.6	0.045	3801.5	0	0
25-Year	0.433	2255.9	0.045	3961.6	0	0

Flat Planter

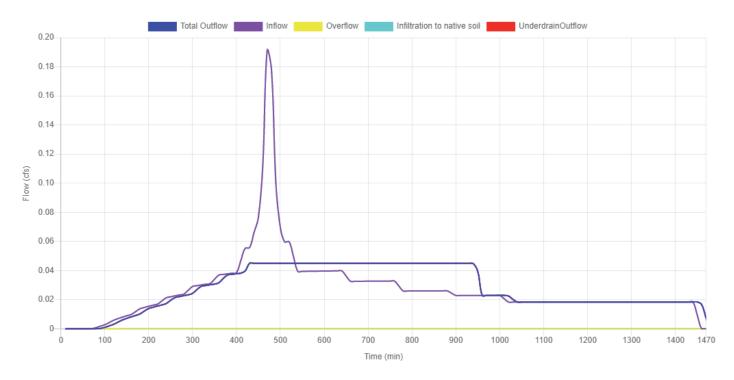
Site Soils & Infiltration Testing	Category
	Flat Planter
	Shape
	Null
	Location
	Parcel
	Configuration
	D: Lined Facility with RS and Ud
	Above Grade Storage Data
	Bottom Area
	324 sq ft
	Bottom Width 5.50 ft
	Overflow Height
	12 in
	Total Depth of Blended Soil plus Rock
	30 in
	Surface Storage Capacity at Overflow
	324 cu ft
	Design Infiltration Rate to Soil Underlying the Facility 0.000 cfs
	Design Infiltration Rate for Imported Blended Soil in the Facility
	0.045 cfs
	Below Grade Storage Data
	Catchment is too small for flow control?
	No
	Rock Area
	177.00 sq ft
	Rock Width
	3.00 ft
	Rock Storage Depth
	12.0 in
	Rock Porosity 0.3
	Underdrain Height

	4 in					
	Percent of Facil 0 %	Percent of Facility Base that Allows Infiltration 0 % Orifice (Y/N)? No				
	Why no orifice Water-quality-only facility					
Facility Facts	Total Facility Area (excluding freeboard) 324.00 sq ft Sizing Ratio 1.55 %					
Pollution Reduction Results	Pollution Reduction Score Pass Overflow Volume 0.00 cf Surface Capacity Used 98.86 %					
Flow Control Results	Flow Control Score Fail					
		STORMWATER FACILITY OUTFLOW (CFS)		PRE- DEVELOPMENT RUNOFF (CFS)		
	$\frac{1}{2}$ the 2 year	0.0450	<=	0.0462		
	5 year	0.3608	<=	0.1402		
	10 year	0.4259	<=	0.1918		
	25 year	0.4778	<=	0.2351		

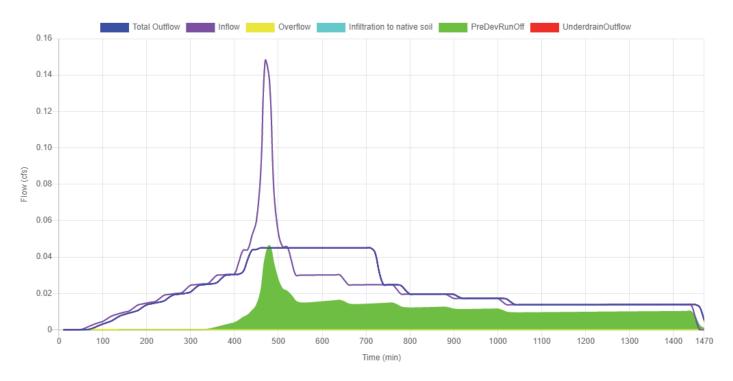
Surface Head

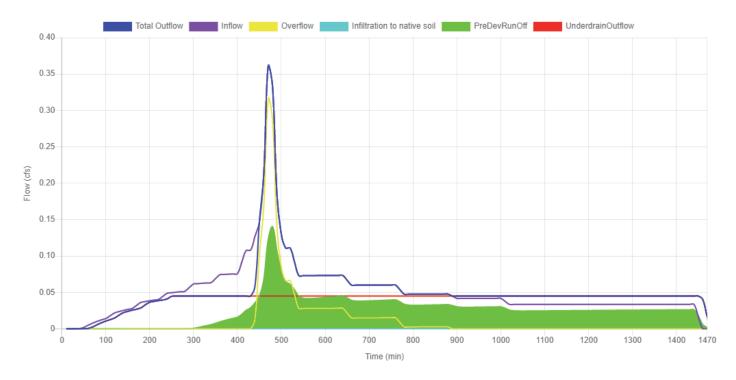


Water Quality

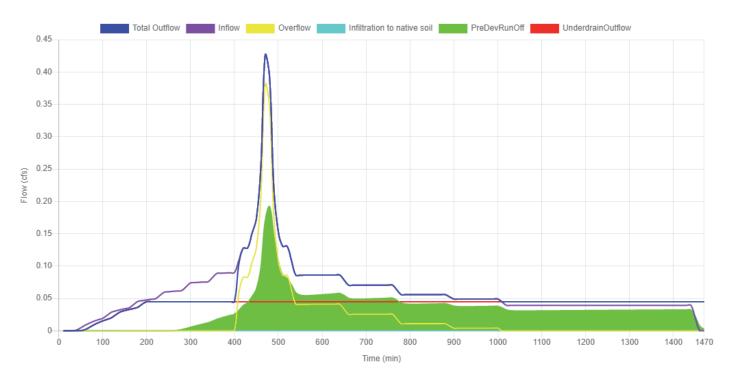


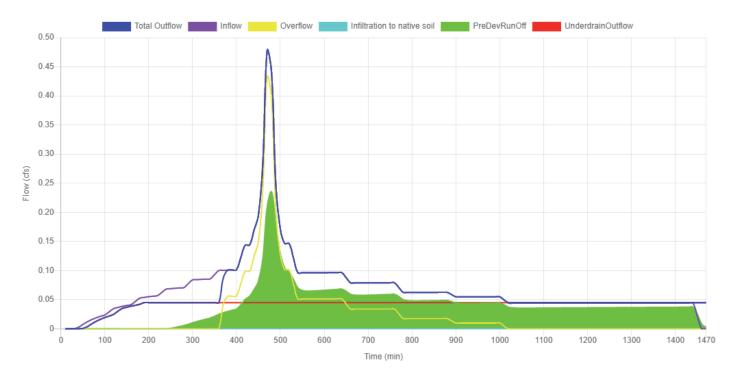
1/2 2-Year





10-Year





12A

Site Soils & Infiltration Testing	Infiltration Testing Procedure
	Tested Native Soil Infiltration Rate 0 in/hr
Correction Factor	CF test 2
Design Infiltration Rates	Native Soil 0 in/hr
	Imported Blended Soil 6 in/hr
Catchment Information	Hierarchy Level 2B
	Hierarchy Description
	Discharge to an overland storm drainage system, including streams, drainageways, and ditches, or to a storm-only pipe system that discharges to an overland storm drainage system.
	Pollution Reduction Requirement
	Filter the post-development stormwater runoff from the water quality storm event through the blended soil.
	Infiltration Requirement
	N/A
	Flow Control Requirement
	Limit the ½ the 2-yr, the 5-yr, and the 10-yr post- development peak flows to their respective pre- development peak flows. Unless the facility is a public facility (i.e., in the public right-of-way), also limit the 25-yr post-development peak flow to the 25-year pre- development peak flow.
	Impervious Area
	8535 sq ft 0.196 acre
	Pre-Development Time of Concentration (Tc pre)
	10 min
	Post-Development Time of Concentration (Tc post)
	5 min
	Pre-Development Curve Number (CN pre) 82
	Post-Development Curve Number (CN post) 98

SBUH Results

PR 1/2 2 yr 5 yr 10 yr 25 yr 25 yr 0.20 0.18 0.16 0.14 0.12 Flow (cfs) 0.10 0.08 0.06 0.04 0.02 0-100 200 300 400 500 600 700 800 900 1000 1100 1200 1300 1400 1470 Ó Time (min)

Deet Develo		Duneff
Post-Develo	pment	RUIIOII

	Pre - Development Rate and Volume		Post - Development Rate and Volume		
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	
PR	0.0115	289.7	0.078	987.6	
1/2 2-Year	0.0188	329	0.0603	772.2	
5-Year	0.0572	925.2	0.1472	1898.1	
10-Year	0.0783	1209.4	0.1737	2252.4	
25-Year	0.0959	1446	0.1949	2536	

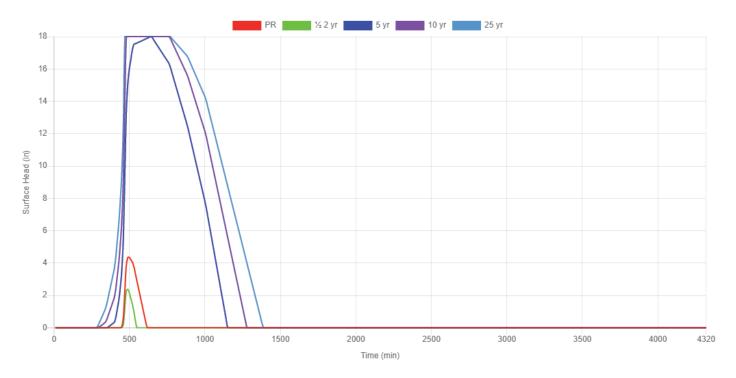
	Overflow		Underdrain Outflow		Infiltration	
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)
PR	0	0	0.029	987.6	0	0
¹ ⁄2 2-Year	0	0	0.029	772.2	0	0
5-Year	0	0	0.029	1898.1	0	0
10-Year	0.061	158	0.029	2094.4	0	0
25-Year	0.151	316.1	0.029	2220	0	0

Flat Planter

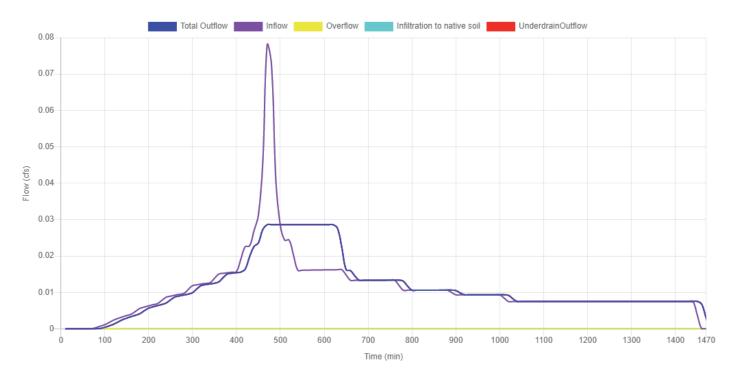
Site Soils & Infiltration Testing	Category
	Flat Planter
	Shape
	Null
	Location
	Parcel
	Configuration
	D: Lined Facility with RS and Ud
	Above Grade Storage Data
	Bottom Area
	206 sq ft
	Bottom Width 4.50 ft
	Overflow Height
	18.0 in
	Total Depth of Blended Soil plus Rock
	30 in
	Surface Storage Capacity at Overflow
	309 cu ft
	Design Infiltration Rate to Soil Underlying the Facility 0.000 cfs
	Design Infiltration Rate for Imported Blended Soil in the Facility
	0.029 cfs
	Below Grade Storage Data
	Catchment is too small for flow control?
	No
	Rock Area
	138.00 sq ft
	Rock Width
	3.00 ft
	Rock Storage Depth
	12.0 in
	Rock Porosity 0.3
	Underdrain Height
	, , , , , , , , , , , , , , , , , , ,

	4 in					
	Percent of Facil 0 %	Percent of Facility Base that Allows Infiltration 0 % Orifice (Y/N)? No				
	Why no orifice Water-quality-only facility					
Facility Facts	Total Facility Area (excluding freeboard) 206.00 sq ft					
	Sizing Ratio 2.41 %					
Pollution Reduction Results	Pollution Reduction Score Pass					
	Overflow Volume 0.00 cf					
	Surface Capacity Used 24.18 %					
Flow Control Results	Flow Control Score Fail					
		STORMWATER FACILITY OUTFLOW (CFS)		PRE- DEVELOPMENT RUNOFF (CFS)		
	½ the 2 year	0.0286	<=	0.0188		
	5 year	0.0286	<=	0.0572		
	10 year	0.0893	<=	0.0783		
	25 year	0.1798	<=	0.0959		

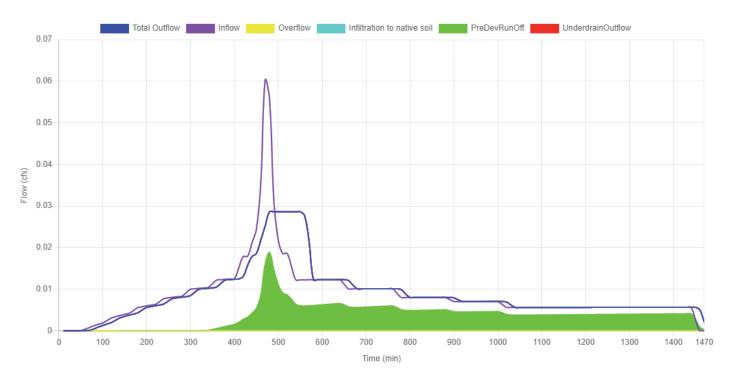
Surface Head

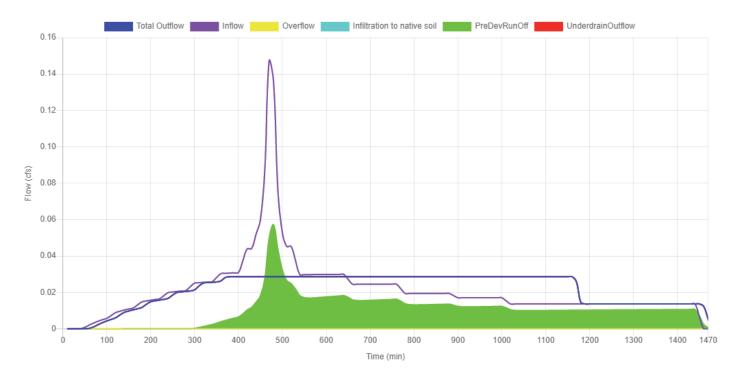


Water Quality

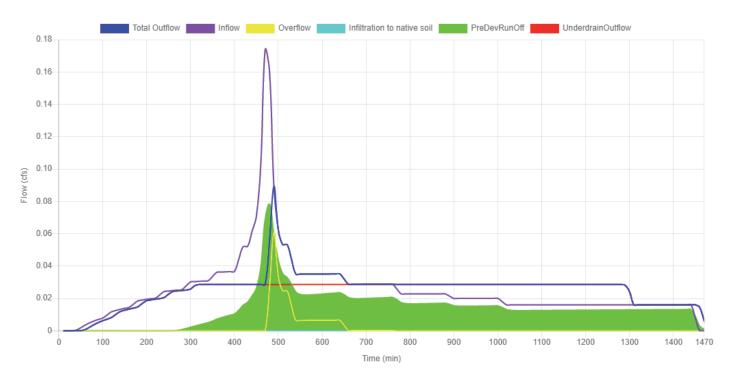


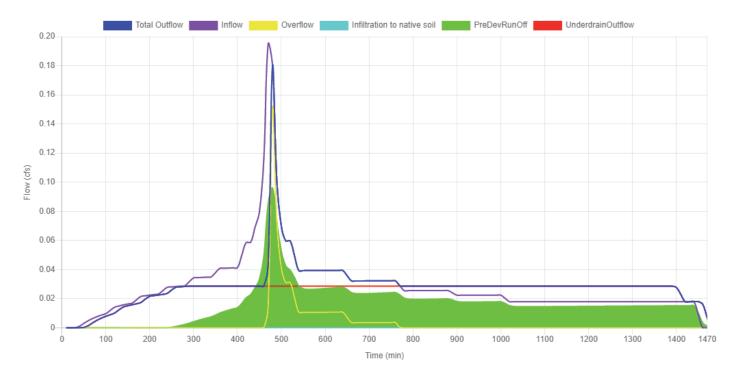
1/2 2-Year





10-Year





11C

Site Soils & Infiltration Testing	Infiltration Testing Procedure NA
	Tested Native Soil Infiltration Rate 0 in/hr
Correction Factor	CF test 2
Design Infiltration Rates	Native Soil 0 in/hr
	Imported Blended Soil 6 in/hr
Catchment Information	Hierarchy Level 2B
	Hierarchy Description
	Discharge to an overland storm drainage system, including streams, drainageways, and ditches, or to a storm-only pipe system that discharges to an overland storm drainage system.
	Pollution Reduction Requirement
	Filter the post-development stormwater runoff from the water quality storm event through the blended soil.
	Infiltration Requirement
	N/A
	Flow Control Requirement
	Limit the ½ the 2-yr, the 5-yr, and the 10-yr post- development peak flows to their respective pre- development peak flows. Unless the facility is a public facility (i.e., in the public right-of-way), also limit the 25-yr post-development peak flow to the 25-year pre- development peak flow.
	Impervious Area
	7918 sq ft 0.182 acre
	Pre-Development Time of Concentration (Tc pre)
	10 min
	Post-Development Time of Concentration (Tc post)
	5 min
	Pre-Development Curve Number (CN pre) 82
	Post-Development Curve Number (CN post) 98

SBUH Results

PR 25 yr 5 yr 10 yr 25 yr 0.20 0.18 0.16 0.14 0.12 Flow (cfs) 0.10 0.08 0.06 0.04 0.02 0-100 200 300 400 500 600 700 800 900 1000 1100 1200 1300 1400 1470 Ó Time (min)

Post-Develo	pment	Runoff
		. canon

	Pre - Development Rate and Volume		Post - Development Rate and Volum	
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)
PR	0.0107	268.8	0.0723	916.2
1/2 2-Year	0.0175	305.3	0.0559	716.4
5-Year	0.0531	858.3	0.1365	1760.9
10-Year	0.0726	1122	0.1612	2089.6
25-Year	0.0889	1341.5	0.1808	2352.7

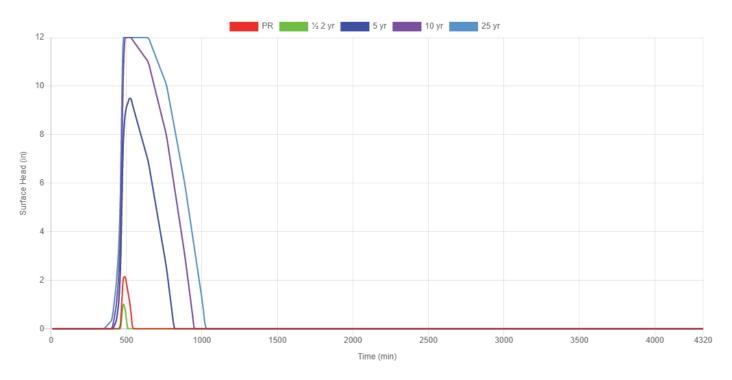
	Overflow		Underdrain Outflow		Infiltration	
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)
PR	0	0	0.036	916.2	0	0
½ 2-Year	0	0	0.036	716.4	0	0
5-Year	0	0	0.036	1760.9	0	0
10-Year	0.02	32	0.036	2057.6	0	0
25-Year	0.057	113.6	0.036	2239.1	0	0

Flat Planter

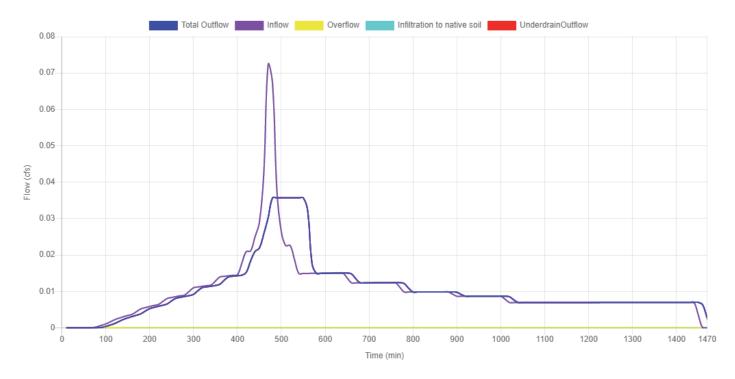
Site Soils & Infiltration Testing	Category
	Flat Planter
	Shape
	Null
	Location
	Parcel
	Configuration
	D: Lined Facility with RS and Ud
	Above Grade Storage Data
	Bottom Area
	257 sq ft
	Bottom Width 5.50 ft
	Overflow Height
	12 in
	Total Depth of Blended Soil plus Rock
	30 in
	Surface Storage Capacity at Overflow
	257 cu ft
	Design Infiltration Rate to Soil Underlying the Facility 0.000 cfs
	Design Infiltration Rate for Imported Blended Soil in the Facility
	0.036 cfs
	Below Grade Storage Data
	Catchment is too small for flow control?
	No
	Rock Area
	140.00 sq ft
	Rock Width
	3.00 ft
	Rock Storage Depth
	12.0 in
	Rock Porosity 0.3
	Underdrain Height

	4 in				
	Percent of Facil 0 %	ity Base that Allov	ws Infil	tration	
	Orifice (Y/N)? No				
	Why no orifice Water-quality-only	facility			
Facility Facts	Total Facility Ar 257.00 sq ft	Total Facility Area (excluding freeboard) 257.00 sq ft			
	Sizing Ratio 3.25 %				
Pollution Reduction Results	Pollution Reduc	Pollution Reduction Score Pass			
	Overflow Volum 0.00 cf	Overflow Volume 0.00 cf			
	Surface Capaci 17.88 %	Surface Capacity Used 17.88 %			
Flow Control Results	Flow Control So Fail	Flow Control Score Fail			
		STORMWATER FACILITY OUTFLOW (CFS)		PRE- DEVELOPMENT RUNOFF (CFS)	
	½ the 2 year	0.0357	<=	0.0175	
	5 year	0.0357	<=	0.0531	
	10 year	0.0561	<=	0.0726	
	25 year	0.0929	<=	0.0889	

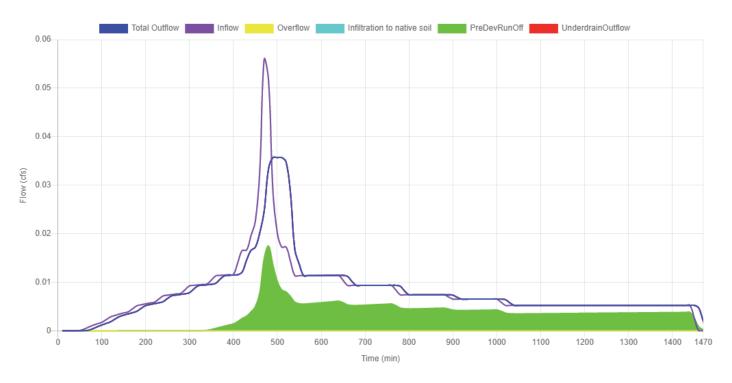
Surface Head

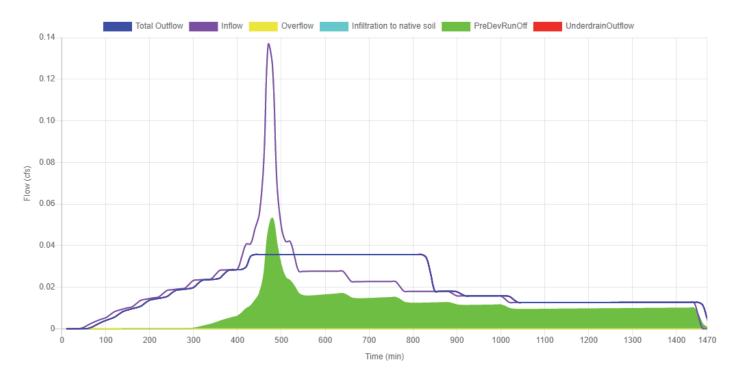


Water Quality

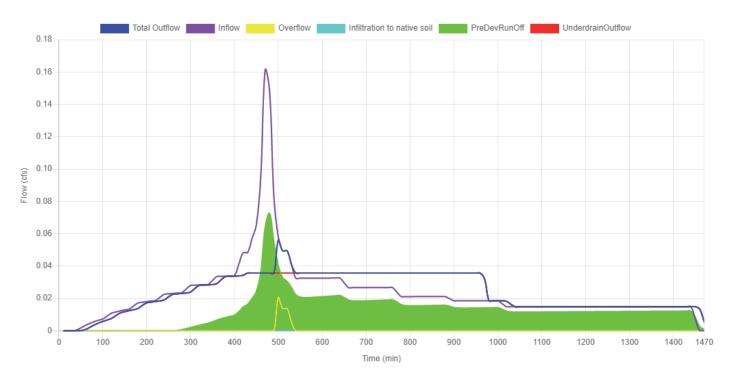


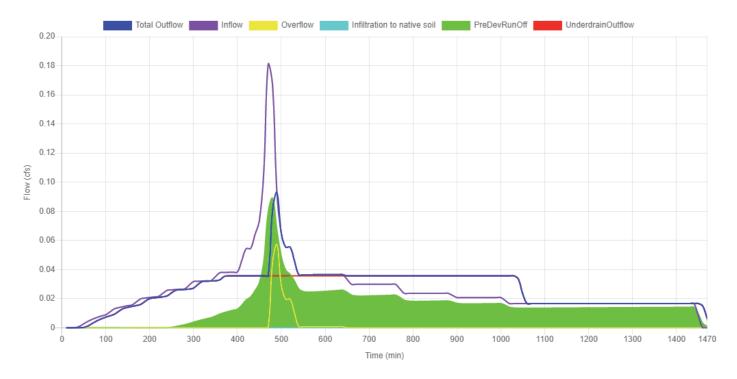
1/2 2-Year





10-Year



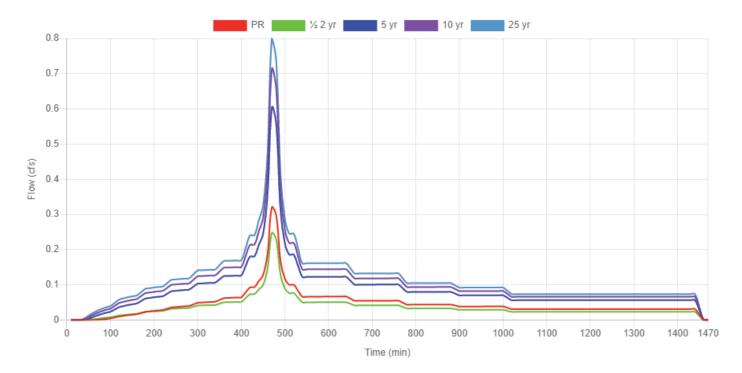


11B

Site Soils & Infiltration Testing	Infiltration Testing Procedure NA
	Tested Native Soil Infiltration Rate 0 in/hr
Correction Factor	CF test 2
Design Infiltration Rates	Native Soil 0 in/hr
	Imported Blended Soil 6 in/hr
Catchment Information	Hierarchy Level 2B
	Hierarchy Description
	Discharge to an overland storm drainage system, including streams, drainageways, and ditches, or to a storm-only pipe system that discharges to an overland storm drainage system.
	Pollution Reduction Requirement
	Filter the post-development stormwater runoff from the water quality storm event through the blended soil.
	Infiltration Requirement
	N/A
	Flow Control Requirement
	Limit the ½ the 2-yr, the 5-yr, and the 10-yr post- development peak flows to their respective pre- development peak flows. Unless the facility is a public facility (i.e., in the public right-of-way), also limit the 25-yr post-development peak flow to the 25-year pre- development peak flow.
	Impervious Area
	35031 sq ft 0.804 acre
	Pre-Development Time of Concentration (Tc pre)
	10 min
	Post-Development Time of Concentration (Tc post) 5 min
	Pre-Development Curve Number (CN pre) 82
	Post-Development Curve Number (CN post) 98

SBUH Results

Post-Development Runoff



	Pre - Development Rate and Volume		Post - Development Rate and Volum		
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	
PR	0.0474	1189.2	0.32	4053.6	
1/2 2-Year	0.0773	1350.5	0.2473	3169.4	
5-Year	0.2347	3797.2	0.6041	7790.6	
10-Year	0.3212	4963.9	0.713	9244.7	
25-Year	0.3935	5935.2	0.7999	10408.9	

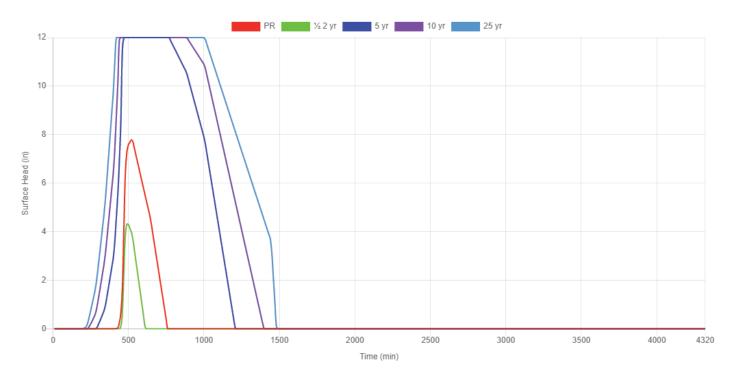
	Overflow		Underdrain Outflow		Infiltration	
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)
PR	0	0	0.091	4053.6	0	0
1/2 2-Year	0	0	0.091	3169.4	0	0
5-Year	0.468	1202.2	0.091	6588.4	0	0
10-Year	0.622	2075.7	0.091	7168.9	0	0
25-Year	0.709	2872.9	0.091	7536	0	0

Flat Planter

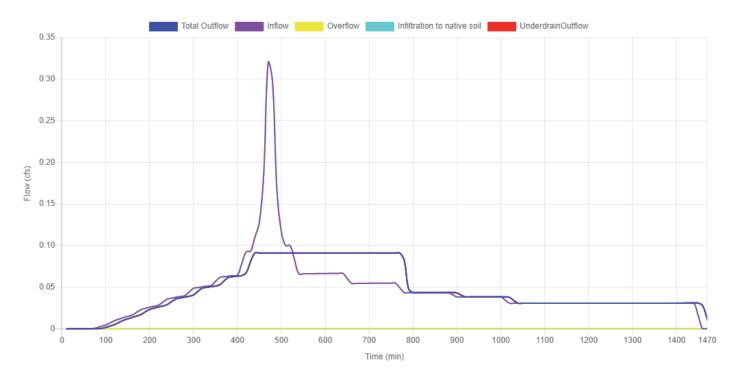
Site Soils & Infiltration Testing	Category
	Flat Planter
	Shape
	Null
	Location
	Parcel
	Configuration
	D: Lined Facility with RS and Ud
	Above Grade Storage Data
	Bottom Area
	656 sq ft
	Bottom Width 9.00 ft
	Overflow Height
	12 in
	Total Depth of Blended Soil plus Rock
	30 in
	Surface Storage Capacity at Overflow
	656 cu ft
	Design Infiltration Rate to Soil Underlying the Facility 0.000 cfs
	Design Infiltration Rate for Imported Blended Soil in the Facility
	0.091 cfs
	Below Grade Storage Data
	Catchment is too small for flow control?
	No
	Rock Area
	219.00 sq ft
	Rock Width
	3.00 ft
	Rock Storage Depth
	12.0 in
	Rock Porosity 0.3
	Underdrain Height

	4 in	4 in				
	Percent of Facil 0 %	Percent of Facility Base that Allows Infiltration 0 %				
	Orifice (Y/N)? No					
	Why no orifice Water-quality-only	facility				
Facility Facts	Total Facility Ar 656.00 sq ft	Total Facility Area (excluding freeboard) 656.00 sq ft				
	Sizing Ratio 1.87 %					
Pollution Reduction Results	Pass Overflow Volum 0.00 cf	Overflow Volume 0.00 cf Surface Capacity Used				
Flow Control Results	Flow Control So Fail	Flow Control Score Fail				
		STORMWATER FACILITY OUTFLOW (CFS)		PRE- DEVELOPMENT RUNOFF (CFS)		
	$\frac{1}{2}$ the 2 year	0.0911	<=	0.0773		
	5 year	0.5589	<=	0.2347		
	10 year	0.7130	<=	0.3212		
	25 year	0.7999	<=	0.3935		

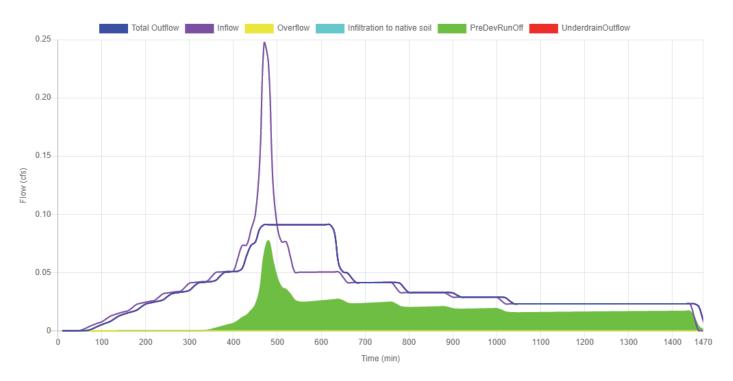
Surface Head

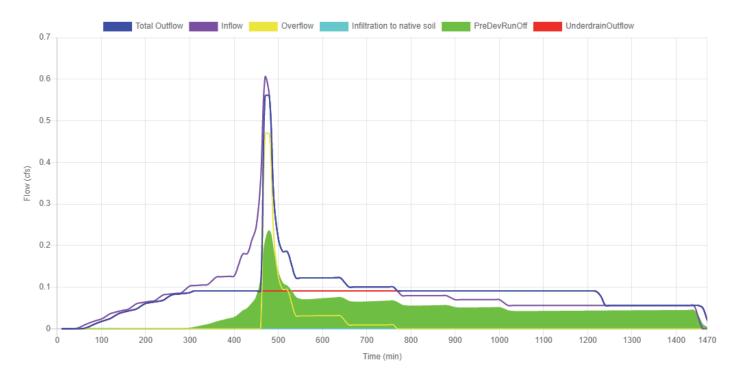


Water Quality

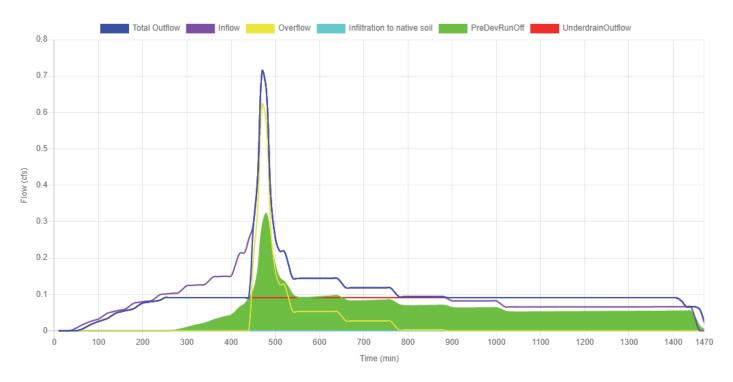


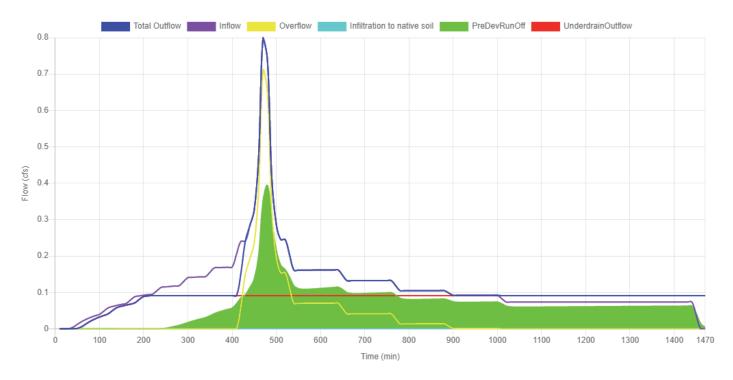
1/2 2-Year





10-Year



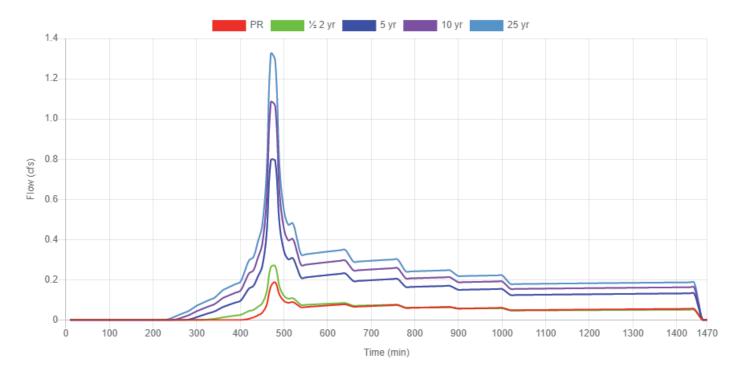


E1

Site Soils & Infiltration Testing	Infiltration Testing Procedure
	Tested Native Soil Infiltration Rate 0 in/hr
Correction Factor	CF test 2
Design Infiltration Rates	Native Soil 0 in/hr
	Imported Blended Soil 6 in/hr
Catchment Information	Hierarchy Level 2B
	Hierarchy Description
	Discharge to an overland storm drainage system, including streams, drainageways, and ditches, or to a storm-only pipe system that discharges to an overland storm drainage system.
	Pollution Reduction Requirement
	Filter the post-development stormwater runoff from the water quality storm event through the blended soil.
	Infiltration Requirement
	N/A
	Flow Control Requirement
	Limit the ½ the 2-yr, the 5-yr, and the 10-yr post- development peak flows to their respective pre- development peak flows. Unless the facility is a public facility (i.e., in the public right-of-way), also limit the 25-yr post-development peak flow to the 25-year pre- development peak flow.
	Impervious Area
	103464 sq ft 2.375 acre
	Pre-Development Time of Concentration (Tc pre)
	10 min
	Post-Development Time of Concentration (Tc post) 5 min
	Pre-Development Curve Number (CN pre) 82
	Post-Development Curve Number (CN post) 83

SBUH Results

Post-Development Runoff



	Pre - Development	Rate and Volume	Post - Development Rate and Volume		
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	
PR	0.14	3512.2	0.1871	3824.2	
1/2 2-Year	0.2282	3988.7	0.2678	4228.8	
5-Year	0.6933	11215	0.7949	11781.9	
10-Year	0.9486	14660.8	1.0831	15302	
25-Year	1.1623	17529.5	1.3239	18222.9	

	Overflow		Underdrain	Outflow	Infiltration		
	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	Peak Rate (cfs)	Total Volume (cf)	
PR	0	0	0.131	3701.5	0	0	
1/2 2-Year	0	0	0.183	4106.1	0	0	
5-Year	0	0	0.278	11659.2	0	0	
10-Year	0.15	231.6	0.304	14947.8	0	0	
25-Year	0.56	1252.9	0.304	16847.3	0	0	

Sloped Facility

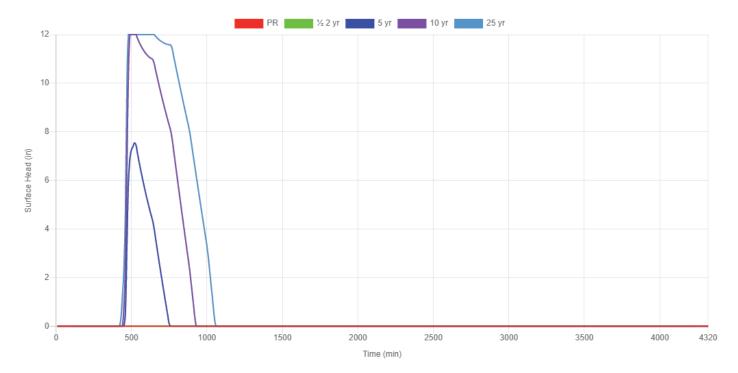
Site Soils & Infiltration Testing	Category Sloped Facility
	Shape Null
	Location Parcel
	Configuration D: Lined Facility with RS and Ud
	Above Grade Storage Data
	Total Depth of Blended Soil plus Rock 24 in
	Surface Storage Capacity at Overflow 1458.63 cu ft
	Design Infiltration Rate to Soil Underlying the Facility 0.000 cfs
	Design Infiltration Rate for Imported Blended Soil in the Facility 0.304 cfs
	Below Grade Storage Data
	Catchment is too small for flow control? No
	Rock Area 450.00 sq ft
	Rock Width 3.00 ft
	Rock Storage Depth 12.0 in
	Rock Porosity 0.3
	Percent of Facility Base that Allows Infiltration 0 %
	Underdrain Height 4 in
	Orifice (Y/N)? Yes
	Orifice Diameter

	3.000 in				
Facility Facts	Total Facility Area (excluding freeboard) 2779.92 sq ft Sizing Ratio 2.69 % Segments Total Length 399.00 ft				
Pollution Reduction Results	Pollution Reduction Score Pass Overflow Volume 0.00 cf Surface Capacity Used 0.00 %				
Flow Control Results	Flow Control S Pass	STORMWATER		PRE-	
		FACILITY OUTFLOW (CFS)		DEVELOPMENT RUNOFF (CFS)	
	$\frac{1}{2}$ the 2 year	0.1828	<=	0.2282	
	5 year	0.2780	<=	0.6933	
	10 year	0.4540	<=	0.9486	
	25 year	0.8616	<=	1.1623	

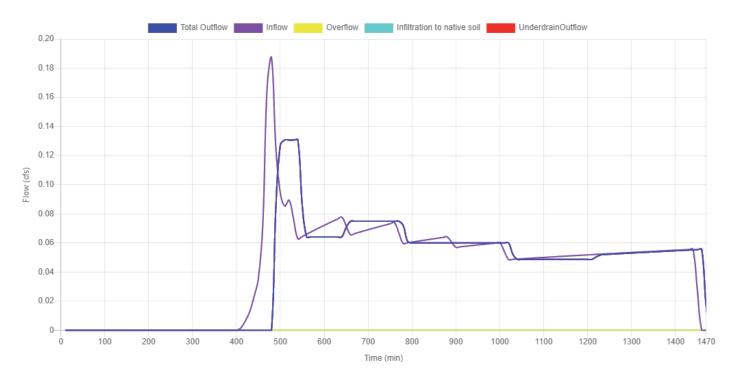
Sloped Facility Worksheet

Segment # Length (ft)	Check Dam	Slope, v/h (ft/ft)	Bottom Width (ft)	Right Side Slope (_h:1v)	Left Side Slope (_h:1v)	Down- gradient Depth (in)	Landscape Width (ft)	Adjusted Length (ft)	Up- gradient Depth (ft)	Surface Capacity Volume (cf)
0 66.00	0.5	0.01	3.00	3.0	3.0	12.0	9.0	66.00	0.34	243.10
1 66.00	0.5	0.01	3.00	3.0	3.0	12.0	9.0	66.00	0.34	243.10
2 66.00	0.5	0.01	3.00	3.0	3.0	12.0	9.0	66.00	0.34	243.10
3 66.00	0.5	0.01	3.00	3.0	3.0	12.0	9.0	66.00	0.34	243.10
4 66.00	0.5	0.01	3.00	3.0	3.0	12.0	9.0	66.00	0.34	243.10
5 66.00	0.5	0.01	3.00	3.0	3.0	12.0	9.0	66.00	0.34	243.10

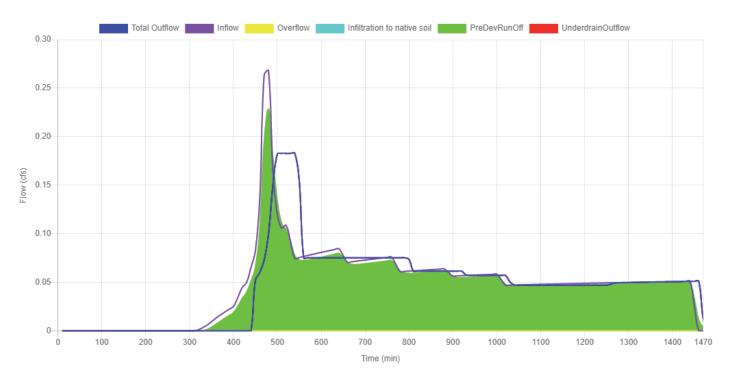
Surface Head

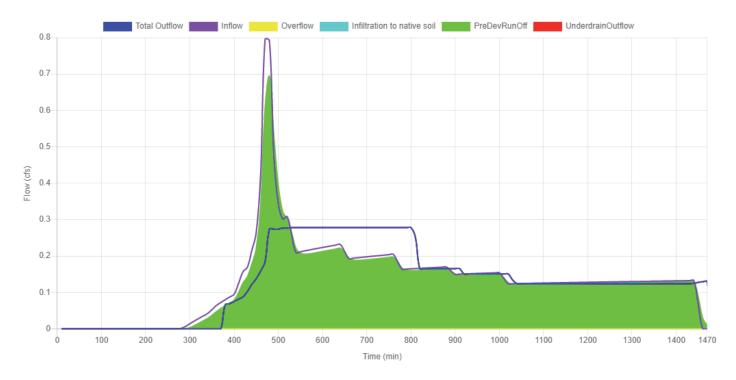


Water Quality

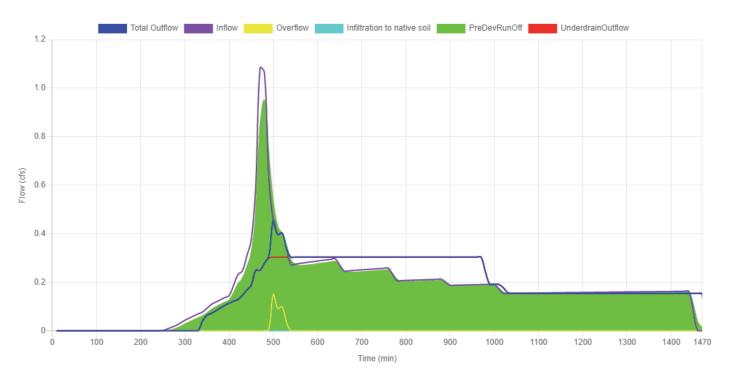


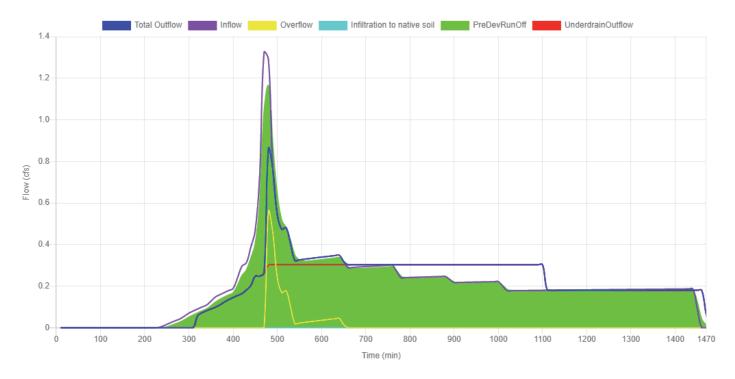
1/2 2-Year

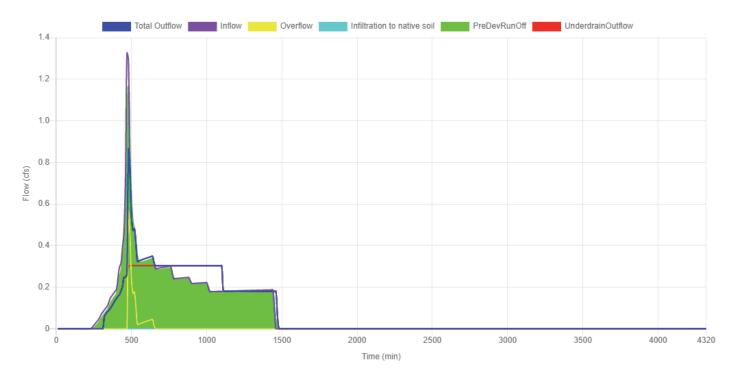




10-Year







Area 12CWater Quality Flow: $A = 51,507 \text{ sf}$ $l = 0.19 \text{ inches}$ WQF = 0.20 cfsBio-Swale Design Data: $Q = 0.202$ Design flow rate (c.f.s.) $n = 0.25$ manning's n $S = 0.5\%$ longitudinal slope of swale (ft/ft) $b = 2.00$ width of bottom (ft.) $Z = 3$ side slope (ft/ft) $t = 9$ Min Residence Time	
A = $51,507$ sfI = 0.19 inchesWQF = 0.20 cfs Bio-Swale Design Data: Q = 0.202 Design flow rate (c.f.s.)n = 0.25 manning's nS = 0.5% longitudinal slope of swale (ft/ft)b = 2.00 width of bottom (ft.)Z =3 side slope (ft/ft)t =9 Min Residence Time	
A = $51,507$ sfI = 0.19 inchesWQF = 0.20 cfs Bio-Swale Design Data: Q = 0.202 Design flow rate (c.f.s.)n = 0.25 manning's nS = 0.5% longitudinal slope of swale (ft/ft)b = 2.00 width of bottom (ft.)Z =3 side slope (ft/ft)t =9 Min Residence Time	
I =0.19 inches WQF =WQF =0.20 cfsBio-Swale Design Data:Q =0.202 Design flow rate (c.f.s.) n =n =0.25 manning's n S =S =0.5% longitudinal slope of swale (ft/ft) b =2.00 width of bottom (ft.) Z =Z =3 side slope (ft/ft) t =y =9 Min Residence Time	
WQF =0.20 cfs Bio-Swale Design Data: $Q =$ 0.202 Design flow rate (c.f.s.) $n =$ 0.25 manning's n $S =$ 0.5% longitudinal slope of swale (ft/ft) $b =$ 2.00 width of bottom (ft.) $Z =$ 3 side slope (ft/ft) $t =$ 9 Min Residence Time	
Bio-Swale Design Data: $Q =$ 0.202 Design flow rate (c.f.s.) $n =$ 0.25 manning's n $S =$ 0.5% longitudinal slope of swale (ft/ft) $b =$ 2.00 width of bottom (ft.) $Z =$ 3 side slope (ft/ft) $t =$ 9 Min Residence Time	
Q =0.202 Design flow rate (c.f.s.) $n =$ 0.25 manning's n $S =$ 0.5% longitudinal slope of swale (ft/ft) $b =$ 2.00 width of bottom (ft.) $Z =$ 3 side slope (ft/ft) $t =$ 9 Min Residence Time	
n = 0.25 manning's nS = 0.5% longitudinal slope of swale (ft/ft)b = 2.00 width of bottom (ft.)Z =3 side slope (ft/ft)t =9 Min Residence Time	
S =0.5% longitudinal slope of swale (ft/ft) $b =$ 2.00 width of bottom (ft.) $Z =$ 3 side slope (ft/ft) $t =$ 9 Min Residence Time	
b =2.00 width of bottom (ft.) $Z =$ 3 side slope (ft/ft) $t =$ 9 Min Residence Time	
Z = 3 side slope (ft/ft) t = 9 Min Residence Time	
t = 9 Min Residence Time	
Depth Area Wetted Perimeter Hydraulic Radius Top Width Velocity Length	
y (ft) A (sf) P (ft) R (ft) T (ft) V (ft/s) L (ft)	
0.3300 0.9867 4.0871 0.2414 3.9800 0.20 110.66	
Water Quality Design Velocity:	
0 - 0.000	
Q = 0.202	
A = 0.99 sf	
V=Q/A	
V = 0.20 ft/s	
Bio-Swale Length	
L=(V)(t)(60)	
t = 9.00 min	
V = 0.20 ft/s	
L= 110.66 ft ok	

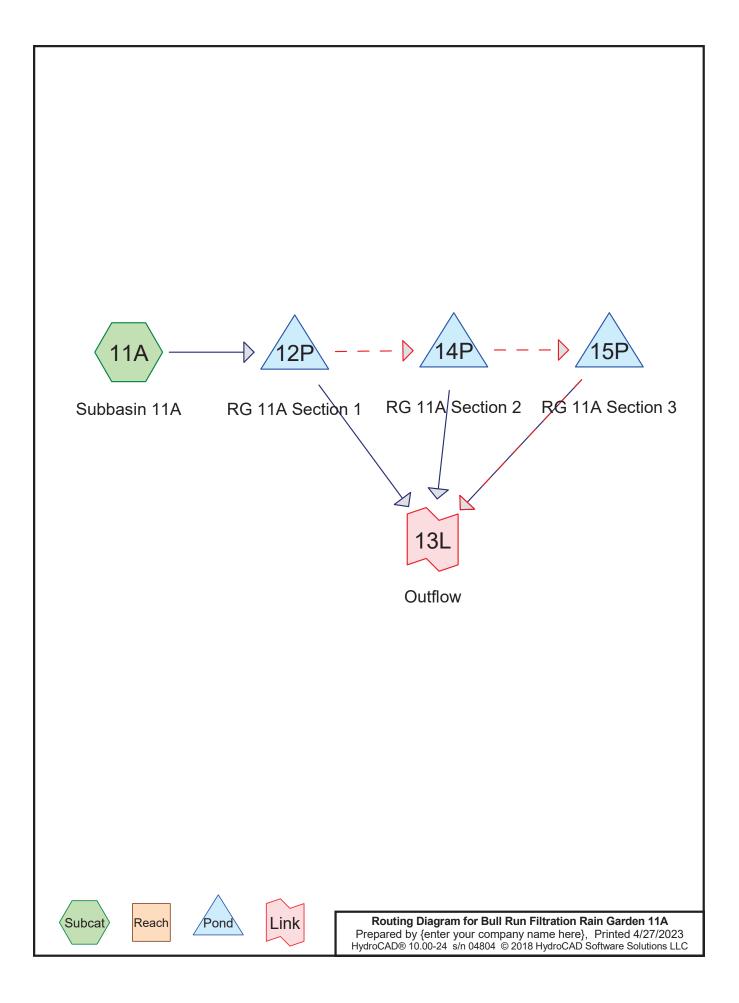
			BIO-SV	VALE DESIGN				
Area 51								
Water Qua	ality Flow:							
A =		sf						
=	,	inches						
WQF =	0.11	cfs						
	e Design Data:							
Q =		Design flow ra	ate (c.f.s.)					
n =	0.24	manning's n						
S =			ope of swale (ft/ft)					
b =		width of botto						
Z =		side slope (ft/						
t =	9	9 Min Residence Time						
I ,								
	Depth	Area	Wetted Perimeter		Top Width	Velocity	Length	
	y (ft)	A (sf)	P (ft)	R (ft)	T (ft)	V (ft/s)	L (ft)	
 	0.3300	1.0956	4.7212	0.2321	4.6400	0.10	55.664	
Water Qua	ality Design Ve	elocitv:						_
		,						
	Q =	0.113						
	A =	1.10	sf					
	V=Q/A							
I	V =	0.10	ft/s					
Bio-Swale	e Length							
1	L=(V)(t)(60)							
	t =	9.00						
	V =	0.10						
	L=	55.66	Use Minimum Swa	ale Length				

			BIO-SV	VALE DESIGN				
Road A								
Water Qua	ality Flow:							
A =		sf						
=	,	inches						
WQF =								
	e Design Data:							
Q =		Design flow r	ate (c.f.s.)					
n =		manning's n						
S =			lope of swale (ft/ft)					
b =		width of botto						
Z =		4 side slope (ft/ft)						
t =	9	Min Residend	ce Time					
	Depth	Area	Wetted Perimeter			Velocity	Length	
	y (ft)	A (sf)	P (ft)	R (ft)	T (ft)	<u>V (ft/s)</u>	L (ft)	
I	0.3300	1.0956	4.7212	0.2321	4.6400	0.08	43.513	
Water Qua	ality Design Ve	elocity:						
	Q =	0.088						
	A =	1.10	sf					
	V=Q/A							
	V =	0.08	ft/s					
Bio-Swale	e Length							
	L=(V)(t)(60)							
	t =	9.00						
	V =	0.08		- - th				
	L=	43.51	Use Minimum Swa	ale Length				

			BIO-SV	VALE DESIGN			
Road B1							
Water Qua	ality Flow:						
A =		ef					
	,	inches					
WQF =	0.19						
WGI -	0.10	015					
Bio-Swale	e Design Data:						
Q =		Design flow ra	ate (c.f.s.)				
n =		manning's n					
S =			ope of swale (ft/ft)				
b =		width of botto					
Z =		4 side slope (ft/ft)					
t =	9	Min Residenc	e Time				
I ,							
	Depth		Wetted Perimeter		Top Width	Velocity	Length
	y (ft)	A (sf)	P (ft)	R (ft)	T (ft)	V (ft/s)	L (ft)
 	0.3300	1.0956	4.7212	0.2321	4.6400	0.18	94.698
Water Qua	ality Design Ve	elocity:					
Trater Que		ciocity.					
	Q =	0.192					
	A =	1.10 s	sf				
	V=Q/A						
	V =	0.18 1	ft/s				
Bio-Swale	e Length						
	L=(V)(t)(60)						
	t =	9.00 ו					
1	V =	0.18 1					
I	L=	94.70	Use Minimum Swa	ale Length			

			BIO-SV	VALE DESIGN				
Road B2								
Water Qua	ality Flow:							
A =		sf						
=		inches						
WQF =	0.14							
	e Design Data:							
Q =		Design flow ra	ate (c.f.s.)					
n =		manning's n						
S =			ope of swale (ft/ft)					
b =		2.00 width of bottom (ft.)						
Z =		4 side slope (ft/ft)						
t =	9	Min Residence	e Time					
I ,		•						
	Depth		Wetted Perimeter			Velocity	Length	
	y (ft)	A (sf)	P (ft)	R (ft)	T (ft)	V (ft/s)	L (ft)	
	0.3300	1.0956	4.7212	0.2321	4.6400	0.13	69.847	
Water Qua	ality Design Ve	elocity:						
	Q =	0.142						
	A =	1.10 s	sf					
	V=Q/A							
	V =	0.13 f	t/s					
Bio-Swale	e Length							
	L=(V)(t)(60)							
	t =	9.00 r						
	V =	0.13 f						
	L=	69.85 l	Jse Minimum Swa	ale Length				

			BIO-SV	VALE DESIGN			
Road F							
Water Qua	ality Flow:						
A =	8,475	sf					
=	,	inches					
WQF =	0.03						
Bio-Swale	e Design Data:						
Q =		Design flow ra	ate (c.f.s.)				
n =		manning's n					
S =			lope of swale (ft/ft)				
b =		width of botto					
Z =		side slope (ft/					
t =	9	Min Residend	e Time				
l r							
	Depth	Area	Wetted Perimeter		Top Width	Velocity	Length
	y (ft)	A (sf)	P (ft)	R (ft)	T (ft)	<u>V (ft/s)</u>	L (ft)
l l	0.3300	1.0956	4.7212	0.2321	4.6400	0.03	16.398
Water Qua	ality Design Ve	elocity:					
	Q =	0.033					
	A =	1.10	sf				
	V=Q/A						
	V =	0.03	ft/s				
Bio-Swale	e Length						
	L=(V)(t)(60)						
	t =	9.00					
	V =	0.03		ala Lanath			
	L=	16.40	Use Minimum Swa	ale Length			



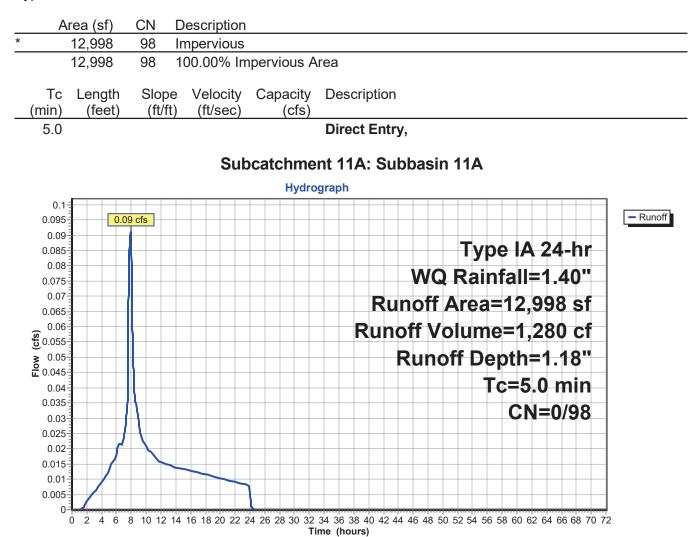
Bull Run Filtration Rain Garden 11A

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Summary for Subcatchment 11A: Subbasin 11A

Runoff = 0.09 cfs @ 7.89 hrs, Volume= 1,280 cf, Depth= 1.18"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr WQ Rainfall=1.40"



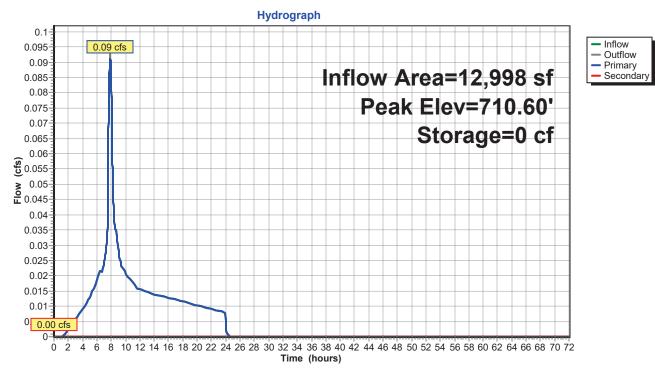
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Summary for Pond 12P: RG 11A Section 1

Outflow :	= 0.09 cfs @ = 0.09 cfs @ = 0.09 cfs @	100.00% Impervious, Inflow Depth = 1.18" for WQ event 7.89 hrs, Volume= 1,280 cf 7.89 hrs, Volume= 1,280 cf, Atten= 0%, Lag= 0.0 min 7.89 hrs, Volume= 1,280 cf 0.00 hrs, Volume= 0 cf
		ne Span= 0.00-72.00 hrs, dt= 0.01 hrs Surf.Area= 11 sf Storage= 0 cf
Center-of-N	/lass det. time= 0.0 m	
Volume		torage Storage Description
#1	710.59' 1,	148 cf Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store Cum.Store (cubic-feet) (cubic-feet)
710.59	9	0 0
713.59	756	1,148 1,148
Device Ro	outing Inver	t Outlet Devices
#1 Se	econdary 713.26	1.0' long x 1.0' breadth Broad-Crested Rectangular Weir
		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
		2.50 3.00
		Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31
		3.30 3.31 3.32
#2 Pr	rimary 708.42	2 4.0" Round Culvert
		L= 26.5' CMP, square edge headwall, Ke= 0.500
		Inlet / Outlet Invert= 708.42' / 708.29' S= 0.0049 '/' Cc= 0.900
		n= 0.013, Flow Area= 0.09 sf
Primary Ou	ItElow Max=0.44 cf	≈ 0.789 brs HW=710.60' (Free Discharge)

Primary OutFlow Max=0.44 cfs @ 7.89 hrs HW=710.60' (Free Discharge) —2=Culvert (Barrel Controls 0.44 cfs @ 4.99 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=710.59' (Free Discharge) —1=Broad-Crested Rectangular Weir (Controls 0.00 cfs) Prepared by {enter your company name here} HydroCAD® 10.00-24 s/n 04804 © 2018 HydroCAD Software Solutions LLC



Pond 12P: RG 11A Section 1

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Summary for Pond 14P: RG 11A Section 2

	= = = / =	0.00 cfs @ 0.00 cfs @ 0.00 cfs @ 0.00 cfs @	0.00 hrs 0.00 hrs	s, Volume= s, Volume= s, Volume= s, Volume=	0 cf 0 cf, Atten= 0%, Lag= 0.0 min 0 cf 0 cf
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 711.01' @ 0.00 hrs Surf.Area= 21 sf Storage= 0 cf					
Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no inflow)					
Volume	Inve	ert Avail.S	torage 🗧	Storage Description	
#1	711.0	1'	988 cf (Custom Stage Data (Pri	smatic) Listed below (Recalc)

#1	711.01	98	So of Custom S	tage Data (Pri	smatic) Listed below (Recalc)
Elevatio (fee		f.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
711.0)1	21	0	0	
713.5	51	769	988	988	
Device	Routing	Invert	Outlet Devices		
#1	Secondary	713.14'	1.0' long x 1.0'	breadth Broa	d-Crested Rectangular Weir
#2	Primary	708.29'	Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32 4.0" Round Culvert L= 30.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 708.29' / 708.14' S= 0.0050 '/' Cc= 0.900 n= 0.013, Flow Area= 0.09 sf		

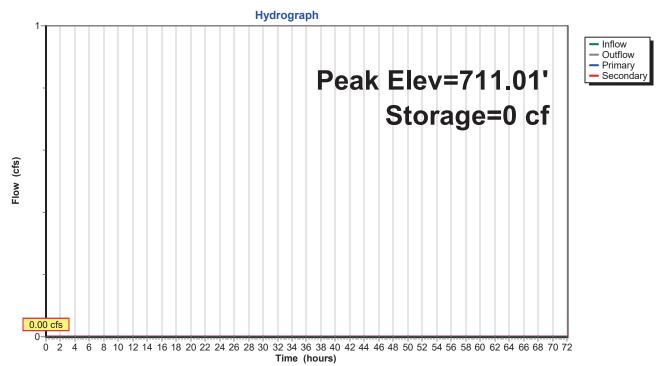
Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=711.01' (Free Discharge) **2=Culvert** (Passes 0.00 cfs of 0.47 cfs potential flow)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=711.01' (Free Discharge) —1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Bull Run Filtration Rain Garden 11A

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315

713.39

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Summary for Pond 15P: RG 11A Section 3

Inflow = Outflow = Primary = Secondary =	0.00 cfs @ 0.00 cfs @ 0.00 cfs @ 0.00 cfs @	-	າe= າe=	0 cf 0 cf, Atten= 0%, Lag= 0.0 min 0 cf 0 cf	
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 711.39' @ 0.00 hrs Surf.Area= 9 sf Storage= 0 cf					
Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no inflow)					
Volume Ir	nvert Avail.S	torage Storage	Description		
#1 712	1.39'	324 cf Custom	Stage Data (Pris	smatic) Listed below (Recalc)	
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
711.39	9	0	0		

Device	Routing	Invert	Outlet Devices
#1	Primary	708.14'	4.0" Round Culvert
	-		L= 13.5' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 708.14' / 708.07' S= 0.0052 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.09 sf
#2	Secondary	712.75'	18.0" Horiz. Beehive C= 0.620 Limited to weir flow at low heads

324

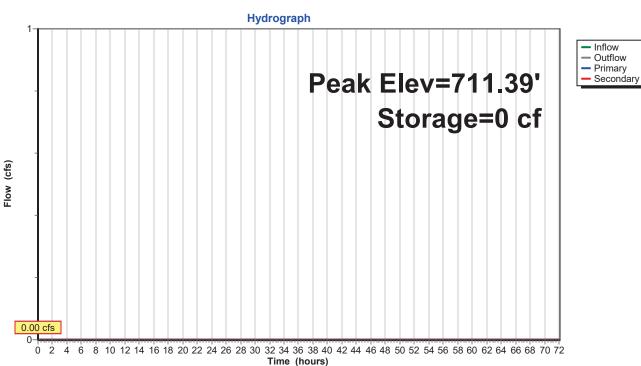
Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=711.39' (Free Discharge) **1=Culvert** (Passes 0.00 cfs of 0.66 cfs potential flow)

324

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=711.39' (Free Discharge) 2=Beehive (Controls 0.00 cfs)

Bull Run Filtration Rain Garden 11A

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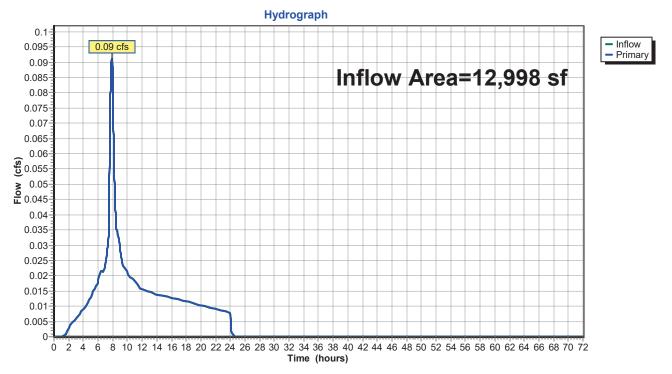


Pond 15P: RG 11A Section 3

Summary for Link 13L: Outflow

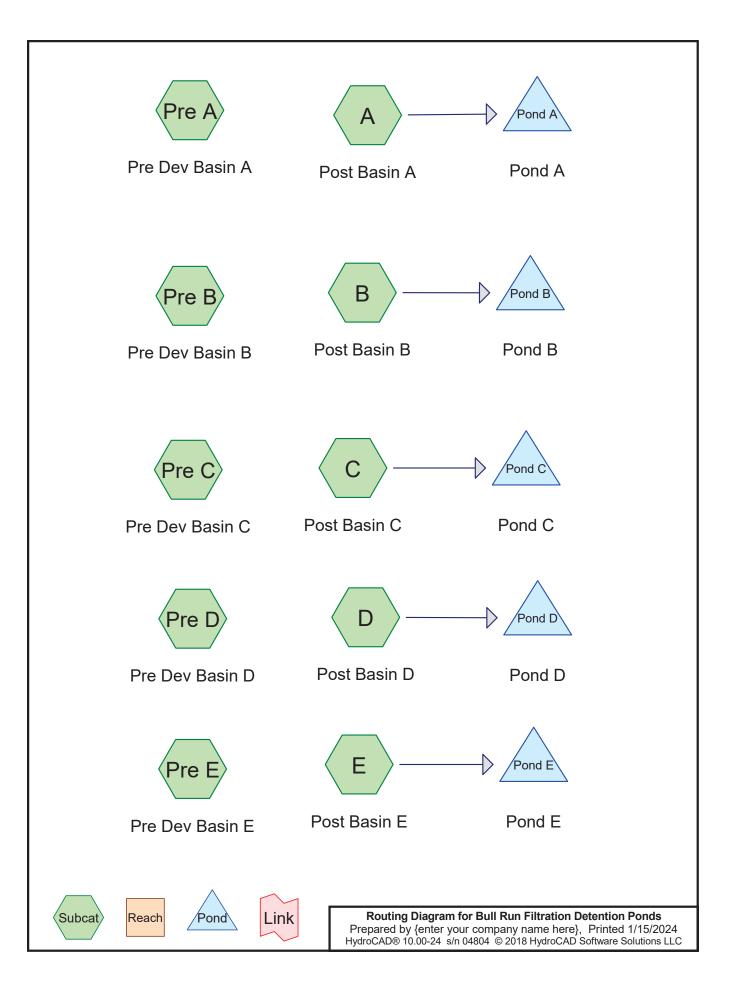
Inflow Are	a =	12,998 sf,	100.00% Impervious,	Inflow Depth = 1.18"	for WQ event
Inflow	=	0.09 cfs @	7.89 hrs, Volume=	1,280 cf	
Primary	=	0.09 cfs @	7.89 hrs, Volume=	1,280 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



Link 13L: Outflow

Attachment E: Hydrologic Analysis of Pre- and Post-Developed Conditions (For Flow Control)



Bull Run Filtration Detention Ponds

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Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
17,049	98	(E)
1,780,163	80	>75% Grass cover, Good, HSG D (A, B, C, D, E)
572,761	98	Impervious Area (A, B, C, D, E)
2,369,973	82	Row crops, SR + CR, Good, HSG C (Pre A, Pre B, Pre C, Pre D, Pre E)
4,739,946	83	TOTAL AREA

Flow (cfs)

2

1

0

0 5

Summary for Subcatchment A: Post Basin A

Runoff = 3.52 cfs @ 7.92 hrs, Volume= 53,630 cf, Depth= 1.84"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

Area (sf)	CN	Description						
* 176,142	98	Impervious	Area					
173,326	80	>75% Grass		ood, HSG D				
349,468	89	Weighted A	verage					
173,326	80	49.60% Per						
176,142	98	50.40% Imp	ervious Are	ea				
		-						
Tc Lengt			Capacity	Description				
(min) (feet	:) (ft/f	t) (ft/sec)	(cfs)					
5.0				Direct Entry,				
		Sı	ubcatchm	nent A: Post	Basin	Α		
			Hydro	graph				
- 3.52 cfs	5							- Runoff
						Туре	IA 24-hr	
3				2.	Year	Rainfa	ll=2.80"	

10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130

Time (hours)

Runoff Area=349,468 sf

Runoff Depth=1.84"

Tc=5.0 min CN=80/98

Runoff Volume=53,630 cf

Bull Run Filtration Detention Ponds	
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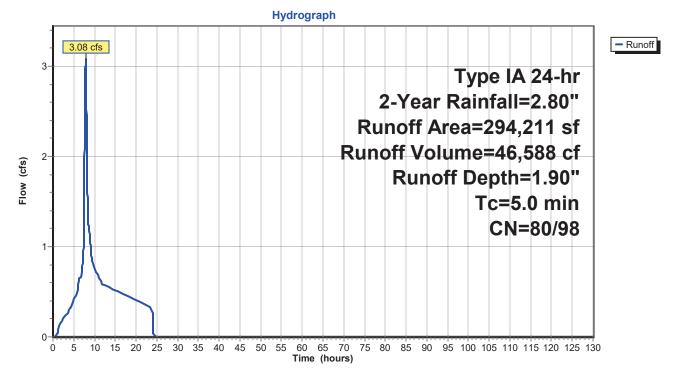
Summary for Subcatchment B: Post Basin B

Runoff = 3.08 cfs @ 7.91 hrs, Volume= 46,588 cf, Depth= 1.90"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

	Area (sf)	CN	Description		
*	160,056	98	Impervious	Area	
	134,155	80	>75% Gras	s cover, Go	bod, HSG D
	294,211	90	Weighted A	verage	
	134,155	80	45.60% Per	vious Area	
	160,056	98	54.40% Imp	ervious Ar	ea
To (min	5	Slop (ft/ft	,	Capacity (cfs)	Description
5.0)				Direct Entry,

Subcatchment B: Post Basin B



Summary for Subcatchment C: Post Basin C

Runoff = 4.78 cfs @ 7.98 hrs, Volume= 79,309 cf, Depth= 1.26"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

*	Area (sf) 82,047 672,290 754,337 672,290 82,047		98 80 > 82 \ 80 8	Veighted A 39.12% Pe	Area s cover, Go verage rvious Area	
(r	Tc <u>min)</u> 5.0	Length (feet)	96 Slope (ft/ft)	Velocity (ft/sec)	(cfs)	Description Direct Entry,
				S		nent C: Post Basin C
Flow (cfs)		4.78 cfs			Hydro	Type IA 24-hr 2-Year Rainfall=2.80" Runoff Area=754,337 sf Runoff Volume=79,309 cf Runoff Depth=1.26" Tc=5.0 min CN=80/98
	- 0- 0	5 10 1	5 20 25	30 35 40		0 65 70 75 80 85 90 95 100 105 110 115 120 125 130 me (hours)

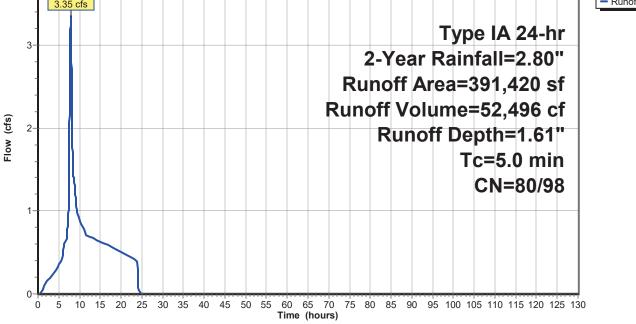
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Summary for Subcatchment D: Post Basin D

Runoff = 3.35 cfs @ 7.94 hrs, Volume= 52,496 cf, Depth= 1.61"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

_	A	rea (sf)	CN	Description							
*	1	35,356	98	98 Impervious Area							
_	2	56,064	80	>75% Gras	s cover, Go	ood, HSG D					
	3	91,420	86	Weighted A	verage						
	2	56,064	80	65.42% Per	vious Area						
	1	35,356	98	34.58% Imp	ervious Ar	ea					
	Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)										
	5.0					Direct Entry	,				
	Subcatchment D: Post Basin D Hydrograph										
	Ţ										
	-	3.35 cfs								- Runoff	



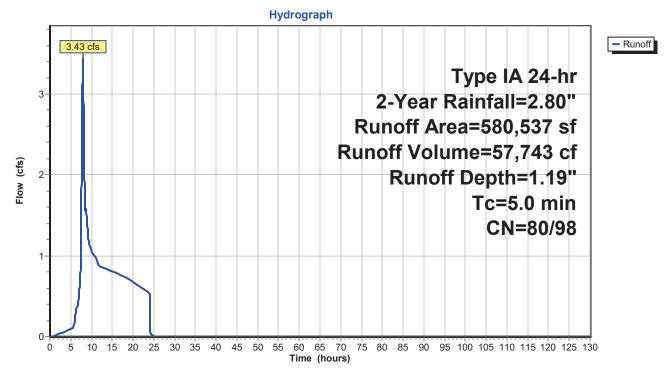
Summary for Subcatchment E: Post Basin E

Runoff = 3.43 cfs @ 7.99 hrs, Volume= 57,743 cf, Depth= 1.19"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

Area (sf)	CN	Description			
19,160	98	Impervious Are	а		
544,328	80	>75% Grass co	ver, Go	bod, HSG D	
17,049	98	98			
580,537 81 Weighted Average			age		
544,328	80	93.76% Perviou	us Area	l	
36,209	98	6.24% Impervic	ous Area	a	
Tc Length		,		Description	
min) (feet)	(ft/	t) (ft/sec)	(cfs)		
5.0				Direct Entry,	
	19,160 544,328 17,049 580,537 544,328 36,209 Tc Length min) (feet)	19,160 98 544,328 80 17,049 98 580,537 81 544,328 80 36,209 98 Tc Length Slop min) (feet) (ft/f	19,160 98 Impervious Are 544,328 80 >75% Grass co 17,049 98 580,537 81 Weighted Avera 544,328 80 93.76% Perviou 544,328 80 93.76% Perviou 36,209 98 6.24% Imperviou Tc Length Slope Velocity Ca min) (feet) (ft/ft) (ft/sec)	19,16098Impervious Area544,32880>75% Grass cover, Go17,04998580,53781Weighted Average544,3288093.76% Pervious Area36,209986.24% Impervious AreaTcLengthSlopeVelocityTcLengthSlopeVelocitymin)(feet)(ft/ft)(ft/sec)	

Subcatchment E: Post Basin E



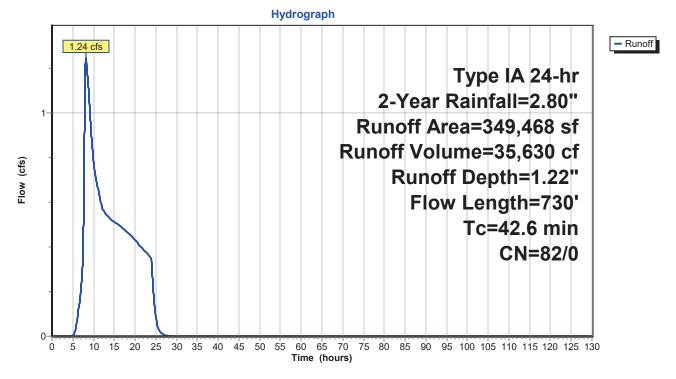
Summary for Subcatchment Pre A: Pre Dev Basin A

Runoff = 1.24 cfs @ 8.19 hrs, Volume= 35,630 cf, Depth= 1.22"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

	A	rea (sf)	CN [Description					
	3	49,468	82 F	Row crops,	SR + CR,	Good, HSG C			
	3	49,468	82 1	100.00% Pervious Area					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
-	36.8	300	0.0100	0.14		Sheet Flow,			
	5.8	430	0.0190	1.24		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps			
	42.6	730	Total						

Subcatchment Pre A: Pre Dev Basin A



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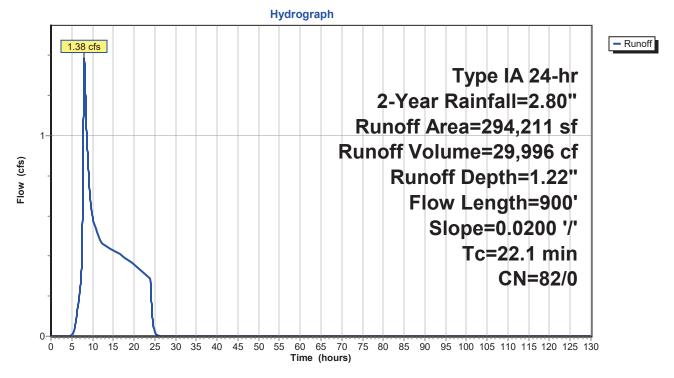
Summary for Subcatchment Pre B: Pre Dev Basin B

Runoff = 1.38 cfs @ 8.01 hrs, Volume= 29,996 cf, Depth= 1.22"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

_	A	rea (sf)	CN I	Description					
	2	94,211	82 I	Row crops,	SR + CR,	Good, HSG C			
	2	94,211	82 ⁻	100.00% Pervious Area					
	Tc Length (min) (feet)		Slope (ft/ft)	,	Capacity (cfs)	Description			
-	11.6	100	0.0200	0.14		Sheet Flow,			
_	10.5	800	0.0200	1.27		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps			
	22.1	900	Total						

Subcatchment Pre B: Pre Dev Basin B



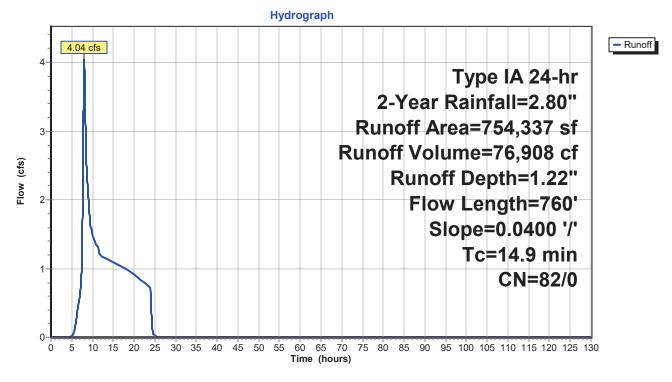
Summary for Subcatchment Pre C: Pre Dev Basin C

Runoff = 4.04 cfs @ 8.00 hrs, Volume= 76,908 cf, Depth= 1.22"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

_	A	rea (sf)	CN E	Description					
	7	54,337	82 F	Row crops,	SR + CR,	Good, HSG C			
_	7	54,337	82 1	100.00% Pervious Area					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
_	8.8	100	0.0400	0.19		Sheet Flow,			
	6.1	660	0.0400	1.80		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps			
	14.9	760	Total						

Subcatchment Pre C: Pre Dev Basin C



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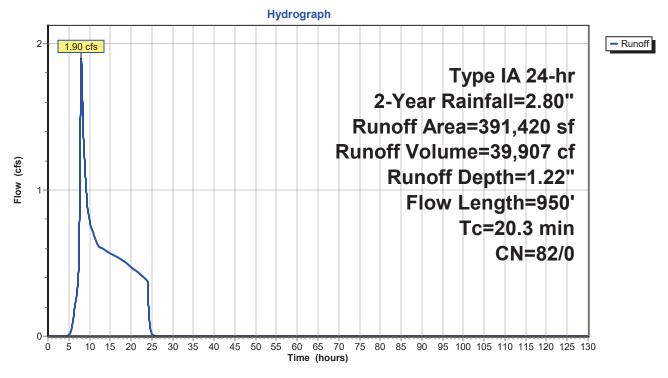
Summary for Subcatchment Pre D: Pre Dev Basin D

Runoff = 1.90 cfs @ 8.01 hrs, Volume= 39,907 cf, Depth= 1.22"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

_	A	rea (sf)	CN E	Description		
	3	91,420	82 F	Row crops,	SR + CR,	Good, HSG C
	3	91,420	82 1	00.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	11.6	100	0.0200	0.14		Sheet Flow,
	8.7	850	0.0330	1.63		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
	20.3	950	Total			

Subcatchment Pre D: Pre Dev Basin D



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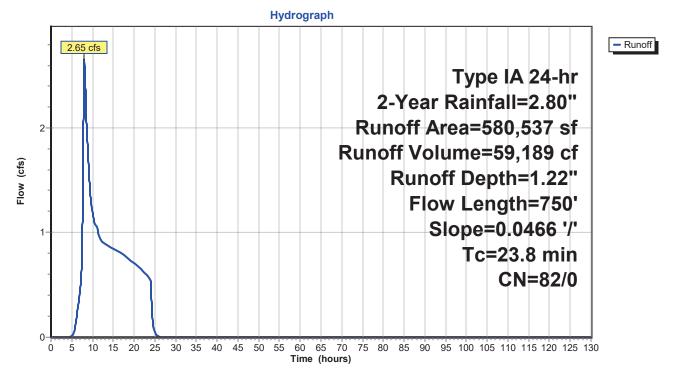
Summary for Subcatchment Pre E: Pre Dev Basin E

Runoff = 2.65 cfs @ 8.01 hrs, Volume= 59,189 cf, Depth= 1.22"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

	A	rea (sf)	CN [Description		
580,537 82 Row crops, SR + CR, Good, HSG C						Good, HSG C
580,537 82 100.00% Pervious Area				100.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	19.9	300	0.0466	0.25		Sheet Flow,
	3.9	450	0.0466	1.94		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
	23.8	750	Total			

Subcatchment Pre E: Pre Dev Basin E



Summary for Pond Pond A: Pond A

Inflow Area	a =	349,468 sf, 50.40% Impervious, Inflow Depth = 1.84" for 2-Year event
Inflow	=	3.52 cfs @ 7.92 hrs, Volume= 53,630 cf
Outflow	=	0.61 cfs @ 14.82 hrs, Volume= 53,630 cf, Atten= 83%, Lag= 414.2 min
Primary	=	0.61 cfs @ 14.82 hrs, Volume= 53,630 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 709.36' @ 14.82 hrs Surf.Area= 8,132 sf Storage= 13,290 cf

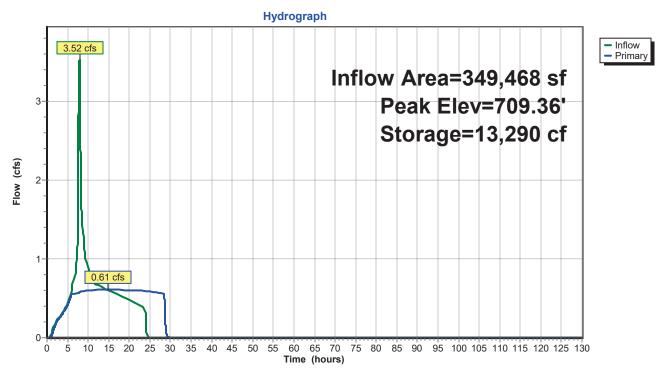
Plug-Flow detention time= 246.7 min calculated for 53,626 cf (100% of inflow) Center-of-Mass det. time= 246.7 min (965.4 - 718.7)

Volume	Invert	t Avail.Sto	rage Storage D	ge Storage Description			
#1	707.50	' 70,55	55 cf Custom S	Stage Data (Prisn	natic) Listed below (Recalc)		
_	-	5 A		0.01			
Elevatio		urf.Area	Inc.Store	Cum.Store			
(fee	t)	(sq-ft)	(cubic-feet)	(cubic-feet)			
707.5	0	6,107	0	0			
708.0	0	6,714	3,205	3,205			
709.0	0	7,742	7,228	10,433			
710.0	0	8,825	8,284	18,717			
711.0	0	9,967	9,396	28,113			
712.0	0	11,161	10,564	38,677			
713.0	0	12,412	11,787	50,463			
714.0	0	13,717	13,065	63,528			
714.5	0	14,392	7,027	70,555			
Device	Routing	Invert	Outlet Devices				
#1	Primary	698.43'	18.0" Round C	Culvert			
	2		L= 138.0' CMF	^{>} , square edge h	eadwall, Ke= 0.500		
					6.24' S= 0.0159 '/' Cc= 0.900		
			n= 0.013, Flow	/ Area= 1.77 sf			
#2	Device 1	696.43'	,		.620 Limited to weir flow at low heads		
#3	Device 1	709.40'	16.0" W x 3.0"	H Vert. Orifice/G	rate C= 0.620		
	Primary OutFlow Max=0.61 cfs @ 14.82 hrs HW=709.36' (Free Discharge)						

1=Culvert (Passes 0.61 cfs of 24.10 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.61 cfs @ 16.45 fps)

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Pond Pond A: Pond A

Summary for Pond Pond B: Pond B

Inflow Area	a =	294,211 sf, 54.40% Impervious, Inflow Depth = 1.90" for 2-Year event	
Inflow	=	3.08 cfs @ 7.91 hrs, Volume= 46,588 cf	
Outflow	=	0.67 cfs @ 11.01 hrs, Volume= 46,588 cf, Atten= 78%, Lag= 186.0 mi	n
Primary	=	0.67 cfs @ 11.01 hrs, Volume= 46,588 cf	

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 709.12' @ 11.01 hrs Surf.Area= 5,248 sf Storage= 9,226 cf

Plug-Flow detention time= 143.1 min calculated for 46,585 cf (100% of inflow) Center-of-Mass det. time= 143.1 min (856.4 - 713.3)

Volume	Inve	rt Avail.Sto	rage Storage Description		
#1	707.00)' 36,8'	13 cf Custom	Stage Data (Pr	ismatic) Listed below (Recalc)
- :					
Elevatio		Surf.Area	Inc.Store	Cum.Store	
(feet	:)	(sq-ft)	(cubic-feet)	(cubic-feet)	
707.0	0	3,493	0	0	
708.0	0	4,288	3,891	3,891	
709.0	0	5,139	4,714	8,604	
710.0	0	6,046	5,593	14,197	
711.0	0	7,010	6,528	20,725	
712.0	0	8,030	7,520	28,245	
713.0	0	9,107	8,569	36,813	
Device	Routing	Invert	Outlet Device	S	
#1	Primary	705.04'	18.0" Round	Culvert	
	-		L= 339.0' CN	/IP, square edge	e headwall, Ke= 0.500
					701.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flo	w Area= 1.77 st	-
#2	Device 1	703.04'	,	rifice/Grate C:	
#3	Device 1	709.15'	14.0" W x 3.0	" H Vert. Orifice	e/Grate C= 0.620

Primary OutFlow Max=0.67 cfs @ 11.01 hrs HW=709.12' (Free Discharge)

-1=Culvert (Passes 0.67 cfs of 12.49 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.67 cfs @ 10.05 fps)

3=Orifice/Grate (Controls 0.00 cfs)

Hydrograph 3.08 cfs Inflow Primary 3-Inflow Area=294,211 sf Peak Elev=709.12' Storage=9,226 cf 2 Flow (cfs) 1 0.67 cfs 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 Ó Time (hours)

Pond Pond B: Pond B

Summary for Pond Pond C: Pond C

Inflow Area	a =	754,337 sf, 10.88% Impervious, Inflow Depth > 7.22" for 2-Year event	
Inflow	=	5.58 cfs @ 7.98 hrs, Volume= 453,738 cf, Incl. 0.80 cfs Base Flow	
Outflow	=	1.86 cfs @ 15.66 hrs, Volume= 453,393 cf, Atten= 67%, Lag= 460.7 n	nin
Primary	=	1.86 cfs @ 15.66 hrs, Volume= 453,393 cf	

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 704.82' @ 15.66 hrs Surf.Area= 20,381 sf Storage= 15,943 cf

Plug-Flow detention time= 38.2 min calculated for 453,388 cf (100% of inflow) Center-of-Mass det. time= 34.8 min (3,392.8 - 3,358.0)

Volume	Invert	Avail.Stor	rage Storage	e Storage Description		
#1	704.00'	95,41	6 cf Custon	n Stage Data (Pr	ismatic) Listed below (Recalc)	
	-					
Elevatio		rf.Area	Inc.Store	Cum.Store		
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)		
704.0	00	18,277	0	0		
705.0	00	20,828	19,553	19,553		
706.0	00	23,671	22,250	41,802		
707.0	00	26,743	25,207	67,009		
708.0	00	30,070	28,407	95,416		
Device	Routing	Invert	Outlet Device	es		
#1	Primary	697.90'	12.0" Round	d Culvert		
	5		L= 53.0' CN	IP, square edge	headwall, Ke= 0.500	
					695.55' S= 0.0443 '/' Cc= 0.900	
			n= 0.013. Fl	ow Area= 0.79 s	f	
#2	Device 1	693.90'	,		= 0.620 Limited to weir flow at low heads	
#3	Device 1	704.85'	16.0" W x 3.0)" H Vert. Orifice	e/Grate C= 0.620	
	o (=)					

Primary OutFlow Max=1.86 cfs @ 15.66 hrs HW=704.82' (Free Discharge)

-**1=Culvert** (Passes 1.86 cfs of 9.59 cfs potential flow)

-2=Orifice/Grate (Orifice Controls 1.86 cfs @ 13.09 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

Hydrograph 6-5.58 cfs Inflow Primary Inflow Area=754,337 sf 5-Peak Elev=704.82' Storage=15,943 cf 4-Flow (cfs) 3-1.86 cfs 2-1 0-5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 Ó Time (hours)

Pond Pond C: Pond C

Summary for Pond Pond D: Pond D

Inflow Area	a =	391,420 sf,	34.58% Impervious,	Inflow Depth = 1.61" for 2-Year event
Inflow	=	3.35 cfs @	7.94 hrs, Volume=	52,496 cf
Outflow	=	0.94 cfs @	9.75 hrs, Volume=	52,496 cf, Atten= 72%, Lag= 108.9 min
Primary	=	0.94 cfs @	9.75 hrs, Volume=	52,496 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 694.57' @ 9.75 hrs Surf.Area= 5,958 sf Storage= 8,181 cf

Plug-Flow detention time= 86.5 min calculated for 52,492 cf (100% of inflow) Center-of-Mass det. time= 86.5 min (830.5 - 744.0)

Volume	Inve	rt Avail.Sto	age Storage Description		
#1	693.00)' 45,10	06 cf Custom S	tage Data (Prismati	i c) Listed below (Recalc)
Elevatio	n S	Surf.Area	Inc.Store	Cum.Store	
(feet	:)	(sq-ft)	(cubic-feet)	(cubic-feet)	
693.0	0	4,453	0	0	
694.0	0	5,388	4,921	4,921	
695.0	0	6,380	5,884	10,805	
696.0	0	7,428	6,904	17,709	
697.0	0	8,533	7,981	25,689	
698.0		9,694	9,114	34,803	
699.0	0	10,912	10,303	45,106	
Device	Routing	Invert	Outlet Devices		
#1	Primary	692.50'	12.0" Round C	ulvert	
			L= 121.0' CMF	, square edge head	wall, Ke= 0.500
			Inlet / Outlet Inv	vert= 692.50' / 688.9	7' S= 0.0292 '/' Cc= 0.900
			n= 0.013, Flow	Area= 0.79 sf	
#2	Device 1	690.50'	4.9" Horiz. Orif	ice/Grate C= 0.620	0 Limited to weir flow at low heads
#3	Device 1	694.60'	15.0" W x 3.0"	H Vert. Orifice/Grate	e C= 0.620

Primary OutFlow Max=0.94 cfs @ 9.75 hrs HW=694.57' (Free Discharge)

-1=Culvert (Passes 0.94 cfs of 4.75 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.94 cfs @ 7.17 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

Hydrograph 3.35 cfs Inflow Primary Inflow Area=391,420 sf 3-Peak Elev=694.57' Storage=8,181 cf Flow (cfs) 2 0.94 cfs 1 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 Ó Time (hours)

Pond Pond D: Pond D

Summary for Pond Pond E: Pond E

Inflow Area =	580,537 sf,	6.24% Impervious,	Inflow Depth = 1.19" for 2-Year event
Inflow =	3.43 cfs @	7.99 hrs, Volume=	57,743 cf
Outflow =	1.30 cfs @	9.08 hrs, Volume=	57,731 cf, Atten= 62%, Lag= 65.5 min
Primary =	1.30 cfs @	9.08 hrs, Volume=	57,731 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 713.96' @ 9.08 hrs Surf.Area= 9,470 sf Storage= 8,316 cf

Plug-Flow detention time= 111.2 min calculated for 57,727 cf (100% of inflow) Center-of-Mass det. time= 111.5 min (925.4 - 813.9)

Volume	Inve	rt Avail.Sto	rage Storage	e Storage Description			
#1	713.0	0' 31,30	08 cf Custom	n Stage Data (Pri	ismatic) Listed below (Recalc)		
Elevatio	on s	Surf.Area	Inc.Store	Cum.Store			
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)			
713.0	00	7,916	0	0			
714.0	00	9,540	8,728	8,728			
715.0	00	11,265	10,403	19,131			
716.0)0	13,090	12,178	31,308			
Device	Routing	Invert	Outlet Device	s			
#1	Primary	713.00'	15.0" Round	I Culvert			
	-		L= 44.0' CM	IP, square edge l	headwall, Ke= 0.500		
			Inlet / Outlet Invert= 713.00' / 712.80' S= 0.0045 '/' Cc= 0.900				
			n= 0.013, Flo	ow Area= 1.23 sf			
#2	Device 1	711.00'	7.0" Horiz. O	rifice/Grate C=	= 0.620 Limited to weir flow at low heads		
#3	Device 1	713.95'	16.0" W x 3.0	" H Vert. Orifice	e/Grate C= 0.620		
Primary	Primary OutFlow Max=1.30 cfs @ 9.08 hrs HW=713.96' (Free Discharge)						

-1=Culvert (Passes 1.30 cfs of 2.49 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.30 cfs @ 4.87 fps)

-3=Orifice/Grate (Orifice Controls 0.00 cfs @ 0.27 fps)

Hydrograph 3.43 cfs Inflow Primary Inflow Area=580,537 sf 3-Peak Elev=713.96' Storage=8,316 cf Flow (cfs) 2 1.30 cfs 0 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 5 Ó Time (hours)

Pond Pond E: Pond E

Summary for Subcatchment A: Post Basin A

Runoff = 4.59 cfs @ 7.91 hrs, Volume= 68,979 cf, Depth= 2.37"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

*	Area (sf) 176,142 173,326 349,468 173,326 176,142	98 lr 80 > 89 V 80 4	Description mpervious 75% Gras Veighted A 9.60% Per 0.40% Imp		
Tc (min) 5.0		Slope (ft/ft)	Velocity (ft/sec)		Description Direct Entry,
			Si		nent A: Post Basin A
5- 	4.59 cfs	5 20 25	30 35 40		Type IA 24-hr 5-Year Rainfall=3.40" Runoff Area=349,468 sf Runoff Volume=68,979 cf Runoff Depth=2.37" Tc=5.0 min CN=80/98

Bull Run Fi	Itration De	etention	Ponds	
Prepared by	{enter your	company	y name here]	}

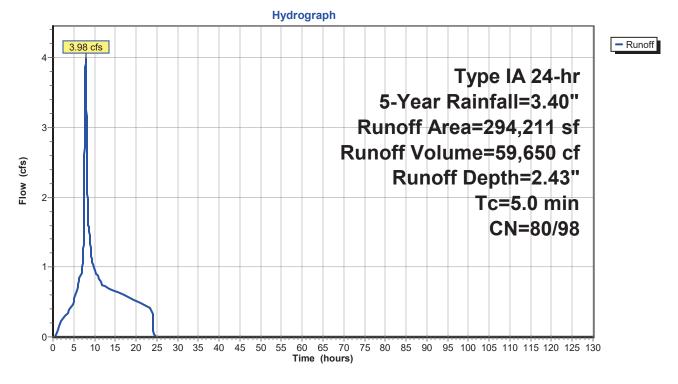
Summary for Subcatchment B: Post Basin B

Runoff = 3.98 cfs @ 7.91 hrs, Volume= 59,650 cf, Depth= 2.43"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

_	A	rea (sf)	CN	Description					
*	1	60,056	98	Impervious	Area				
_	1	34,155	80	>75% Gras	>75% Grass cover, Good, HSG D				
	2	94,211	90 Weighted Average						
134,155 80 45.60% Pervious Area					vious Area				
	160,056 98 54.40% Impervious Are			54.40% Imp	pervious Ar	ea			
	Tc (min)	Length (feet)	Slop (ft/ft		Capacity (cfs)	Description			
	5.0					Direct Entry,			

Subcatchment B: Post Basin B

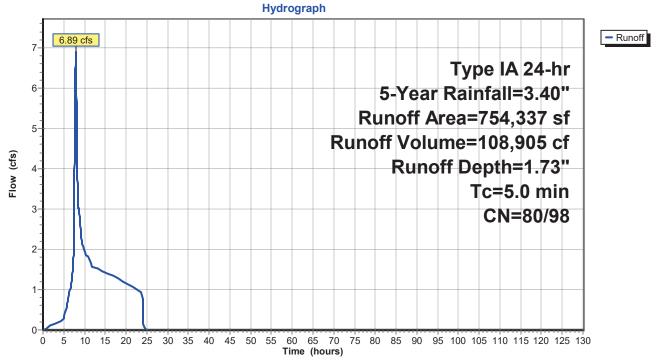


Summary for Subcatchment C: Post Basin C

Runoff = 6.89 cfs @ 7.97 hrs, Volume= 108,905 cf, Depth= 1.73"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

_	Area (sf)	CN Description								
*	82,047	98 Impervious Area								
	672,290	80 >75% Grass cover, Good, HSG D	_							
	754,337	82 Weighted Average								
	672,290	80 89.12% Pervious Area								
	82,047	98 10.88% Impervious Area								
	Tc Length (min) (feet)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)								
	5.0	Direct Entry,								
	Subcatchment C: Post Basin C									



Summary for Subcatchment D: Post Basin D

Runoff = 4.50 cfs @ 7.93 hrs, Volume= 68,953 cf, Depth= 2.11"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

*	Area (sf)CNDescription135,35698Impervious Area256,06480>75% Grass cover, Good, HSG D391,42086Weighted Average256,0648065.42% Pervious Area135,3569834.58% Impervious AreaTcLengthSlopeVelocityCapacityDescription								
(min) (feet)	(ft/ft)	(ft/sec)	Capacity (cfs)	Description				
5.0	C				Direct Entry,				
			Si	ubcatchm	nent D: Post Basin D				
	5-			Hydro	ograph				
Flow (cfs)	4.50 cfs 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	5 20 25	30 35 40		Type IA 24-hr 5-Year Rainfall=3.40" Runoff Area=391,420 sf Runoff Volume=68,953 cf Runoff Depth=2.11" Tc=5.0 min CN=80/98				

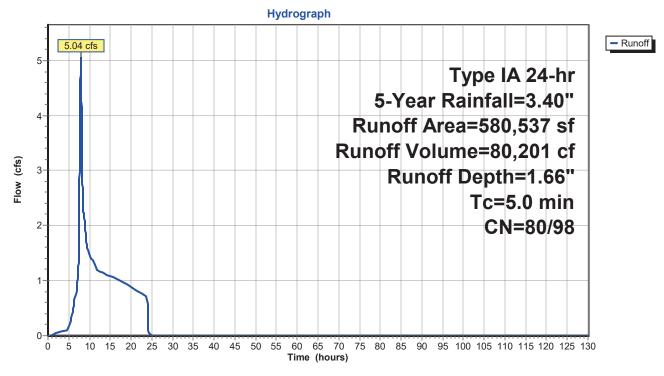
Summary for Subcatchment E: Post Basin E

Runoff = 5.04 cfs @ 7.97 hrs, Volume= 80,201 cf, Depth= 1.66"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

Area (sf)	CN	Description		
19,160	98	Impervious Area		
544,328	80	>75% Grass cover, Good, HSG D		
17,049	98			
580,537	81	Weighted Average		
544,328	80	93.76% Pervious Area		
36,209	98	6.24% Impervious Area		
Tc Length	Slop			
(min) (feet)	(ft/	/ft) (ft/sec) (cfs)		
5.0		Direct Entry,		
	19,160 544,328 17,049 580,537 544,328 36,209 Tc Length (min) (feet)	19,160 98 544,328 80 17,049 98 580,537 81 544,328 80 36,209 98 Tc Length Slop (min) (feet) (ft/		

Subcatchment E: Post Basin E



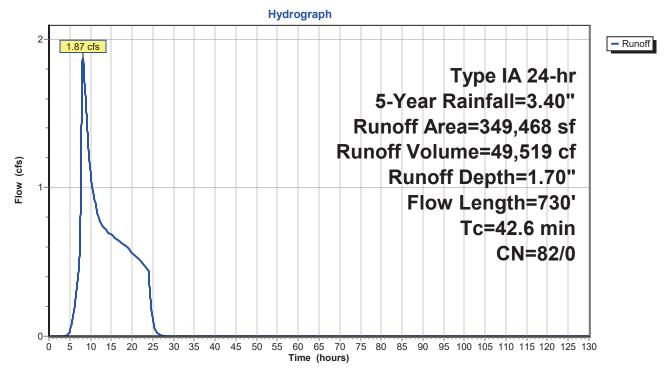
Summary for Subcatchment Pre A: Pre Dev Basin A

Runoff = 1.87 cfs @ 8.15 hrs, Volume= 49,519 cf, Depth= 1.70"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

	A	rea (sf)	CN E	Description				
349,468 82 Row crops, SR + CR, Good, HSG C								
	3	49,468	82 1	2 100.00% Pervious Area				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
	36.8	300	0.0100	0.14	· · ·	Sheet Flow,		
	5.8	430	0.0190	1.24		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps		
	42.6	730	Total					

Subcatchment Pre A: Pre Dev Basin A



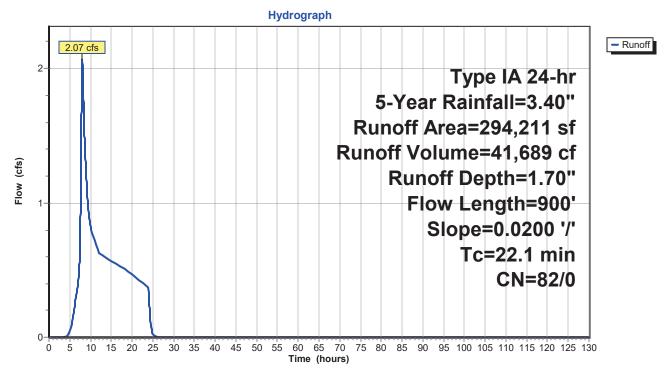
Summary for Subcatchment Pre B: Pre Dev Basin B

Runoff = 2.07 cfs @ 8.01 hrs, Volume= 41,689 cf, Depth= 1.70"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

_	A	rea (sf)	CN E	Description				
	2	94,211	82 F	Row crops,	SR + CR,	Good, HSG C		
294,211			82 1	100.00% Pervious Area				
Tc Length Slope Velocity Capacity (min) (feet) (ft/ft) (ft/sec) (cfs)		Capacity (cfs)	Description					
-	11.6	100	0.0200	0.14		Sheet Flow,		
	10.5	800	0.0200	1.27		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps		
	22.1	900	Total					

Subcatchment Pre B: Pre Dev Basin B



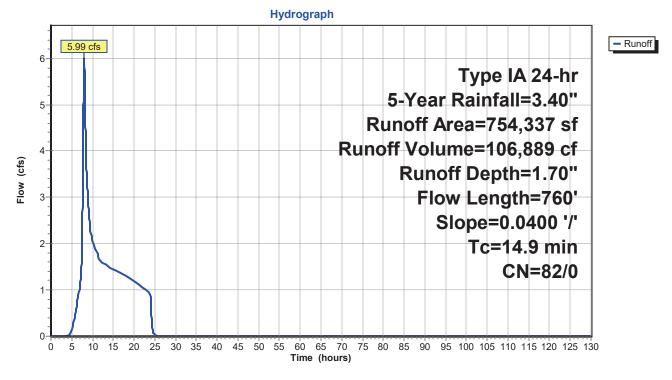
Summary for Subcatchment Pre C: Pre Dev Basin C

Runoff = 5.99 cfs @ 8.00 hrs, Volume= 106,889 cf, Depth= 1.70"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

_	A	rea (sf)	CN E	Description		
	7	54,337	Good, HSG C			
_	7	54,337	82 1	00.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	8.8	100	0.0400	0.19		Sheet Flow,
	6.1	660	0.0400	1.80		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
	14.9	760	Total			

Subcatchment Pre C: Pre Dev Basin C



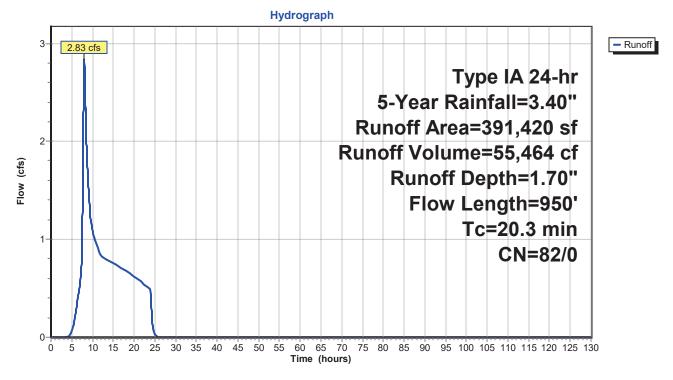
Summary for Subcatchment Pre D: Pre Dev Basin D

Runoff = 2.83 cfs @ 8.01 hrs, Volume= 55,464 cf, Depth= 1.70"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

Α	vrea (sf)	CN E	Description				
3	391,420	Good, HSG C					
3	391,420	82 1	100.00% Pervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
11.6	100	0.0200	0.14		Sheet Flow,		
8.7	850	0.0330	1.63		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps		
20.3	950	Total					

Subcatchment Pre D: Pre Dev Basin D



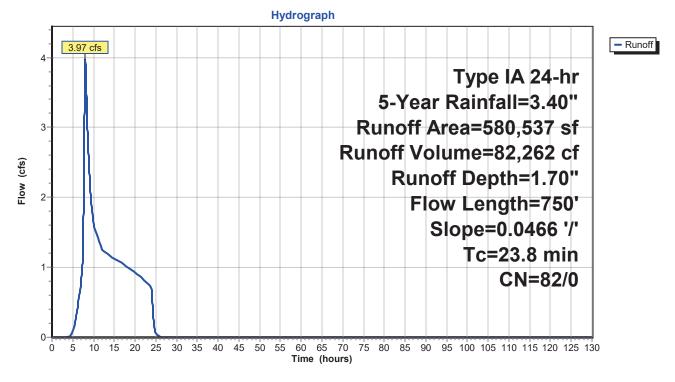
Summary for Subcatchment Pre E: Pre Dev Basin E

Runoff = 3.97 cfs @ 8.01 hrs, Volume= 82,262 cf, Depth= 1.70"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

_	A	rea (sf)	CN [Description			
580,537 82 Row crops, SR + CR, Good, HSG C							
	580,537 82 100.00% Pervious Area					a	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
-	19.9	300	0.0466	0.25		Sheet Flow,	
_	3.9	450	0.0466	1.94		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps	
	23.8	750	Total				

Subcatchment Pre E: Pre Dev Basin E



Summary for Pond Pond A: Pond A

Inflow Area	a =	349,468 sf,	50.40% Impervious,	Inflow Depth = 2.37" for 5-Year event
Inflow	=	4.59 cfs @	7.91 hrs, Volume=	68,979 cf
Outflow	=	1.18 cfs @	9.84 hrs, Volume=	68,979 cf, Atten= 74%, Lag= 115.6 min
Primary	=	1.18 cfs @	9.84 hrs, Volume=	68,979 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 709.65' @ 9.84 hrs Surf.Area= 8,451 sf Storage= 15,735 cf

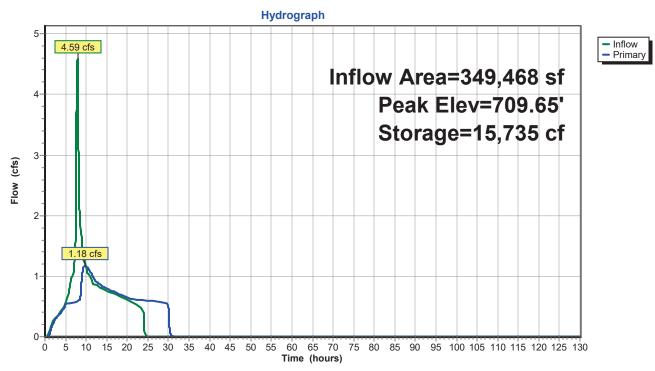
Plug-Flow detention time= 240.8 min calculated for 68,979 cf (100% of inflow) Center-of-Mass det. time= 240.8 min (953.5 - 712.7)

Volume	Inve	ert Avail.Sto	rage Storage D	escription					
#1	707.5	0' 70,5	55 cf Custom S	Stage Data (Prisr	natic) Listed below (Recalc)				
		o ()							
Elevatio		Surf.Area	Inc.Store	Cum.Store					
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)					
707.5	50	6,107	0	0					
708.0	00	6,714	3,205	3,205					
709.0	00	7,742	7,228	10,433					
710.0	00	8,825	8,284	18,717					
711.0	00	9,967	9,396	28,113					
712.0	00	11,161	10,564	38,677					
713.0	00	12,412	11,787	50,463					
714.0	00	13,717	13,065	63,528					
714.5	50	14,392	7,027	70,555					
Device	Routing	Invert	Outlet Devices						
#1	Primary	698.43'	18.0" Round C	Culvert					
			L= 138.0' CMF	P, square edge h	eadwall, Ke= 0.500				
			Inlet / Outlet Inv	/ert= 698.43' / 69	6.24' S= 0.0159 '/' Cc= 0.900				
			n= 0.013, Flow	Area= 1.77 sf					
#2	Device 1	696.43'			0.620 Limited to weir flow at low heads				
#3	Device 1	709.40'	16.0" W x 3.0"	H Vert. Orifice/G	irate C= 0.620				
	Primary OutFlow Max=1.18 cfs @ 9.84 hrs HW=709.65' (Free Discharge)								

___1=Culvert (Passes 1.18 cfs of 24.40 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.61 cfs @ 16.67 fps)

-3=Orifice/Grate (Orifice Controls 0.57 cfs @ 1.70 fps)



Pond Pond A: Pond A

Summary for Pond Pond B: Pond B

Inflow Area =	294,211 sf,	54.40% Impervious,	Inflow Depth = 2.43" for 5-Year event
Inflow =	3.98 cfs @	7.91 hrs, Volume=	59,650 cf
Outflow =	1.34 cfs @	8.97 hrs, Volume=	59,650 cf, Atten= 66%, Lag= 63.7 min
Primary =	1.34 cfs @	8.97 hrs, Volume=	59,650 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 709.48' @ 8.97 hrs Surf.Area= 5,573 sf Storage= 11,166 cf

Plug-Flow detention time= 157.4 min calculated for 59,645 cf (100% of inflow) Center-of-Mass det. time= 157.4 min (864.9 - 707.5)

Volume	Inver	t Avail.Sto	rage Storage D	Description	
#1	707.00)' 36,81	13 cf Custom S	Stage Data (Pri	ismatic) Listed below (Recalc)
E 1					
Elevatio		Surf.Area	Inc.Store	Cum.Store	
(fee	t)	(sq-ft)	(cubic-feet)	(cubic-feet)	
707.0	0	3,493	0	0	
708.0	0	4,288	3,891	3,891	
709.0	0	5,139	4,714	8,604	
710.0	0	6,046	5,593	14,197	
711.0	0	7,010	6,528	20,725	
712.0	0	8,030	7,520	28,245	
713.0	0	9,107	8,569	36,813	
Device	Routing	Invert	Outlet Devices		
#1	Primary	705.04'	18.0" Round C	Culvert	
	2		L= 339.0' CM	P. square edge	headwall, Ke= 0.500
					701.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow		
#2	Device 1	703.04'	3.5" Horiz. Ori		
#3	Device 1	709.15'	14.0" W x 3.0"	H vert. Orifice	/Grate C= 0.620

Primary OutFlow Max=1.34 cfs @ 8.97 hrs HW=709.48' (Free Discharge)

-**1=Culvert** (Passes 1.34 cfs of 12.86 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.70 cfs @ 10.48 fps)

-3=Orifice/Grate (Orifice Controls 0.64 cfs @ 2.21 fps)

Hydrograph 3.98 cfs Inflow Primary 4 Inflow Area=294,211 sf Peak Elev=709.48' 3-Storage=11,166 cf Flow (cfs) 2-1.34 cfs 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 Ó Time (hours)

Pond Pond B: Pond B

Summary for Pond Pond C: Pond C

Inflow Area	ı =	754,337 sf, 10.88% Impervious, Inflow Depth > 7.69" for 5-Year event
Inflow	=	7.69 cfs @ 7.97 hrs, Volume= 483,333 cf, Incl. 0.80 cfs Base Flow
Outflow	=	2.53 cfs @ 11.16 hrs, Volume= 482,989 cf, Atten= 67%, Lag= 191.7 min
Primary	=	2.53 cfs @ 11.16 hrs, Volume= 482,989 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 705.13' @ 11.16 hrs Surf.Area= 21,205 sf Storage= 22,341 cf

Plug-Flow detention time= 49.9 min calculated for 482,984 cf (100% of inflow) Center-of-Mass det. time= 46.5 min (3,244.5 - 3,198.0)

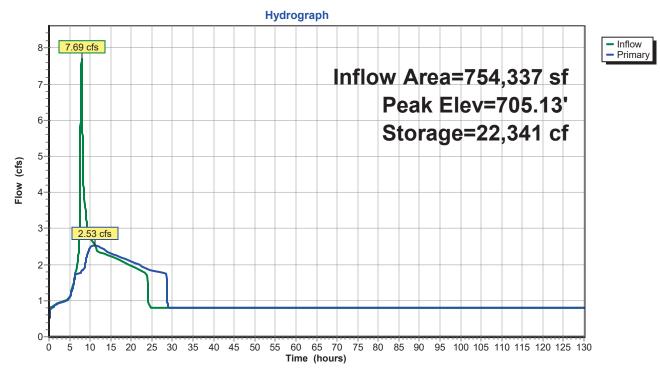
Volume	Inver	t Avail.Sto	rage Storag	ge Description	
#1	704.00)' 95,47	16 cf Custo	om Stage Data (Pr	rismatic) Listed below (Recalc)
Elevatio	n S	Surf.Area	Inc.Store	Cum.Store	
(fee		(sq-ft)	(cubic-feet)	(cubic-feet)	
704.0	00	18,277	0	0	
705.0	00	20,828	19,553	19,553	
706.0	00	23,671	22,250	41,802	
707.0	•	26,743	25,207	67,009	
708.0	00	30,070	28,407	95,416	
Device	Routing	Invert	Outlet Devi	ces	
#1	Primary	697.90'	12.0" Roui	nd Culvert	
			L= 53.0' C	MP, square edge	e headwall, Ke= 0.500
					/ 695.55' S= 0.0443 '/' Cc= 0.900
			,	low Area= 0.79 s	
#2	Device 1	693.90'			C= 0.620 Limited to weir flow at low heads
#3	Device 1	704.85'	16.0" W x 3	3.0" H Vert. Orifice	ce/Grate C= 0.620

Primary OutFlow Max=2.54 cfs @ 11.16 hrs HW=705.13' (Free Discharge)

-1=Culvert (Passes 2.54 cfs of 9.81 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.90 cfs @ 13.38 fps)

-3=Orifice/Grate (Orifice Controls 0.64 cfs @ 1.92 fps)



Pond Pond C: Pond C

Summary for Pond Pond D: Pond D

Inflow Area	a =	391,420 sf,	34.58% Impervious,	Inflow Depth = 2.11" for 5-Year event
Inflow	=	4.50 cfs @	7.93 hrs, Volume=	68,953 cf
Outflow	=	1.75 cfs @	8.79 hrs, Volume=	68,953 cf, Atten= 61%, Lag= 51.7 min
Primary	=	1.75 cfs @	8.79 hrs, Volume=	68,953 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 694.95' @ 8.79 hrs Surf.Area= 6,333 sf Storage= 10,505 cf

Plug-Flow detention time= 103.0 min calculated for 68,948 cf (100% of inflow) Center-of-Mass det. time= 103.0 min (839.2 - 736.1)

Volume	Inve	rt Avail.Sto	rage Storage D	escription	
#1	693.00	0' 45,10	06 cf Custom S	Stage Data (Pri	smatic) Listed below (Recalc)
Elevatio		Surf.Area	Inc.Store	Cum.Store	
(fee		(sq-ft)	(cubic-feet)	(cubic-feet)	
693.0	/	4,453	0	0	
694.0	00	5,388	4,921	4,921	
695.0		6,380	5,884	10,805	
696.0		7,428	6,904	17,709	
697.0		8,533	7,981	25,689	
698.0	-	9,694	9,114	34,803	
699.0	00	10,912	10,303	45,106	
Device	Routing	Invert	Outlet Devices		
#1	Primary	692.50'	12.0" Round C	Culvert	
			L= 121.0' CMF	⊃, square edge	headwall, Ke= 0.500
					688.97' S= 0.0292 '/' Cc= 0.900
			n= 0.013, Flow		
#2	Device 1	690.50'			= 0.620 Limited to weir flow at low heads
#3	Device 1	694.60'	15.0" W x 3.0"	H Vert. Orifice	/ Grate C= 0.620

Primary OutFlow Max=1.75 cfs @ 8.79 hrs HW=694.95' (Free Discharge)

-**1=Culvert** (Passes 1.75 cfs of 5.28 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.02 cfs @ 7.79 fps)

-3=Orifice/Grate (Orifice Controls 0.73 cfs @ 2.34 fps)

Hydrograph 5 4.50 cfs Inflow Primary Inflow Area=391,420 sf 4 Peak Elev=694.95' Storage=10,505 cf 3-Flow (cfs) 2-1.75 cfs 1 0 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 5 Ó Time (hours)

Pond Pond D: Pond D

Summary for Pond Pond E: Pond E

Inflow Area	=	580,537 sf,	6.24% Impervious,	Inflow Depth = 1.66" for 5-Year event
Inflow	=	5.04 cfs @	7.97 hrs, Volume=	80,201 cf
Outflow	=	2.22 cfs @	8.69 hrs, Volume=	80,188 cf, Atten= 56%, Lag= 42.8 min
Primary	=	2.22 cfs @	8.69 hrs, Volume=	80,188 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 714.27' @ 8.69 hrs Surf.Area= 10,009 sf Storage= 11,384 cf

Plug-Flow detention time= 105.7 min calculated for 80,182 cf (100% of inflow) Center-of-Mass det. time= 106.1 min (902.3 - 796.2)

Volume	Inver	t Avail.Sto	rage Stora	ge Description	
#1	713.00)' 31,30	08 cf Custo	om Stage Data (Pr	rismatic) Listed below (Recalc)
_					
Elevatio	on S	Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
713.0	00	7,916	0	0)
714.(00	9,540	8,728	8,728	
715.0	00	11,265	10,403	19,131	
716.0	00	13,090	12,178	31,308	6
Device	Routing	Invert	Outlet Devi	ices	
#1	Primary	713.00'	15.0" Rou	nd Culvert	
	-		L= 44.0' C	CMP, square edge	e headwall, Ke= 0.500
			Inlet / Outle	et Invert= 713.00' /	/ 712.80' S= 0.0045 '/' Cc= 0.900
			n= 0.013,	Flow Area= 1.23 s	sf
#2	Device 1	711.00'	7.0" Horiz.	Orifice/Grate C	C= 0.620 Limited to weir flow at low heads
#3	Device 1	713.95'	16.0" W x 3	3.0" H Vert. Orifice	ce/Grate C= 0.620
			0	HW=714.27' (Free	ee Discharge)
1=Cu	ilvort (Pas	ses 2.22 cfs of	383 cfe not	ential flow)	

1=Culvert (Passes 2.22 cfs of 3.83 cfs potential flow)

-2=Orifice/Grate (Orifice Controls 1.50 cfs @ 5.61 fps)

-3=Orifice/Grate (Orifice Controls 0.72 cfs @ 2.17 fps)

Hydrograph 5.04 cfs Inflow Primary 5 Inflow Area=580,537 sf Peak Elev=714.27' 4-Storage=11,384 cf Flow (cfs) 3-2.22 cfs 2-1 0 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 5 Ó Time (hours)

Pond Pond E: Pond E

Bull Run Filtration Detention Ponds	Type IA 24-
Prepared by {enter your company name here}	
HydroCAD® 10.00-24 s/n 04804 © 2018 HydroCAD Software Solution	ns LLC

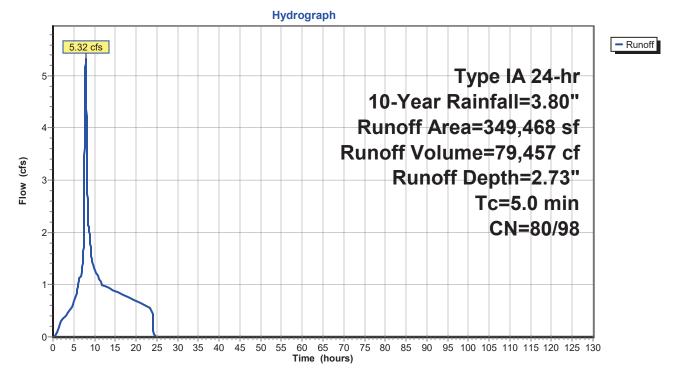
Summary for Subcatchment A: Post Basin A

Runoff = 5.32 cfs @ 7.91 hrs, Volume= 79,457 cf, Depth= 2.73"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

	Area (sf)	CN	Description				
*	176,142	98	Impervious	Area			
	173,326	80	>75% Gras	s cover, Go	bod, HSG D		
	349,468	89	Weighted Average				
	173,326	80	49.60% Pervious Area				
	176,142	98	50.40% Imp	pervious Ar	ea		
Tc (min)		Slope (ft/ft)	,	Capacity (cfs)	Description		
5.0					Direct Entry,		
			-				

Subcatchment A: Post Basin A



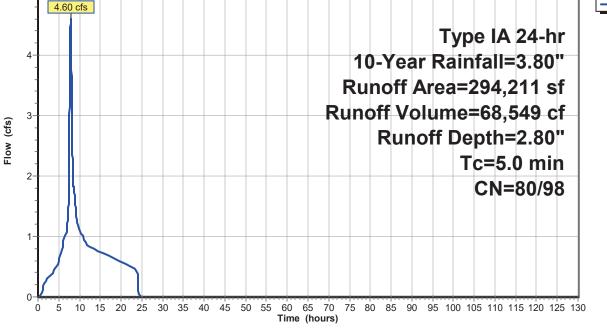
Bull Run Filtration Detention Ponds	Туре І
Prepared by {enter your company name here}	
HydroCAD® 10.00-24 s/n 04804 © 2018 HydroCAD Software Solution	is LLC

Summary for Subcatchment B: Post Basin B

Runoff = 4.60 cfs @ 7.91 hrs, Volume= 68,549 cf, Depth= 2.80"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

Area (sf)	CN Description
* 160,056	98 Impervious Area
134,155	80 >75% Grass cover, Good, HSG D
294,211	90 Weighted Average
134,155	80 45.60% Pervious Area
160,056	98 54.40% Impervious Area
Tc Length (min) (feet)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)
5.0	Direct Entry,
	Subcatchment B: Post Basin B
	Hydrograph
5- - <u>4.60 cfs</u>	
	Type IA 24-hr
4	10-Year Rainfall=3.80"
	Runoff Area=294,211 sf



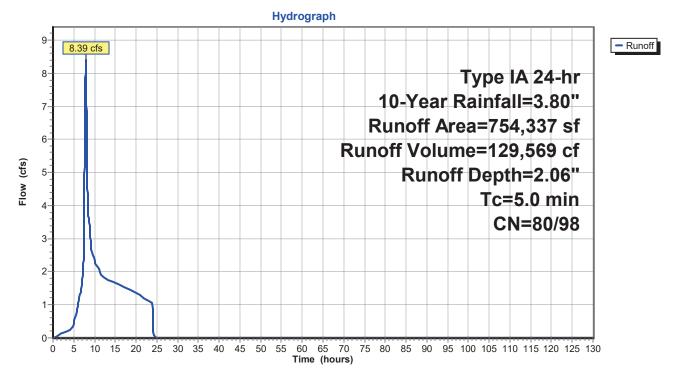
HydroCAD® 10.00-24 s/n 04804 © 2018 HydroCAD Software Solutions LLC

Summary for Subcatchment C: Post Basin C

Runoff = 8.39 cfs @ 7.96 hrs, Volume= 129,569 cf, Depth= 2.06"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

	Area (sf)	CN	Description							
*	82,047	98	Impervious	Area						
	672,290	80	>75% Gras	s cover, Go	bod, HSG D					
	754,337	754,337 82 Weighted Average								
	672,290	80	89.12% Per	vious Area						
	82,047	98	10.88% Imp	pervious Ar	ea					
	Tc Length (min) (feet)	Slop (ft/f	,	Capacity (cfs)	Description					
	5.0				Direct Entry,					
	Subcatchment C: Post Basin C									



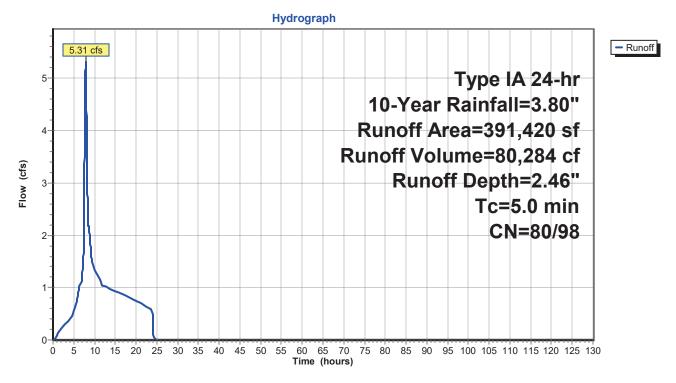
Bull Run Filtration Detention Ponds	Гуре IA 24
Prepared by {enter your company name here}	
HydroCAD® 10.00-24 s/n 04804 © 2018 HydroCAD Software Solutions	LLC

Summary for Subcatchment D: Post Basin D

Runoff = 5.31 cfs @ 7.93 hrs, Volume= 80,284 cf, Depth= 2.46"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

	Area	a (sf)	CN	Description					
*	135	5,356	98	Impervious	Area				
	256	5,064	80	>75% Gras	s cover, Go	ood, HSG D			
	391	391,420 86 Weighted Average							
	256	5,064	80	65.42% Per	vious Area	3			
	135	5,356	98	34.58% Imp	ervious Ar	rea			
	Tc L (min)	_ength (feet)	Slope (ft/ft		Capacity (cfs)	Description			
	5.0					Direct Entry,			
	Subcatchment D: Post Basin D								



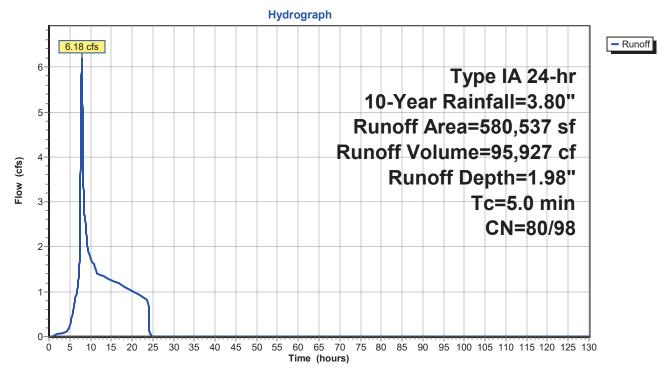
Summary for Subcatchment E: Post Basin E

Runoff = 6.18 cfs @ 7.96 hrs, Volume= 95,927 cf, Depth= 1.98"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

_	Area (sf)	CN	Description				
*	19,160	98	Impervious	Area			
	544,328	80	>75% Gras	s cover, Go	ood, HSG D		
*	17,049	98					
	580,537	81	Weighted A	verage			
	544,328	80	93.76% Per	vious Area	а		
	36,209	98	6.24% Impe	ervious Are	ea		
	Tc Length	Slop		Capacity	Description		
_	(min) (feet)	(ft/	ft) (ft/sec)	(cfs)			
	5.0				Direct Entry,		

Subcatchment E: Post Basin E



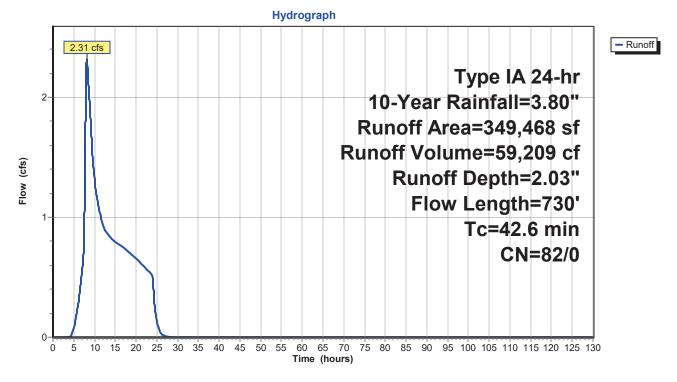
Summary for Subcatchment Pre A: Pre Dev Basin A

Runoff = 2.31 cfs @ 8.14 hrs, Volume= 59,209 cf, Depth= 2.03"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

_	A	rea (sf)	CN	Description		
349,468 82 Row crops, SR + CR, Good, HSG C						Good, HSG C
	3	49,468	82	100.00% Pe	ervious Are	а
	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description
-	36.8	300	0.0100			Sheet Flow,
	5.8	430	0.0190	1.24		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
	42.6	730	Total			

Subcatchment Pre A: Pre Dev Basin A



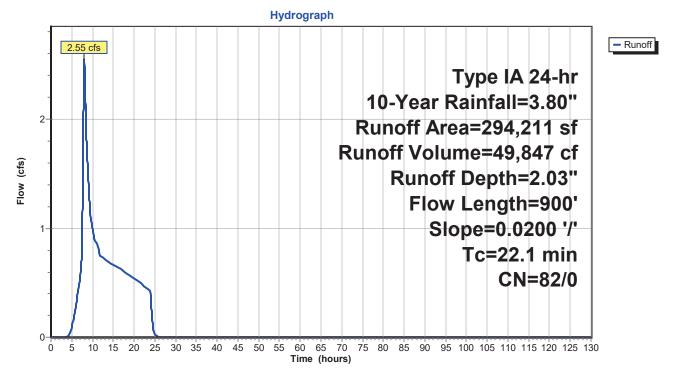
Summary for Subcatchment Pre B: Pre Dev Basin B

Runoff = 2.55 cfs @ 8.01 hrs, Volume= 49,847 cf, Depth= 2.03"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

_	A	rea (sf)	CN [Description		
	2	94,211	82 F	Row crops,	SR + CR,	Good, HSG C
	2	94,211	82 1	100.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	11.6	100	0.0200	0.14		Sheet Flow,
	10.5	800	0.0200	1.27		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
	22.1	900	Total			

Subcatchment Pre B: Pre Dev Basin B



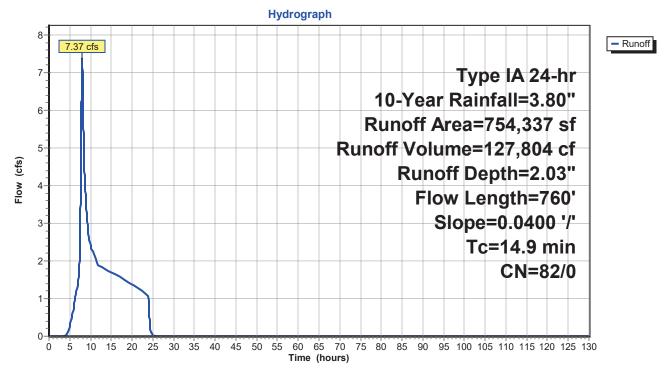
Summary for Subcatchment Pre C: Pre Dev Basin C

Runoff = 7.37 cfs @ 8.00 hrs, Volume= 127,804 cf, Depth= 2.03"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

_	A	rea (sf)	CN E	Description		
	7	54,337	82 F	Row crops,	SR + CR,	Good, HSG C
	7	54,337	82 1	00.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	8.8	100	0.0400	0.19		Sheet Flow,
	6.1	660	0.0400	1.80		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
	14.9	760	Total			

Subcatchment Pre C: Pre Dev Basin C



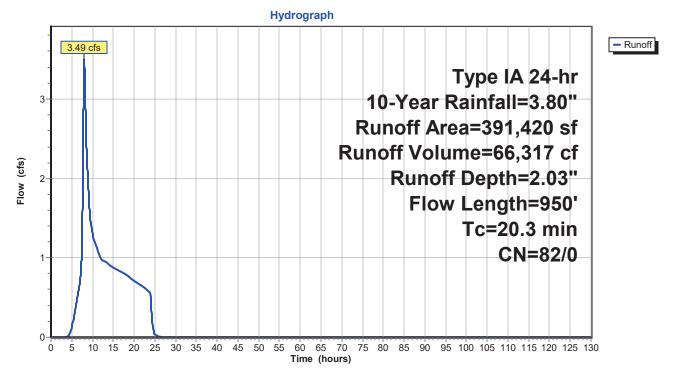
Summary for Subcatchment Pre D: Pre Dev Basin D

Runoff = 3.49 cfs @ 8.01 hrs, Volume= 66,317 cf, Depth= 2.03"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

_	A	rea (sf)	CN E	Description		
	3	91,420	82 F	Row crops,	SR + CR,	Good, HSG C
_	3	91,420	82 1	00.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	11.6	100	0.0200	0.14		Sheet Flow,
	8.7	850	0.0330	1.63		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
	20.3	950	Total			

Subcatchment Pre D: Pre Dev Basin D



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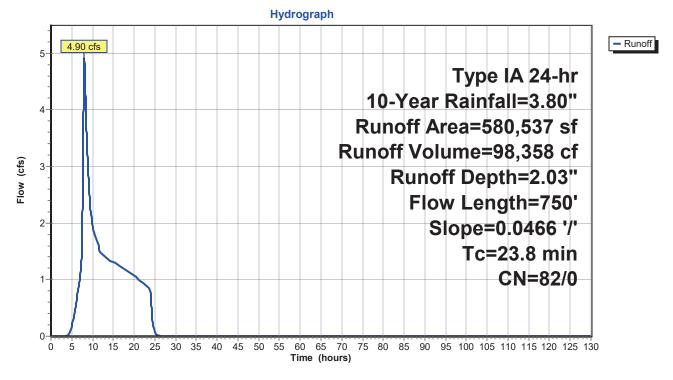
Summary for Subcatchment Pre E: Pre Dev Basin E

Runoff = 4.90 cfs @ 8.01 hrs, Volume= 98,358 cf, Depth= 2.03"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

_	A	rea (sf)	CN [Description		
	5	80,537	82 F	Row crops,	SR + CR,	Good, HSG C
	580,537 82 100.00% Pervious Area			100.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	19.9	300	0.0466	0.25	· · ·	Sheet Flow,
_	3.9	450	0.0466	1.94		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
	23.8	750	Total			

Subcatchment Pre E: Pre Dev Basin E



Summary for Pond Pond A: Pond A

Inflow Area	1 =	349,468 sf,	50.40% Impervious,	Inflow Depth = 2.73" for 10-Year event
Inflow	=	5.32 cfs @	7.91 hrs, Volume=	79,457 cf
Outflow	=	1.56 cfs @	9.22 hrs, Volume=	79,457 cf, Atten= 71%, Lag= 78.7 min
Primary	=	1.56 cfs @	9.22 hrs, Volume=	79,457 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 709.85' @ 9.22 hrs Surf.Area= 8,661 sf Storage= 17,390 cf

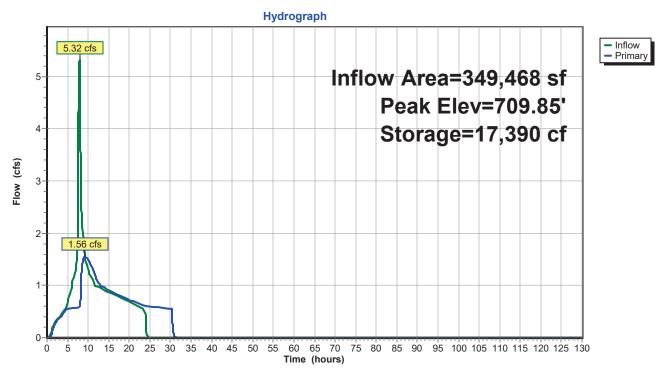
Plug-Flow detention time= 223.3 min calculated for 79,451 cf (100% of inflow) Center-of-Mass det. time= 223.3 min (932.4 - 709.1)

Volume	Inver	t Avail.Sto	rage Storage I	Description					
#1	707.50	י' 70,5 5	55 cf Custom	Stage Data (Prismati	c) Listed below (Recalc)				
				-					
Elevatio		Surf.Area	Inc.Store	Cum.Store					
(feet	I)	(sq-ft)	(cubic-feet)	(cubic-feet)					
707.50	0	6,107	0	0					
708.00	0	6,714	3,205	3,205					
709.00	0	7,742	7,228	10,433					
710.00	0	8,825	8,284	18,717					
711.00	0	9,967	9,396	28,113					
712.00	0	11,161	10,564	38,677					
713.00	0	12,412	11,787	50,463					
714.00	0	13,717	13,065	63,528					
714.50	0	14,392	7,027	70,555					
Device	Routing	Invert	Outlet Devices	5					
#1	Primary	698.43'	18.0" Round	Culvert					
	· · · · · · · · · · · · · · · · · · ·			IP, square edge head	wall. Ke= 0.500				
					4' S= 0.0159 '/' Cc= 0.900				
				w Area= 1.77 sf					
#2	Device 1	696.43'	,		0 Limited to weir flow at low heads				
#3	Device 1	709.40'	16.0" W x 3.0"	H Vert. Orifice/Grate	e C= 0.620				
	Primary OutFlow Max=1.56 cfs @ 9.22 hrs HW=709.85' (Free Discharge)								

__1=Culvert (Passes 1.56 cfs of 24.60 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.62 cfs @ 16.81 fps)

-3=Orifice/Grate (Orifice Controls 0.94 cfs @ 2.81 fps)



Pond Pond A: Pond A

Summary for Pond Pond B: Pond B

Inflow Area	=	294,211 sf,	54.40% Impervious,	Inflow Depth = 2.80" for 10-Year even	ent
Inflow	=	4.60 cfs @	7.91 hrs, Volume=	68,549 cf	
Outflow	=	1.70 cfs @	8.80 hrs, Volume=	68,549 cf, Atten= 63%, Lag= 53	3.7 min
Primary	=	1.70 cfs @	8.80 hrs, Volume=	68,549 cf	

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 709.73' @ 8.80 hrs Surf.Area= 5,801 sf Storage= 12,599 cf

Plug-Flow detention time= 154.7 min calculated for 68,549 cf (100% of inflow) Center-of-Mass det. time= 154.7 min (858.9 - 704.2)

Volume	Inve	rt Avail.Sto	rage Storage D	Storage Description				
#1	707.0	0' 36,8'	13 cf Custom S	Stage Data (Pri	ismatic) Listed below (Recalc)			
Elevetie		Curf Area	In a Chana	Curra Charra				
Elevatio		Surf.Area	Inc.Store	Cum.Store				
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)				
707.0)0	3,493	0	0				
708.0)0	4,288	3,891	3,891				
709.0	00	5,139	4,714	8,604				
710.0	00	6,046	5,593	14,197				
711.0	00	7,010	6,528	20,725				
712.0)0	8,030	7,520	28,245				
713.0	00	9,107	8,569	36,813				
Device	Routing	Invert	Outlet Devices					
#1	Primary	705.04'	18.0" Round C	Culvert				
	,		L= 339.0' CMF	. square edge	e headwall, Ke= 0.500			
					701.65' S= 0.0100 '/' Cc= 0.900			
			n= 0.013, Flow					
#2	Device 1	703.04'	,		= 0.620 Limited to weir flow at low heads			
#2	Device 1	709.15	••••	4.0" W x 3.0" H Vert. Orifice/Grate C= 0.620				
#3	Device I	709.15	14.U VV X J.U	n ven. Onlice				

Primary OutFlow Max=1.70 cfs @ 8.80 hrs HW=709.73' (Free Discharge)

1=Culvert (Passes 1.70 cfs of 13.11 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.72 cfs @ 10.78 fps)

-3=Orifice/Grate (Orifice Controls 0.98 cfs @ 3.35 fps)

Hydrograph 5-4.60 cfs Inflow Primary Inflow Area=294,211 sf 4 Peak Elev=709.73' Storage=12,599 cf 3 Flow (cfs) 2-1.70 cfs 1 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 Ó Time (hours)

Pond Pond B: Pond B

Summary for Pond Pond C: Pond C

Inflow Area	a =	754,337 sf, 10.88% Impervious, Inflow Depth > 8.02	for 10-Year event
Inflow	=	9.19 cfs @ 7.96 hrs, Volume= 503,998 cf, Inc	I. 0.80 cfs Base Flow
Outflow	=	2.92 cfs @ 10.92 hrs, Volume= 503,653 cf, Att	en= 68%, Lag= 177.7 min
Primary	=	2.92 cfs @ 10.92 hrs, Volume= 503,653 cf	

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 705.34' @ 10.92 hrs Surf.Area= 21,787 sf Storage= 26,741 cf

Plug-Flow detention time= 54.2 min calculated for 503,648 cf (100% of inflow) Center-of-Mass det. time= 50.9 min (3,147.8 - 3,096.8)

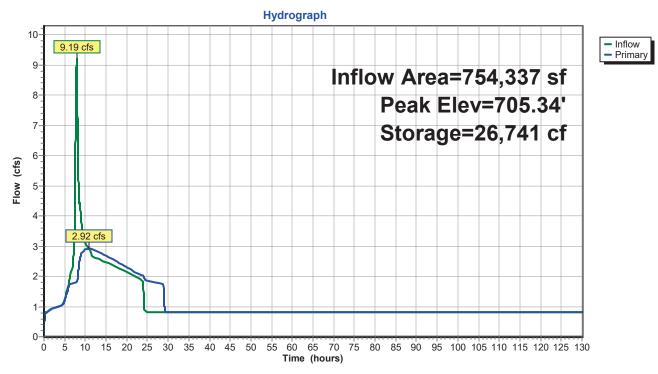
Volume	Inver	t Avail.Sto	rage Storage	Description		
#1	704.00)' 95,41	l6 cf Custom	Stage Data (Pr	ismatic) Listed below (I	Recalc)
Floyetia		urf Aree	Ino Store	Cum Store		
Elevatio (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
	/	18,277	1 /			
704.0 705.0		20,828	0 19,553	0 19,553		
705.0	-	20,828	22,250	41,802		
700.0	-	26,743	25,207	67,009		
708.0	-	30,070	28,407	95,416		
			,			
Device	Routing	Invert	Outlet Device	s		
#1	Primary	697.90'	12.0" Round	Culvert		
			L= 53.0' CM	P, square edge	headwall, Ke= 0.500	
			Inlet / Outlet I	nvert= 697.90' /	695.55' S= 0.0443 '/'	Cc= 0.900
			,	ow Area= 0.79 sf		
#2	Device 1	693.90'			= 0.620 Limited to weil	r flow at low heads
#3	Device 1	704.85'	16.0" W x 3.0	" H Vert. Orifice	e/Grate C= 0.620	

Primary OutFlow Max=2.92 cfs @ 10.92 hrs HW=705.34' (Free Discharge)

-1=Culvert (Passes 2.92 cfs of 9.96 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.92 cfs @ 13.57 fps)

-3=Orifice/Grate (Orifice Controls 0.99 cfs @ 2.98 fps)



Pond Pond C: Pond C

Summary for Pond Pond D: Pond D

Inflow Area	a =	391,420 sf,	34.58% Impervious,	Inflow Depth = 2.46" for 10-Year event
Inflow	=	5.31 cfs @	7.93 hrs, Volume=	80,284 cf
Outflow	=	2.18 cfs @	8.65 hrs, Volume=	80,284 cf, Atten= 59%, Lag= 43.4 min
Primary	=	2.18 cfs @	8.65 hrs, Volume=	80,284 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 695.23' @ 8.65 hrs Surf.Area= 6,622 sf Storage= 12,303 cf

Plug-Flow detention time= 106.7 min calculated for 80,284 cf (100% of inflow) Center-of-Mass det. time= 106.7 min (838.2 - 731.5)

Volume	Inver	t Avail.Sto	rage Storage D	Description	
#1	693.00	45,10	06 cf Custom S	Stage Data (Pri	ismatic) Listed below (Recalc)
	0				
Elevation		Surf.Area	Inc.Store	Cum.Store	
(feet)		(sq-ft)	(cubic-feet)	(cubic-feet)	
693.00)	4,453	0	0	
694.00		5,388	4,921	4,921	
695.00		6,380	5,884	10,805	
696.00		7,428	6,904	17,709	
697.00		8,533	7,981	25,689	
698.00		9,694	9,114	34,803	
699.00		10,912	10,303	45,106	
Device I	Routing	Invert	Outlet Devices		
#1 F	Primary	692.50'	12.0" Round C	Culvert	
	2		L= 121.0' CMI	P, square edge	e headwall, Ke= 0.500
				, I U	688.97' S= 0.0292 '/' Cc= 0.900
			n= 0.013, Flow		
#2 [Device 1	690.50'			= 0.620 Limited to weir flow at low heads
	Device 1	694.60'	15.0" W x 3.0"		

Primary OutFlow Max=2.18 cfs @ 8.65 hrs HW=695.23' (Free Discharge)

-**1=Culvert** (Passes 2.18 cfs of 5.65 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.08 cfs @ 8.22 fps)

-3=Orifice/Grate (Orifice Controls 1.10 cfs @ 3.53 fps)

Hydrograph 5.31 cfs Inflow Primary Inflow Area=391,420 sf 5 Peak Elev=695.23' Storage=12,303 cf 4 Flow (cfs) 3-2.18 cfs 2 1 0-10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 5 Ó Time (hours)

Pond Pond D: Pond D

Summary for Pond Pond E: Pond E

Inflow Area	=	580,537 sf,	6.24% Impervious,	Inflow Depth = 1.98" for 10-Year event
Inflow :	=	6.18 cfs @	7.96 hrs, Volume=	95,927 cf
Outflow :	=	2.71 cfs @	8.48 hrs, Volume=	95,915 cf, Atten= 56%, Lag= 31.0 min
Primary :	=	2.71 cfs @	8.48 hrs, Volume=	95,915 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 714.50' @ 8.48 hrs Surf.Area= 10,410 sf Storage= 13,759 cf

Plug-Flow detention time= 105.0 min calculated for 95,915 cf (100% of inflow) Center-of-Mass det. time= 104.9 min (891.5 - 786.6)

Volume	Inve	ert Avail.Sto	rage Storage	Description				
#1	#1 713.00' 31,30		08 cf Custom	n Stage Data (Pr	ismatic) Listed below (F	Recalc)		
ElevationSurf.Area(feet)(sq-ft)713.007,916		Inc.Store (cubic-feet) 0	Cum.Store (cubic-feet) 0					
714.0		9,540	8,728	8,728				
715.0		11,265	10,403	19,131				
716.0	00	13,090	12,178	31,308				
Device	Device Routing Invert		Outlet Device	es				
#1	Primary Device 1	713.00' 711.00'	0' 15.0'' Round Culvert L= 44.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 713.00' / 712.80' S= 0.0045 '/' n= 0.013, Flow Area= 1.23 sf		Cc= 0.900 flow at low heads			
#3	Device 1	713.95'	16.0" W x 3.0	" H Vert. Orifice	e/Grate C= 0.620			
Primary OutFlow Max=2 71 cfs @ 8.48 brs HW=714 50' (Free Discharge)								

Primary OutFlow Max=2.71 cfs @ 8.48 hrs HW=714.50' (Free Discharge)

-1=Culvert (Passes 2.71 cfs of 4.67 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.63 cfs @ 6.10 fps)

-3=Orifice/Grate (Orifice Controls 1.08 cfs @ 3.25 fps)

Hydrograph 6.18 cfs Inflow Primary 6-Inflow Area=580,537 sf Peak Elev=714.50' 5-Storage=13,759 cf 4 Flow (cfs) 3-2.71 cfs 2-1. 0-10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 5 Ó Time (hours)

Pond Pond E: Pond E

Bull Run Filtration Detention Ponds	Type IA
Prepared by {enter your company name here}	
HydroCAD® 10.00-24 s/n 04804 © 2018 HydroCAD Software Solution	ns LLC

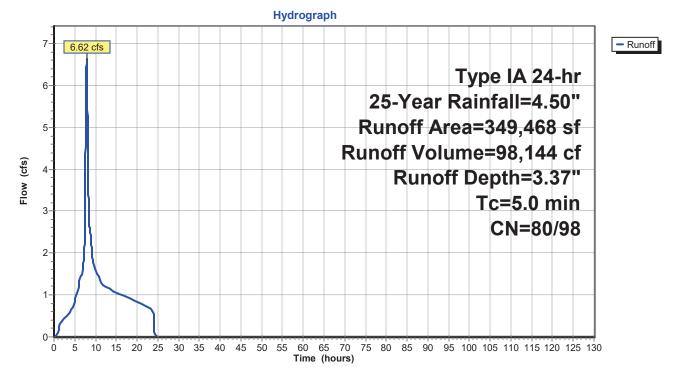
Summary for Subcatchment A: Post Basin A

Runoff = 6.62 cfs @ 7.91 hrs, Volume= 98,144 cf, Depth= 3.37"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

_	Are	ea (sf)	CN	Description				
*	17	6,142	98	Impervious	Area			
_	17	3,326	80	>75% Gras	s cover, Go	bod, HSG D		
	34	9,468	89	9 Weighted Average				
	17	3,326	80					
	17	6,142	98	50.40% Imp	pervious Ar	ea		
	Tc (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description		
	5.0					Direct Entry,		

Subcatchment A: Post Basin A



Bull Run Filtration Detention Ponds Prepared by {enter your company name here}

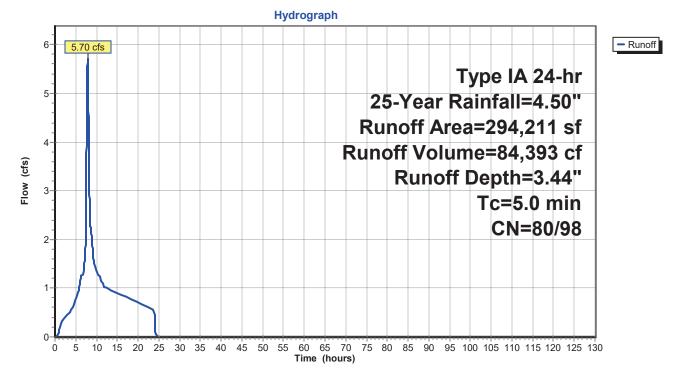
Summary for Subcatchment B: Post Basin B

Runoff = 5.70 cfs @ 7.90 hrs, Volume= 84,393 cf, Depth= 3.44"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

	Are	ea (sf)	CN	Description					
*	16	0,056	98	Impervious	Area				
	13	4,155	80	>75% Gras	s cover, Go	bod, HSG D			
	29	4,211	90	90 Weighted Average					
	13	4,155	80	45.60% Per	rvious Area				
	16	0,056	98	54.40% Imp	pervious Ar	ea			
(n	Tc I nin)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description			
	5.0					Direct Entry,			
	Subastabrant B. Bast Pasin B								

Subcatchment B: Post Basin B



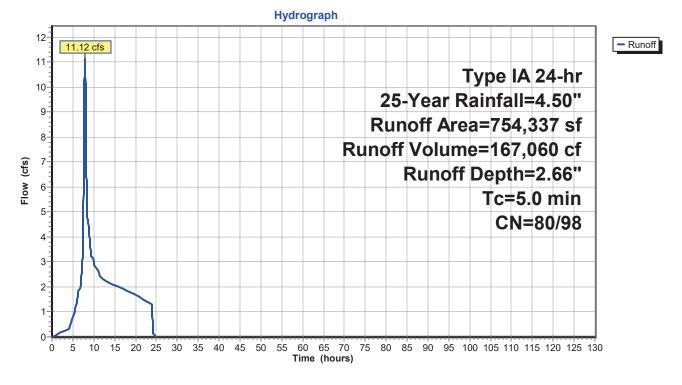
Summary for Subcatchment C: Post Basin C

Runoff = 11.12 cfs @ 7.94 hrs, Volume= 167,060 cf, Depth= 2.66"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

_	A	rea (sf)	CN	Description				
*		82,047	98	Impervious	Area			
	6	672,290	80	>75% Gras	s cover, Go	bod, HSG D		
	7	′54,337	4,337 82 Weighted Average					
	672,290 80 89.12% Pervious Area				vious Area	l		
	82,047 98 10.88% Impervious Are			10.88% Imp	pervious Ar	ea		
_	Tc (min)	Length (feet)	Slop (ft/f	,	Capacity (cfs)	Description		
	5.0					Direct Entry,		

Subcatchment C: Post Basin C



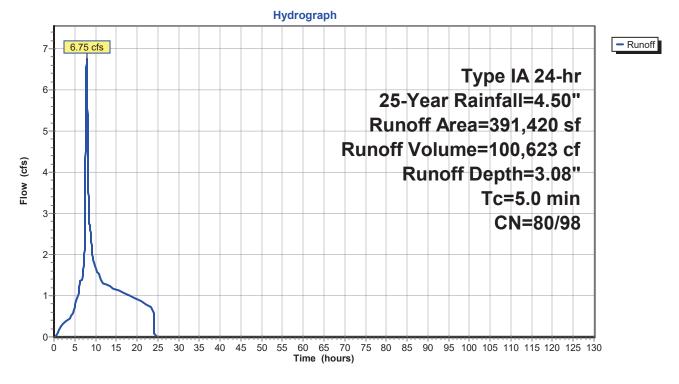
Summary for Subcatchment D: Post Basin D

Runoff = 6.75 cfs @ 7.92 hrs, Volume= 100,623 cf, Depth= 3.08"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

_	A	rea (sf)	CN	Description				
*	1	35,356	98	Impervious	Area			
_	2	256,064	80	>75% Gras	s cover, Go	bod, HSG D		
	3	91,420	86	36 Weighted Average				
	2	256,064	80 65.42% Pervious Area					
	135,356 98 34.58% Impervious Are			34.58% Imp	pervious Ar	ea		
	Tc (min)	Length (feet)	Slop (ft/ft		Capacity (cfs)	Description		
	5.0					Direct Entry,		

Subcatchment D: Post Basin D



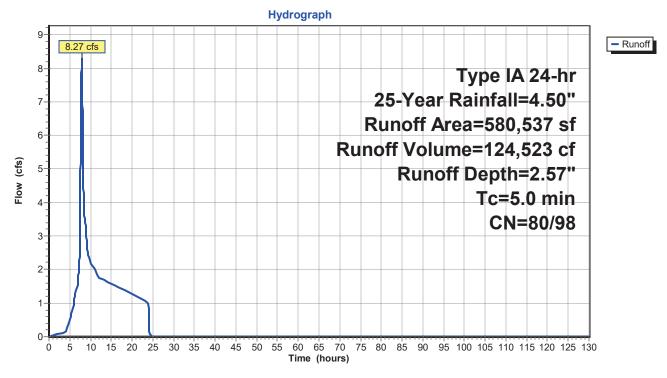
Summary for Subcatchment E: Post Basin E

Runoff = 8.27 cfs @ 7.95 hrs, Volume= 124,523 cf, Depth= 2.57"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

Area (sf)	CN	Description				
19,160	98	Impervious Area				
544,328	80	>75% Grass cover, Good, HSG D				
17,049	98					
580,537 81 Weighted Average			age			
544,328 80 93.76% Pervious Area 36,209 98 6.24% Impervious Area			us Area	l		
			ous Area	a		
Tc Length		,		Description		
min) (feet)	(ft/	t) (ft/sec)	(cfs)			
5.0				Direct Entry,		
	19,160 544,328 17,049 580,537 544,328 36,209 Tc Length min) (feet)	19,160 98 544,328 80 17,049 98 580,537 81 544,328 80 36,209 98 Tc Length Slop min) (feet) (ft/f	19,160 98 Impervious Are 544,328 80 >75% Grass co 17,049 98 580,537 81 Weighted Avera 544,328 80 93.76% Perviou 544,328 80 93.76% Perviou 36,209 98 6.24% Imperviou Tc Length Slope Velocity Ca min) (feet) (ft/ft) (ft/sec)	19,16098Impervious Area544,32880>75% Grass cover, Go17,04998580,53781Weighted Average544,3288093.76% Pervious Area36,209986.24% Impervious AreaTcLengthSlopeVelocityTcLengthSlopeVelocitymin)(feet)(ft/ft)(ft/sec)		

Subcatchment E: Post Basin E



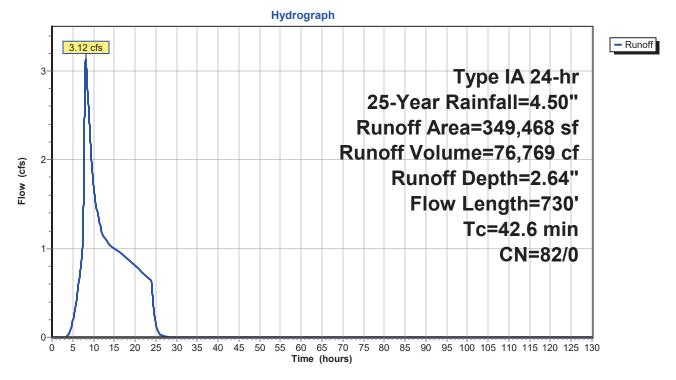
Summary for Subcatchment Pre A: Pre Dev Basin A

Runoff = 3.12 cfs @ 8.11 hrs, Volume= 76,769 cf, Depth= 2.64"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

	Area (sf) CN Description							
	3	49,468	82 F	Row crops, SR + CR, Good, HSG C				
	349,468 8			100.00% Pervious Area				
(Tc min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
	36.8	300	0.0100	0.14		Sheet Flow,		
	5.8	430	0.0190	1.24		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps		
	42.6	730	Total					

Subcatchment Pre A: Pre Dev Basin A



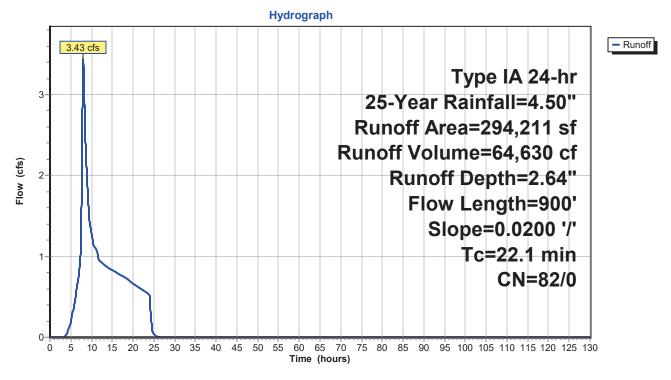
Summary for Subcatchment Pre B: Pre Dev Basin B

Runoff = 3.43 cfs @ 8.01 hrs, Volume= 64,630 cf, Depth= 2.64"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

Area (sf) CN Description							
	2	94,211	82 Row crops, SR + CR, Good, HSG C				
	2	94,211	82 1	00.00% Pe	ervious Are	a	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
-	11.6	100	0.0200	0.14		Sheet Flow,	
	10.5	800	0.0200	1.27		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps	
	22.1	900	Total				

Subcatchment Pre B: Pre Dev Basin B



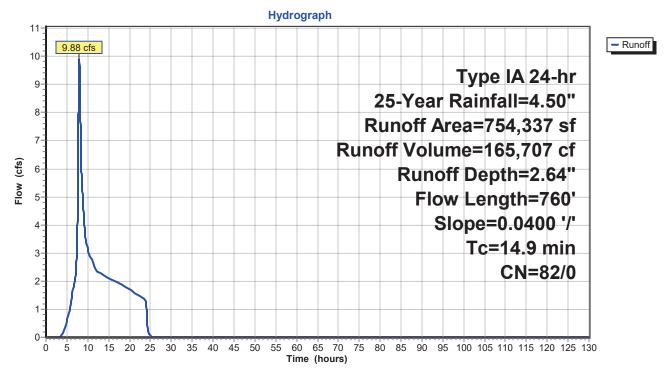
Summary for Subcatchment Pre C: Pre Dev Basin C

Runoff = 9.88 cfs @ 8.00 hrs, Volume= 165,707 cf, Depth= 2.64"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

A	Area (sf)	CN E	Description				
	754,337	82 F	Row crops,	SR + CR,	Good, HSG C		
	754,337	82 1	100.00% Pervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
8.8	100	0.0400	0.19		Sheet Flow,		
6.1	660	0.0400	1.80		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps		
14.9	760	Total					

Subcatchment Pre C: Pre Dev Basin C



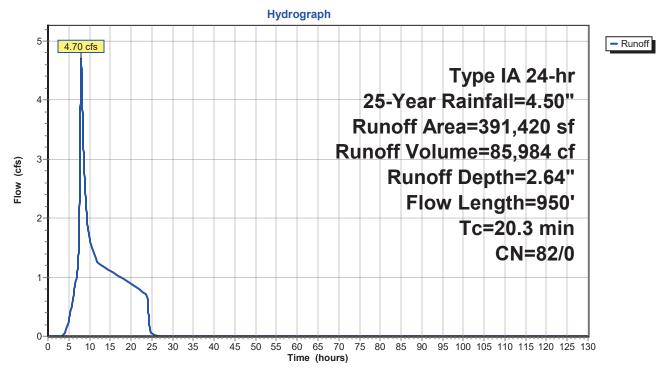
Summary for Subcatchment Pre D: Pre Dev Basin D

Runoff = 4.70 cfs @ 8.01 hrs, Volume= 85,984 cf, Depth= 2.64"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

	Area (sf)	CN [Description				
	391,420	82 F	Row crops,	SR + CR,	Good, HSG C		
	391,420	82 1	100.00% Pervious Area				
To (min		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
11.6	5 100	0.0200	0.14		Sheet Flow,		
8.7	850	0.0330	1.63		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps		
20.3	950	Total					

Subcatchment Pre D: Pre Dev Basin D



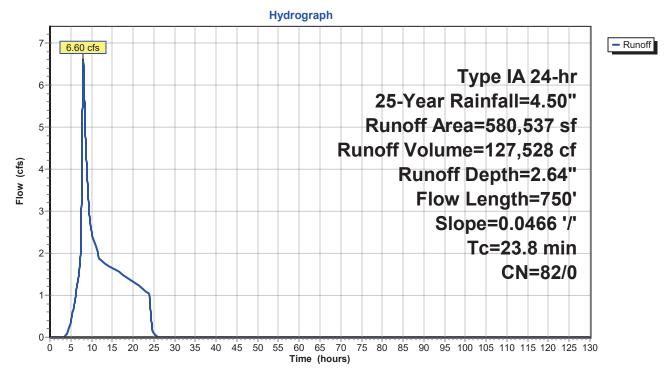
Summary for Subcatchment Pre E: Pre Dev Basin E

Runoff = 6.60 cfs @ 8.01 hrs, Volume= 127,528 cf, Depth= 2.64"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

 A	rea (sf)	CN E	Description				
5	80,537	82 F	Row crops,	SR + CR,	Good, HSG C		
5	80,537	82 1	100.00% Pervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
 19.9	300	0.0466	0.25		Sheet Flow,		
 3.9	450	0.0466	1.94		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps		
 23.8	750	Total					

Subcatchment Pre E: Pre Dev Basin E



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Summary for Pond Pond A: Pond A

Inflow Area	a =	349,468 sf,	50.40% Impervious,	Inflow Depth = 3.37" for 25-Year event
Inflow	=	6.62 cfs @	7.91 hrs, Volume=	98,144 cf
Outflow	=	2.11 cfs @	9.05 hrs, Volume=	98,144 cf, Atten= 68%, Lag= 68.5 min
Primary	=	2.11 cfs @	9.05 hrs, Volume=	98,144 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 710.32' @ 9.05 hrs Surf.Area= 9,193 sf Storage= 21,617 cf

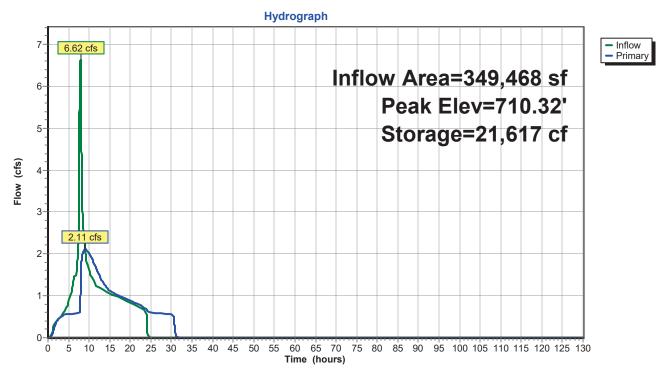
Plug-Flow detention time= 200.9 min calculated for 98,144 cf (100% of inflow) Center-of-Mass det. time= 200.9 min (904.6 - 703.7)

Volume	Inve	ert Avail.Sto	rage Storage D	Description		
#1	707.5	0' 70,5	55 cf Custom S	Stage Data (Prism	atic) Listed below (Recalc)	
				-		
Elevatio		Surf.Area	Inc.Store	Cum.Store		
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)		
707.5	50	6,107	0	0		
708.0	00	6,714	3,205	3,205		
709.0	00	7,742	7,228	10,433		
710.0	00	8,825	8,284	18,717		
711.0	00	9,967	9,396	28,113		
712.0	00	11,161	10,564	38,677		
713.0	00	12,412	11,787	50,463		
714.0	00	13,717	13,065	63,528		
714.5	50	14,392	7,027	70,555		
. .	D ()					
Device	Routing	Invert	Outlet Devices			
#1	Primary	698.43'	18.0" Round 0			
					adwall, Ke= 0.500	
			Inlet / Outlet In	vert= 698.43' / 696	6.24' S= 0.0159 '/' Cc= 0.900	
			n= 0.013, Flow			
#2	Device 1	696.43'	2.6" Horiz. Ori	fice/Grate C= 0.	620 Limited to weir flow at low heads	
#3	Device 1	709.40'	16.0" W x 3.0"	H Vert. Orifice/Gr	rate C= 0.620	
	Primary OutFlow Max=2.11 cfs @ 9.05 hrs HW=710.32' (Free Discharge)					

1=Culvert (Passes 2.11 cfs of 25.08 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.63 cfs @ 17.16 fps)

—3=Orifice/Grate (Orifice Controls 1.48 cfs @ 4.44 fps)



Pond Pond A: Pond A

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Summary for Pond Pond B: Pond B

Inflow Area	a =	294,211 sf,	54.40% Impervious,	Inflow Depth = 3.44"	for 25-Year event
Inflow	=	5.70 cfs @	7.90 hrs, Volume=	84,393 cf	
Outflow	=	2.22 cfs @	8.68 hrs, Volume=	84,393 cf, Atte	n= 61%, Lag= 46.5 min
Primary	=	2.22 cfs @	8.68 hrs, Volume=	84,393 cf	

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 710.29' @ 8.68 hrs Surf.Area= 6,322 sf Storage= 15,967 cf

Plug-Flow detention time= 146.4 min calculated for 84,393 cf (100% of inflow) Center-of-Mass det. time= 146.4 min (845.4 - 699.0)

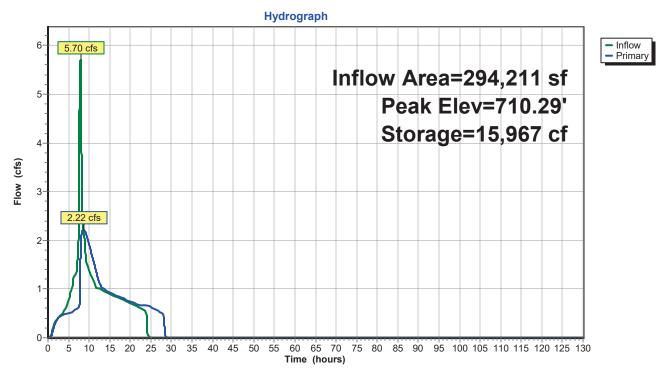
Volume	Inve	rt Avail.Sto	age Storage Description			
#1	707.0	D' 36,8 ⁻	13 cf Custom S	Stage Data (Pr	ismatic) Listed below (Recalc)	
- :						
Elevatio		Surf.Area	Inc.Store	Cum.Store		
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)		
707.0	00	3,493	0	0		
708.0	00	4,288	3,891	3,891		
709.0	00	5,139	4,714	8,604		
710.0	00	6,046	5,593	14,197		
711.0	00	7,010	6,528	20,725		
712.0	00	8,030	7,520	28,245		
713.0	00	9,107	8,569	36,813		
Device	Routing	Invert	Outlet Devices			
#1	Primary	705.04'	18.0" Round C	Culvert		
	,		L= 339.0' CM	P. square edge	e headwall, Ke= 0.500	
					701.65' S= 0.0100 '/' Cc= 0.900	
			n= 0.013, Flow			
#2	Device 1	703.04'	,		= 0.620 Limited to weir flow at low heads	
#2	Device 1	709.15	14.0" W x 3.0"			
#3	Device I	109.15	14.0 WX 3.0			

Primary OutFlow Max=2.22 cfs @ 8.68 hrs HW=710.29' (Free Discharge)

-**1=Culvert** (Passes 2.22 cfs of 13.65 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.76 cfs @ 11.40 fps)

-3=Orifice/Grate (Orifice Controls 1.46 cfs @ 5.00 fps)



Pond Pond B: Pond B

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Summary for Pond Pond C: Pond C

Inflow Are	a =	754,337 sf, 10.88% Impervious, Inflow Depth > 8.61" for 25-Year event	
Inflow	=	11.92 cfs @ 7.94 hrs, Volume= 541,489 cf, Incl. 0.80 cfs Base Flow	
Outflow	=	3.50 cfs @ 10.90 hrs, Volume= 541,144 cf, Atten= 71%, Lag= 177.5	min
Primary	=	3.50 cfs @ 10.90 hrs, Volume= 541,144 cf	

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 705.81' @ 10.90 hrs Surf.Area= 23,124 sf Storage= 37,299 cf

Plug-Flow detention time= 65.5 min calculated for 541,139 cf (100% of inflow) Center-of-Mass det. time= 62.3 min (2,994.7 - 2,932.3)

Volume	Inver	t Avail.Sto	rage Storage	age Storage Description		
#1	704.00)' 95,47	16 cf Custom	Stage Data (Pri	rismatic) Listed below (Recalc)	
Elevatio	n c	Surf.Area	Inc.Store	Cum.Store		
(fee		(sq-ft)	(cubic-feet)	(cubic-feet)		
704.0	00	18,277	0	0		
705.0	00	20,828	19,553	19,553		
706.0	-	23,671	22,250	41,802		
707.0	-	26,743	25,207	67,009		
708.0	00	30,070	28,407	95,416		
Device	Routing	Invert	Outlet Device	S		
#1	Primary	697.90'	12.0" Round	Culvert		
	-		L= 53.0' CM	P, square edge l	headwall, Ke= 0.500	
					/ 695.55' S= 0.0443 '/' Cc= 0.900	
			,	w Area= 0.79 sf		
#2	Device 1	693.90'			= 0.620 Limited to weir flow at low heads	
#3	Device 1	704.85'	16.0" W X 3.0	" H Vert. Orifice	e/Grate C= 0.620	

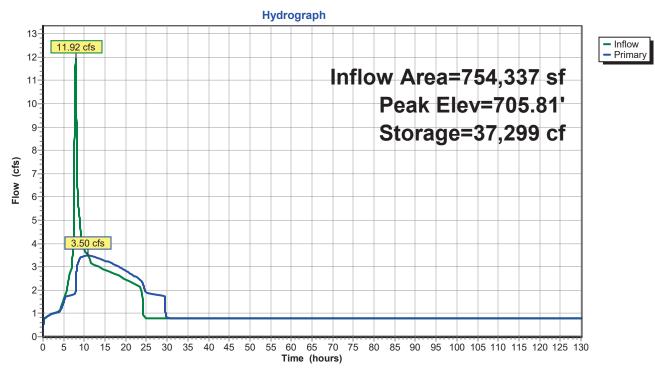
Primary OutFlow Max=3.50 cfs @ 10.90 hrs HW=705.81' (Free Discharge)

-1=Culvert (Passes 3.50 cfs of 10.29 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.98 cfs @ 13.99 fps)

-3=Orifice/Grate (Orifice Controls 1.51 cfs @ 4.54 fps)

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Pond Pond C: Pond C

Summary for Pond Pond D: Pond D

Inflow Area	a =	391,420 sf,	34.58% Impervious,	Inflow Depth = 3.08" for 25-Year event
Inflow	=	6.75 cfs @	7.92 hrs, Volume=	100,623 cf
Outflow	=	2.80 cfs @	8.47 hrs, Volume=	100,623 cf, Atten= 59%, Lag= 33.1 min
Primary	=	2.80 cfs @	8.47 hrs, Volume=	100,623 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 695.80' @ 8.47 hrs Surf.Area= 7,223 sf Storage= 16,277 cf

Plug-Flow detention time= 106.2 min calculated for 100,615 cf (100% of inflow) Center-of-Mass det. time= 106.2 min (830.6 - 724.3)

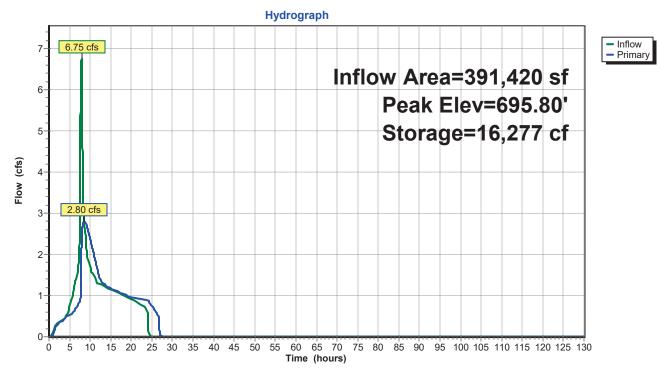
Volume	Invei	t Avail.Sto	rage Storage D	age Storage Description			
#1	693.00)' 45,10	06 cf Custom S	Stage Data (Pri	ismatic) Listed below (Recalc)		
Elevatio		Sumf Area	line Chaire	Curra Charra			
Elevatio		Surf.Area	Inc.Store	Cum.Store			
(fee	t)	(sq-ft)	(cubic-feet)	(cubic-feet)			
693.0	0	4,453	0	0			
694.0	0	5,388	4,921	4,921			
695.0	0	6,380	5,884	10,805			
696.0	0	7,428	6,904	17,709			
697.0	0	8,533	7,981	25,689			
698.0	0	9,694	9,114	34,803			
699.0	0	10,912	10,303	45,106			
Device	Routing	Invert	Outlet Devices				
#1	Primary	692.50'	12.0" Round C	Culvert			
	-		L= 121.0' CM	P. square edge	e headwall, Ke= 0.500		
					688.97' S= 0.0292 '/' Cc= 0.900		
			n= 0.013, Flow				
			,				
#2	Device 1	690.50'			= 0.620 Limited to weir flow at low heads		
#3	Device 1	694.60'	15.0" W x 3.0"	H Vert. Orifice	e/Grate C= 0.620		

Primary OutFlow Max=2.80 cfs @ 8.47 hrs HW=695.80' (Free Discharge)

-**1=Culvert** (Passes 2.80 cfs of 6.33 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.18 cfs @ 9.04 fps)

-3=Orifice/Grate (Orifice Controls 1.61 cfs @ 5.17 fps)



Pond Pond D: Pond D

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Summary for Pond Pond E: Pond E

Inflow Area	a =	580,537 sf,	6.24% Impervious,	Inflow Depth = 2.57" for 25-Year event
Inflow	=	8.27 cfs @	7.95 hrs, Volume=	124,523 cf
Outflow	=	3.45 cfs @	8.72 hrs, Volume=	124,511 cf, Atten= 58%, Lag= 46.1 min
Primary	=	3.45 cfs @	8.72 hrs, Volume=	124,511 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 714.98' @ 8.72 hrs Surf.Area= 11,227 sf Storage= 18,886 cf

Plug-Flow detention time= 102.6 min calculated for 124,511 cf (100% of inflow) Center-of-Mass det. time= 102.5 min (875.2 - 772.7)

Volume	Inve	ert Avail.Sto	Storage Storage Description			
#1	713.0	0' 31,30	08 cf Custom	Stage Data (Pris	smatic) Listed below (Recalc)	
Elevation Surf.Area (feet) (sq-ft)		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)			
713.0	00	7,916	0	0		
714.(714.00 9,540		8,728	8,728		
715.0	715.00 11,265		10,403	19,131		
716.0	00	13,090	12,178	31,308		
Device	Routing	Invert	Outlet Device	S		
#1	Primary	713.00'	15.0" Round Culvert			
	,		L= 44.0' CM	P, square edge h	eadwall, Ke= 0.500	
			Inlet / Outlet I	nvert= 713.00' / 7	'12.80' S= 0.0045 '/' Cc= 0.900	
			n= 0.013, Flo	w Area= 1.23 sf		
#2	Device 1	711.00'	7.0" Horiz. Oı	rifice/Grate C=	0.620 Limited to weir flow at low heads	
#3	Device 1	713.95'	16.0" W x 3.0	" H Vert. Orifice/	Grate C= 0.620	
Primary OutFlow Max=3.45 cfs @ 8.72 hrs. $HW=714.98'$ (Free Discharge)						

Primary OutFlow Max=3.45 cfs @ 8.72 hrs HW=714.98' (Free Discharge)

—1=Culvert (Passes 3.45 cfs of 5.97 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.87 cfs @ 7.00 fps)

-3=Orifice/Grate (Orifice Controls 1.57 cfs @ 4.72 fps)

4-

3-

2-

1

0-

5 Ó

3.45 cfs

10 15 20 25 30 35 40 45 50 55

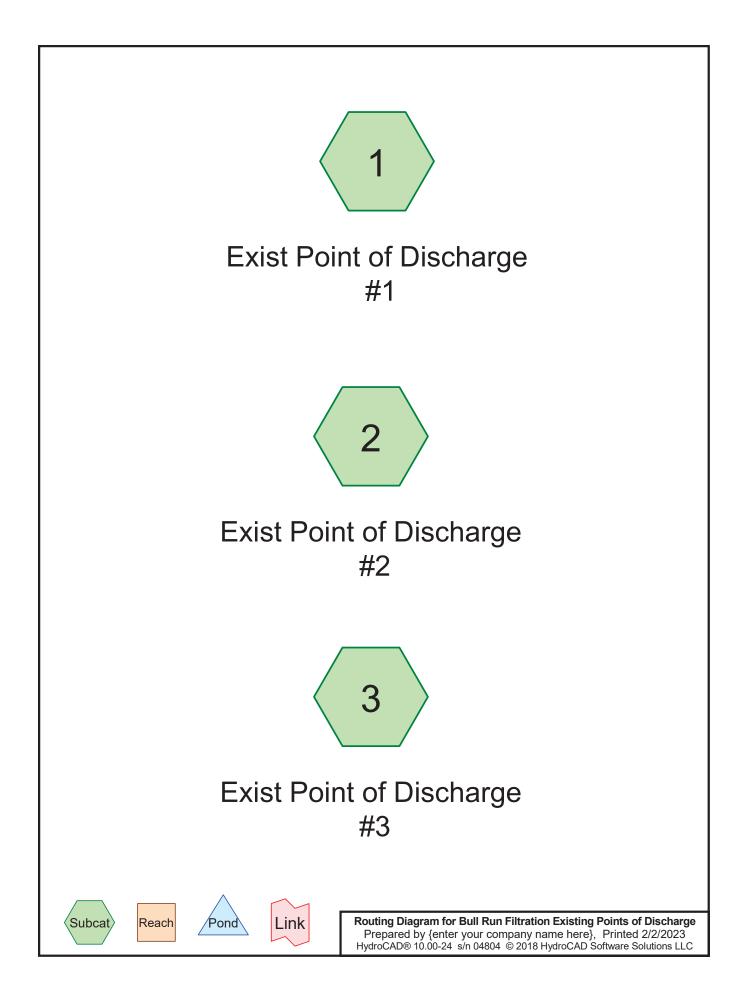
60 65 70 75 80 85 90 95 100 105 110 115 120 125 130

 Inflow Primary

Pond Pond E: Pond E Hydrograph 9-8.27 cfs 8-Inflow Area=580,537 sf Peak Elev=714.98' 7. Storage=18,886 cf 6-Flow (cfs) 5-

Time (hours)

Attachment F: Hydrologic Analysis of Pre-Developed Points of Discharge Calculations



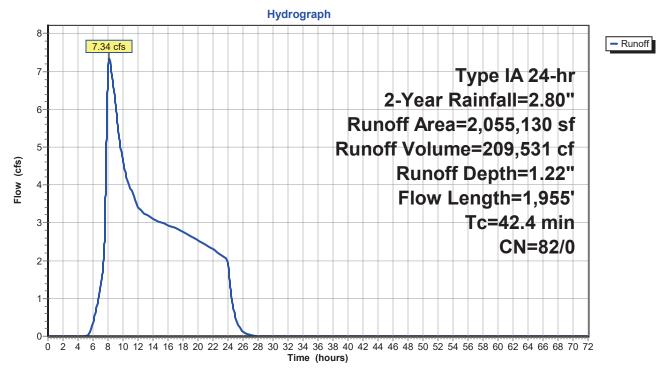
Summary for Subcatchment 1: Exist Point of Discharge #1

Runoff = 7.34 cfs @ 8.19 hrs, Volume= 209,531 cf, Depth= 1.22"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

_	A	rea (sf)	CN E	Description		
	2,055,130 82 Row crops, SR + CR, G					Good, HSG C
	2,055,130 82 100.00% Pervious Area			00.00% Pe	ervious Are	а
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	23.7	300	0.0300	0.21		Sheet Flow,
_	18.7	1,655	0.0270	1.48		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
_	42.4	1,955	Total			

Subcatchment 1: Exist Point of Discharge #1



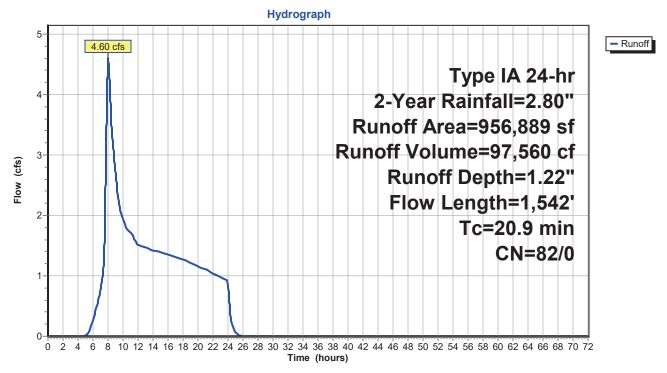
Summary for Subcatchment 2: Exist Point of Discharge #2

Runoff = 4.60 cfs @ 8.01 hrs, Volume= 97,560 cf, Depth= 1.22"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

A	rea (sf)	CN E	Description		
9	956,889	82 F	Row crops,	SR + CR,	Good, HSG C
ę	956,889 82 100.00% Pervious Area			ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.9	300	0.1670	0.42		Sheet Flow,
9.0	1,242	0.0660	2.31		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
20.9	1,542	Total			

Subcatchment 2: Exist Point of Discharge #2



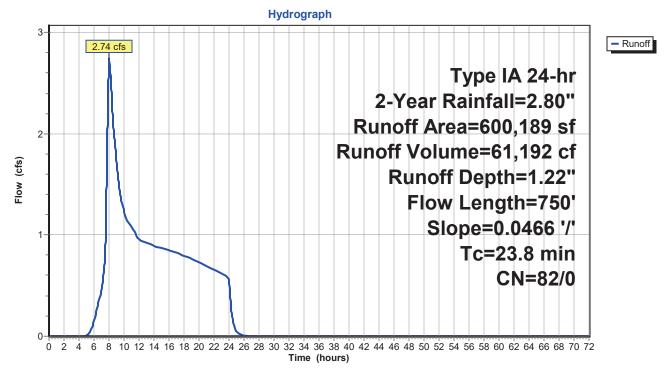
Summary for Subcatchment 3: Exist Point of Discharge #3

Runoff = 2.74 cfs @ 8.01 hrs, Volume= 61,192 cf, Depth= 1.22"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

_	A	rea (sf)	CN [Description		
600,189 82 Row crops, SR + CR, Good, HSG C						Good, HSG C
600,189 82 100.00% Pervious Area				00.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	19.9	300	0.0466	0.25		Sheet Flow,
_	3.9	450	0.0466	1.94		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
	23.8	750	Total			

Subcatchment 3: Exist Point of Discharge #3



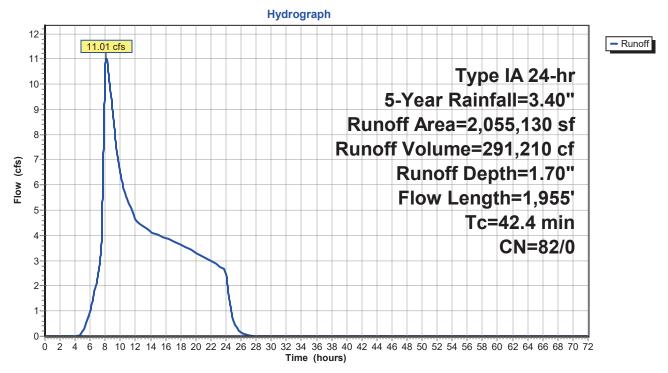
Summary for Subcatchment 1: Exist Point of Discharge #1

Runoff = 11.01 cfs @ 8.15 hrs, Volume= 291,210 cf, Depth= 1.70"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

_	A	rea (sf)	CN E	Description		
	2,0	55,130	82 F	Row crops,	SR + CR,	Good, HSG C
_	2,0	55,130	82 1	00.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	23.7	300	0.0300	0.21		Sheet Flow,
	18.7	1,655	0.0270	1.48		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
-	42.4	1,955	Total			

Subcatchment 1: Exist Point of Discharge #1



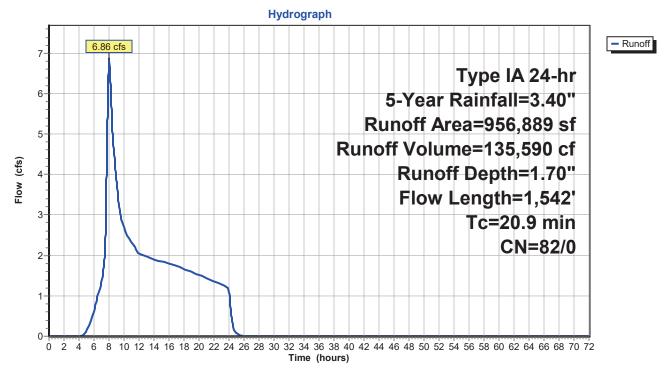
Summary for Subcatchment 2: Exist Point of Discharge #2

Runoff = 6.86 cfs @ 8.01 hrs, Volume= 135,590 cf, Depth= 1.70"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

	Area (sf)	CN [Description		
	956,889 82 Row crops, SR + CR, G				Good, HSG C
	956,889 82 100.00% Pervious Area		ervious Are	a	
To (min		Slope (ft/ft)		Capacity (cfs)	Description
11.9	300	0.1670	0.42	· · ·	Sheet Flow,
9.0) 1,242	0.0660	2.31		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
20.9) 1,542	Total			

Subcatchment 2: Exist Point of Discharge #2



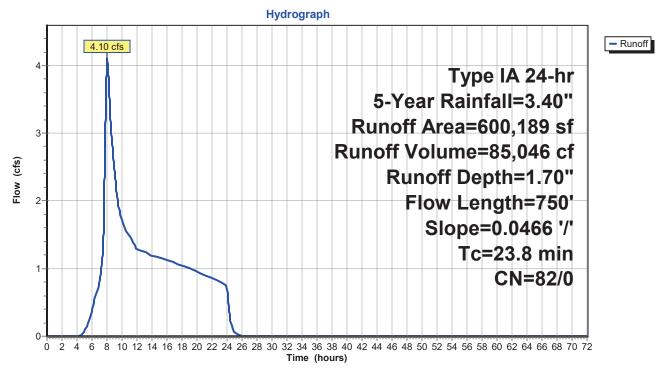
Summary for Subcatchment 3: Exist Point of Discharge #3

Runoff = 4.10 cfs @ 8.01 hrs, Volume= 85,046 cf, Depth= 1.70"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

	A	rea (sf)	CN [Description		
600,189 82 Row crops,					SR + CR,	Good, HSG C
600,189 82 100.00% Pervious Area			100.00% Pe	ervious Are	a	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	19.9	300	0.0466	0.25		Sheet Flow,
	3.9	450	0.0466	1.94		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
	23.8	750	Total			

Subcatchment 3: Exist Point of Discharge #3



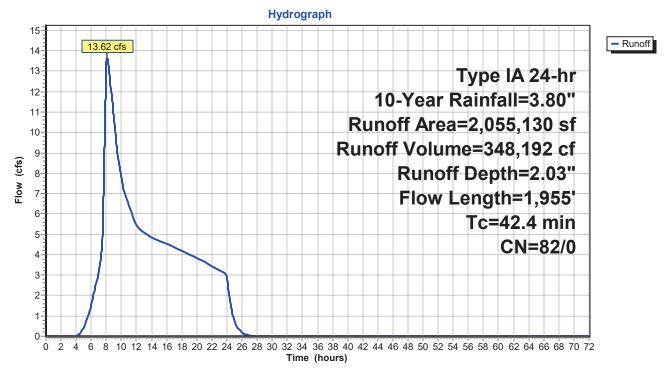
Summary for Subcatchment 1: Exist Point of Discharge #1

Runoff = 13.62 cfs @ 8.13 hrs, Volume= 348,192 cf, Depth= 2.03"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

_	A	rea (sf)	CN E	Description		
	2,0	55,130	82 F	Row crops,	SR + CR,	Good, HSG C
_	2,0	55,130	82 1	00.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	23.7	300	0.0300	0.21		Sheet Flow,
	18.7	1,655	0.0270	1.48		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
-	42.4	1,955	Total			

Subcatchment 1: Exist Point of Discharge #1



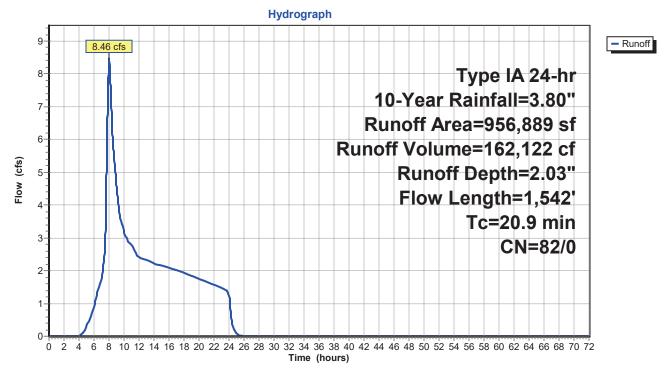
Summary for Subcatchment 2: Exist Point of Discharge #2

Runoff = 8.46 cfs @ 8.01 hrs, Volume= 162,122 cf, Depth= 2.03"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

	Area (sf)	CN [Description		
956,889 82 Row crops, SR + CR, G					Good, HSG C
	956,889 82 100.00% Pervious Area			ervious Are	a
Tc (min)		Slope (ft/ft)	,	Capacity (cfs)	Description
11.9	300	0.1670	0.42		Sheet Flow,
9.0	1,242	0.0660	2.31		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
20.9	1,542	Total			

Subcatchment 2: Exist Point of Discharge #2



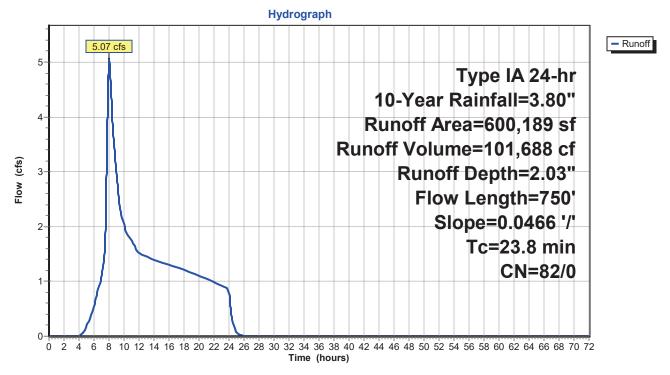
Summary for Subcatchment 3: Exist Point of Discharge #3

Runoff = 5.07 cfs @ 8.01 hrs, Volume= 101,688 cf, Depth= 2.03"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

_	A	rea (sf)	CN E	Description					
	6	00,189	82 F	Row crops, SR + CR, Good, HSG C					
600,189 82 100.00% Pe			00.00% Pe	ervious Are	a				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
-	19.9	300	0.0466	0.25		Sheet Flow,			
	3.9	450	0.0466	1.94		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow,			
	0.0	400	0.0400	1.04		Cultivated Straight Rows Kv= 9.0 fps			
-	23.8	750	Total						

Subcatchment 3: Exist Point of Discharge #3



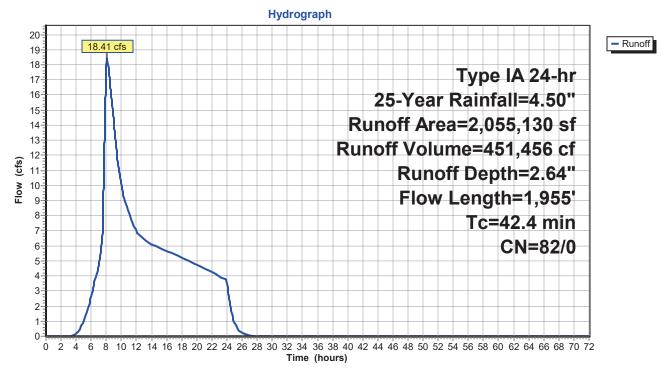
Summary for Subcatchment 1: Exist Point of Discharge #1

Runoff = 18.41 cfs @ 8.11 hrs, Volume= 451,456 cf, Depth= 2.64"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

_	A	rea (sf)	CN [Description		
	2,0	55,130	82 F	Row crops,	SR + CR,	Good, HSG C
-	2,0	55,130	82 1	100.00% Pe	ervious Are	а
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	23.7	300	0.0300	0.21		Sheet Flow,
						Cultivated: Residue>20% n= 0.170 P2= 2.80"
	18.7	1,655	0.0270	1.48		Shallow Concentrated Flow,
_						Cultivated Straight Rows Kv= 9.0 fps
	42.4	1,955	Total			

Subcatchment 1: Exist Point of Discharge #1



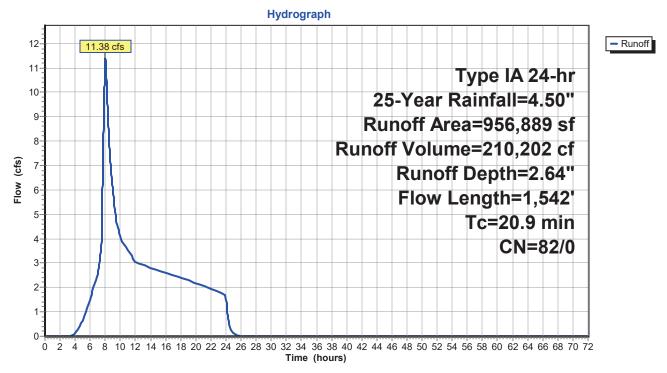
Summary for Subcatchment 2: Exist Point of Discharge #2

Runoff = 11.38 cfs @ 8.01 hrs, Volume= 210,202 cf, Depth= 2.64"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

A	rea (sf)	CN E	Description		
9	56,889	82 F	Row crops,	SR + CR,	Good, HSG C
9	56,889	82 1	00.00% Pe	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.9	300	0.1670	0.42		Sheet Flow,
9.0	1,242	0.0660	2.31		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps
20.9	1,542	Total			

Subcatchment 2: Exist Point of Discharge #2



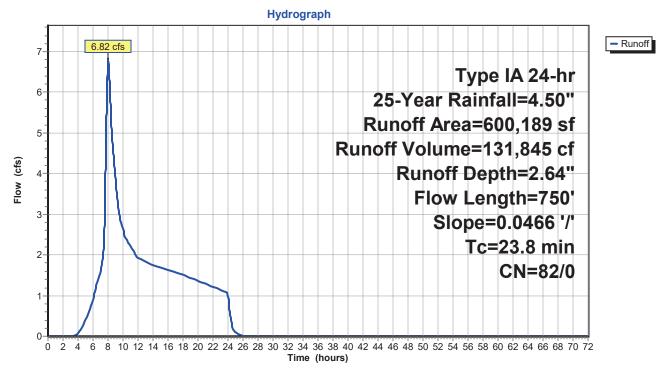
Summary for Subcatchment 3: Exist Point of Discharge #3

Runoff = 6.82 cfs @ 8.01 hrs, Volume= 131,845 cf, Depth= 2.64"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

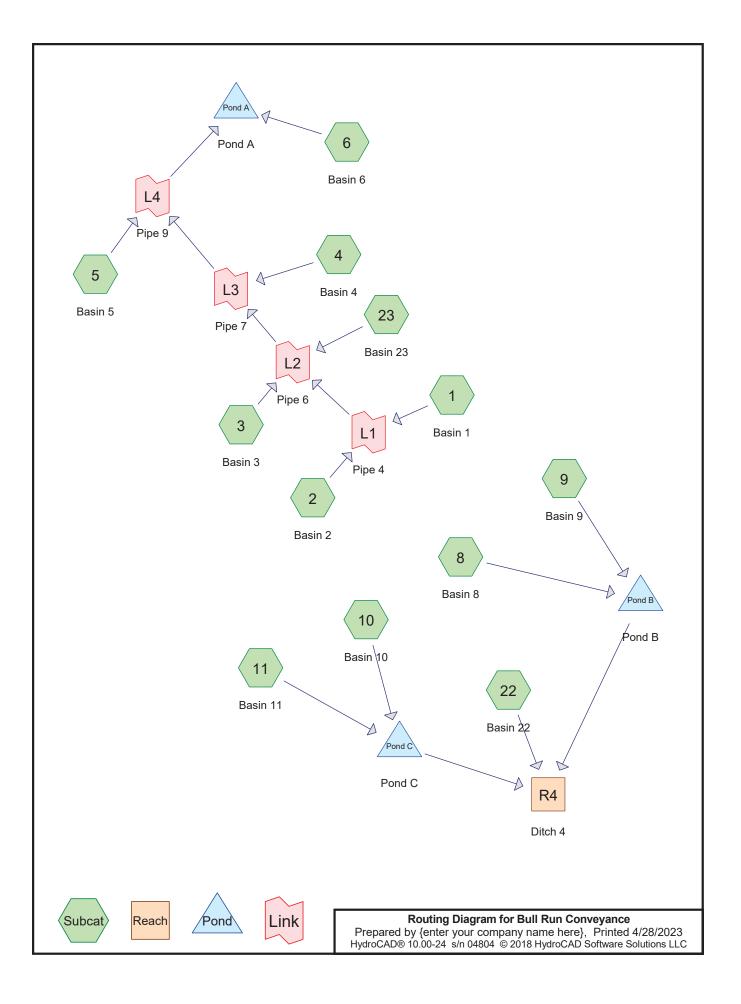
	A	rea (sf)	CN E	Description					
	6	00,189	82 F	Row crops,	SR + CR,	Good, HSG C			
600,189			82 1	82 100.00% Pervious Area					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
-	19.9	300	0.0466	0.25		Sheet Flow,			
	3.9	450	0.0466	1.94		Cultivated: Residue>20% n= 0.170 P2= 2.80" Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps			
	23.8	750	Total						

Subcatchment 3: Exist Point of Discharge #3



Attachment G: Conveyance System Calculations

HydroCAD Plots (Tributary Basins, Conveyance Ditches, Flow Splitter Manhole) Piped System Calculation Spreadsheet



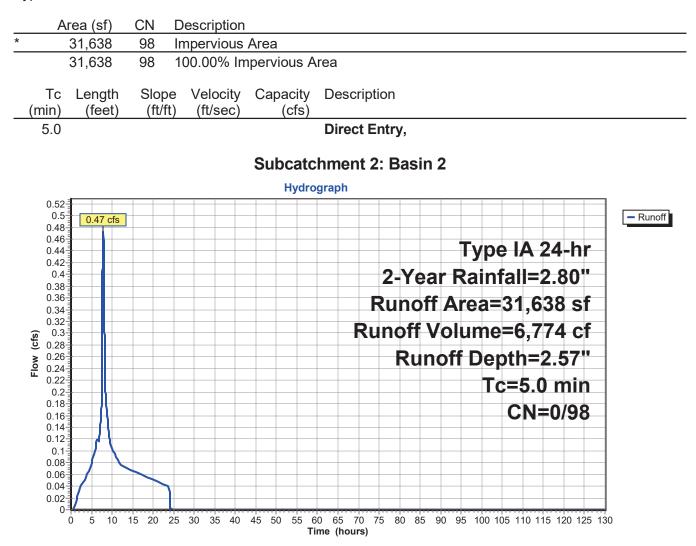
Summary for Subcatchment 1: Basin 1

Runoff = 0.51 cfs @ 7.92 hrs, Volume= 7,811 cf, Depth= 1.78"

	А	rea (sf)	CN	Description									
		24,364	98	Impervious									
		28,249		Pervious									
		52,613		Weighted A									
		28,249 24,364		53.69% Per 46.31% Imp									
		24,304	90 4	40.3170 mi		ea							
	Тс	Length	Slope	Velocity	Capacity	Description							
(r	nin)	(feet)	(ft/ft)	(ft/sec)	(cfs)								
	5.0					Direct Entr	у,						
					Subcato	hment 1: E	Basin	1					
Hydrograph													
	0.55	0.51 cfs											- Runoff
	0.5										21 k		
	0.45										24-ł		
	0.4					2	-Yea	ir R	aint	all=	2.80		
		-				Ru	nof	f Ar	ea=	52,6	613 s	sf	
	0.35					Run	مff ۱	/olu	Ime	=7 8	311 0	f	
Flow (cfs)	0.3	-								•			
_lo ≷	0.25						Run	ΟΤ	Deb	otn=	1.78		
-									Тс	;=5.	0 mi	n	
	0.2								(CN=	80/9	8	
	0.15	<u> </u>										-	
	0.1												
	0.05												
	0.05												
	0	0 5 10	15 20 2	5 30 35 40	45 50 55 6	60 65 70 75	80 85						

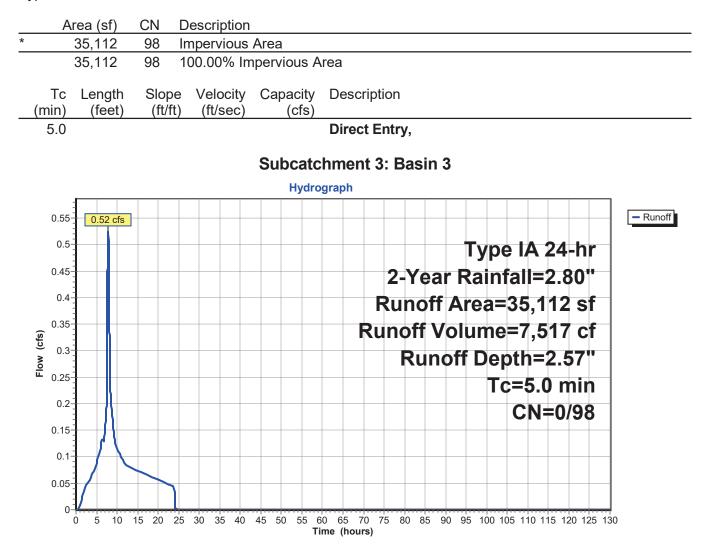
Summary for Subcatchment 2: Basin 2

Runoff = 0.47 cfs @ 7.88 hrs, Volume= 6,774 cf, Depth= 2.57"



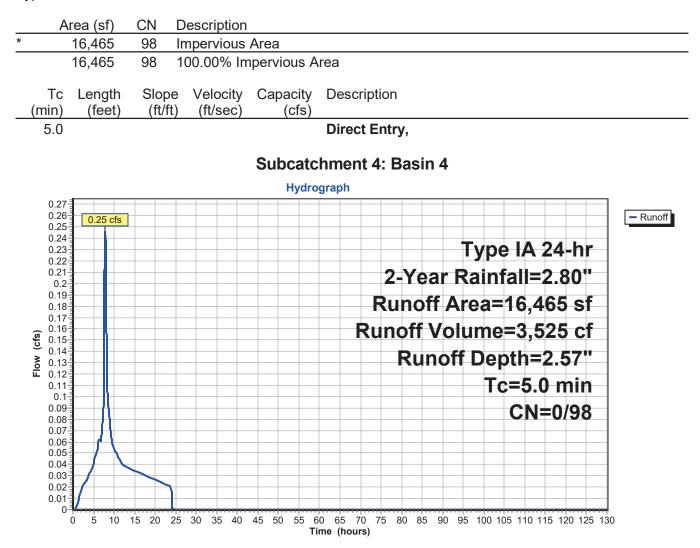
Summary for Subcatchment 3: Basin 3

Runoff = 0.52 cfs @ 7.88 hrs, Volume= 7,517 cf, Depth= 2.57"



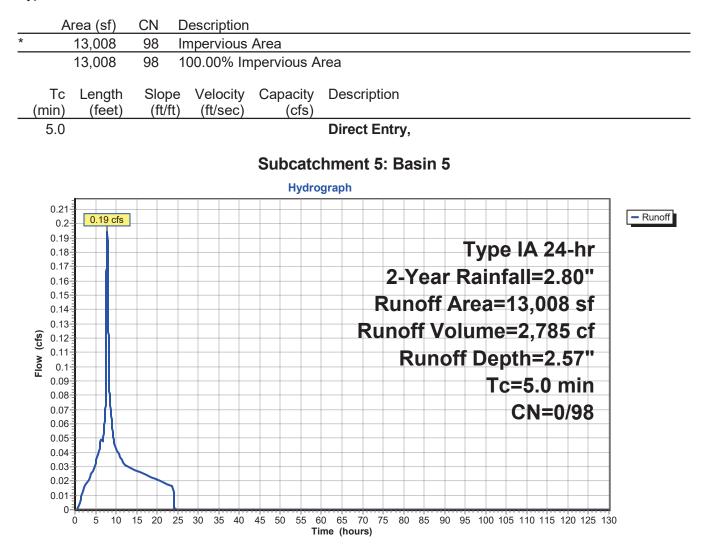
Summary for Subcatchment 4: Basin 4

Runoff = 0.25 cfs @ 7.88 hrs, Volume= 3,525 cf, Depth= 2.57"



Summary for Subcatchment 5: Basin 5

Runoff = 0.19 cfs @ 7.88 hrs, Volume= 2,785 cf, Depth= 2.57"



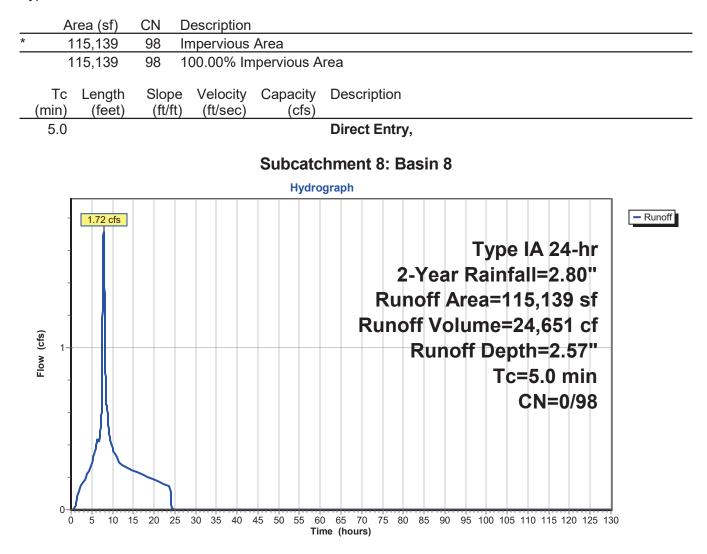
Summary for Subcatchment 6: Basin 6

Runoff = 1.09 cfs @ 7.97 hrs, Volume= 17,748 cf, Depth= 1.34"

	Area (sf)	CN D	escription								
*	133,479		Pervious								
*	25,640		mpervious	Verege							
	159,119 133,479		Veighted A 3.89% Per	verage vious Area							
	25,640 98 16.11% Impervious Area										
Т	c Length	Slope	Velocity	Capacity	Description						
(min	-	(ft/ft)	(ft/sec)	(cfs)	Description						
5.0 Direct Entry,											
	-			Hydro	graph						
	1.09 cfs							- Runoff			
	1					Ти	pe IA 24-hr				
					2_`		nfall=2.80''				
	-						159,119 sf				
fs)							=17,748 cf				
Flow (cfs)	-				F	Runoff Do	epth=1.34"				
Flo							Гc=5.0 min				
							CN=80/98				
	-										
						· · · · · · · · · · · · · · · · · · ·					
	0 5 10 1	5 20 25	30 35 40		0 65 70 75 80 ne (hours)	85 90 95 100	105 110 115 120 125 13	30			

Summary for Subcatchment 8: Basin 8

Runoff = 1.72 cfs @ 7.88 hrs, Volume= 24,651 cf, Depth= 2.57"



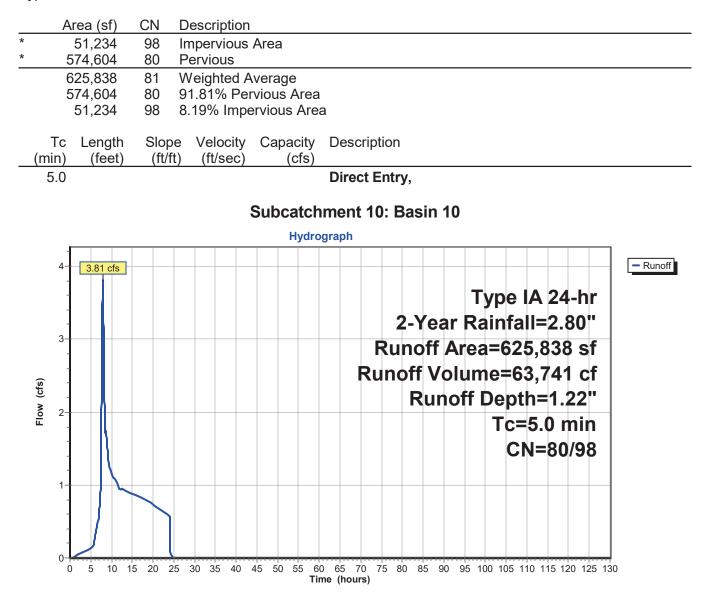
Summary for Subcatchment 9: Basin 9

Runoff = 1.37 cfs @ 7.95 hrs, Volume= 21,940 cf, Depth= 1.47"

A	vrea (sf)	CN E	escription		
*	44,929		npervious		
-	134,155		andscape		
	179,084 134,155		Veighted A	verage vious Area	2
	44,929			pervious Area	
	,				
Tc (min)	Length	Slope	Velocity		
<u>(min)</u> 5.0	(feet)	(ft/ft)	(ft/sec)	(cfs)	Direct Entry,
5.0					Direct Litti y,
				Subcatc	chment 9: Basin 9
				Hydro	ograph
- 1- 1-	1.37 cfs				Type IA 24-hr 2-Year Rainfall=2.80" Runoff Area=179,084 sf Runoff Volume=21,940 cf Runoff Depth=1.47" Tc=5.0 min CN=80/98
0– (0 5 10 1	5 20 25	30 35 40		60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 ime (hours)

Summary for Subcatchment 10: Basin 10

Runoff = 3.81 cfs @ 7.99 hrs, Volume= 63,741 cf, Depth= 1.22"



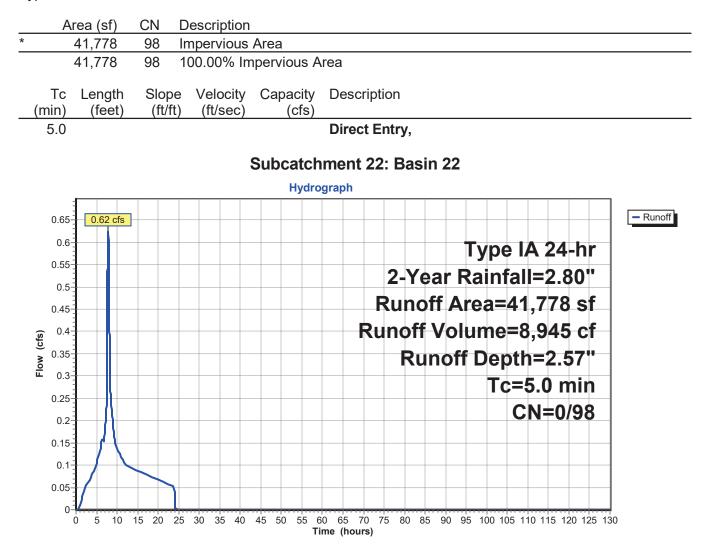
Summary for Subcatchment 11: Basin 11

Runoff = 0.82 cfs @ 7.97 hrs, Volume= 13,427 cf, Depth= 1.36"

		<u>rea (sf)</u> 20,813		Description Impervious							
		97,686	80	Pervious							
		18,499		Weighted A							
		97,686		82.44% Per							
		20,813	98	17.56% Imp	pervious Ar	ea					
	Тс	Length	Slope	Velocity	Capacity	Descriptio	n				
r	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	5.0					Direct En	try,				
				:	Subcatch	ment 11:	Basin 11				
		- i	· · · · ·		Hydro	graph					
	0.9· 0.85·										- Runof
	0.00										
	0.75							-	-	24-hr	
	0.7						2-Yea	r Ra	infall	=2.80"	
	0.65						Runoff /	Area	=118.	499 sf	
	0.6- 0.55-						noff Vo				
										=1.36"	
	0.45						Iterr		••••	.0 min	
	0.4									-	
	0.35	- IN							CN	=80/98	
	0.25										
	0.2										
	0.15										
	0.1		$ \rightarrow $								
	0.05										
	0-		<u></u>								

Summary for Subcatchment 22: Basin 22

Runoff = 0.62 cfs @ 7.88 hrs, Volume= 8,945 cf, Depth= 2.57"



Summary for Subcatchment 23: Basin 23

Runoff = 0.51 cfs @ 7.90 hrs, Volume= 7,474 cf, Depth= 2.16"

	А	rea (sf)	CN	Description											
ł		29,948	98	Impervious											
۲		11,565	80	Pervious											
		41,513	93	Weighted A											
		11,565	80	27.86% Per											
		29,948	98	72.14% lmp	Dervious Ar	ea									
	Тс	Length	Slop		Capacity	Desc	ription								
()	min)	(feet)	(ft/f	t) (ft/sec)	(cfs)										
	5.0					Direc	t Entry	y ,							
					Subcatch	nment	23: B	asir	ו 23						
					Hydro	ograph									
	0.55	- - 0.51 cfs													- Runoff
	0.5														
	0.45-									Ту	pe	IA	24	-hr	
							2-	Ye	ar f	Rai	infa	all=	2.8	0 "	
	0.4						Ru	nof	fΑ	rea	a=4	11.	513	sf	
	0.35-	-													
(cfc)	0.3	-					Runo								
Flow (cfs)	0.05	-						Rur	າof	f D	ер	th=	:2.1	6"	
ш	0.25	3									Тс	=5.	0 n	nin	
	0.2-										C	N=	80	98	
	0.15												.00/	30	
	0.1-	1/													
			\searrow												
	0.05	1/													
	0-	0 5 10	15 20	25 30 35 40		20 65 7				05 4/				0 125 4	120

Summary for Reach R4: Ditch 4

 Inflow Area =
 1,080,338 sf, 25.35% Impervious, Inflow Depth > 5.63" for 2-Year event

 Inflow =
 2.99 cfs @
 7.97 hrs, Volume=
 506,788 cf

 Outflow =
 2.99 cfs @
 7.97 hrs, Volume=
 506,763 cf, Atten= 0%, Lag= 0.3 min

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 4.65 fps, Min. Travel Time= 0.4 min Avg. Velocity = 3.53 fps, Avg. Travel Time= 0.5 min

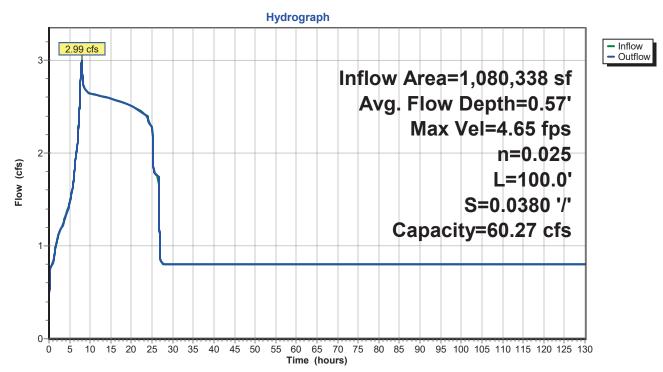
Peak Storage= 64 cf @ 7.97 hrs Average Depth at Peak Storage= 0.57' Bank-Full Depth= 1.75' Flow Area= 6.1 sf, Capacity= 60.27 cfs

Custom cross-section, Length= 100.0' Slope= 0.0380 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 694.00', Outlet Invert= 690.20'



	Offset	Elevation	Chan.Depth
_	(feet)	(feet)	(feet)
	-3.50	1.75	0.00
	0.00	0.00	1.75
	3.50	1.75	0.00

Depth E	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	0.0	0	0.00
1.75	6.1	7.8	613	60.27



Reach R4: Ditch 4

Summary for Pond Pond A: Pond A

Inflow Are	a =	349,468 sf, 50.41% Impervious, Inflow Depth = 1.84" for 2-Year event
Inflow	=	3.52 cfs @ 7.92 hrs, Volume= 53,634 cf
Outflow	=	0.61 cfs @ 14.82 hrs, Volume= 53,634 cf, Atten= 83%, Lag= 414.3 min
Primary	=	0.61 cfs @ 14.82 hrs, Volume= 53,634 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 709.36' @ 14.82 hrs Surf.Area= 8,132 sf Storage= 13,293 cf

Plug-Flow detention time= 246.7 min calculated for 53,630 cf (100% of inflow) Center-of-Mass det. time= 246.8 min (965.4 - 718.7)

Volume	Invert	t Avail.Sto	rage Storage D	escription	
#1	707.50	' 70,55	55 cf Custom S	Stage Data (Prisn	natic) Listed below (Recalc)
_	-	5 A		0.01	
Elevatio		urf.Area	Inc.Store	Cum.Store	
(fee	t)	(sq-ft)	(cubic-feet)	(cubic-feet)	
707.5	0	6,107	0	0	
708.0	0	6,714	3,205	3,205	
709.0	0	7,742	7,228	10,433	
710.0	0	8,825	8,284	18,717	
711.0	0	9,967	9,396	28,113	
712.0	0	11,161	10,564	38,677	
713.0	0	12,412	11,787	50,463	
714.0	0	13,717	13,065	63,528	
714.5	0	14,392	7,027	70,555	
Device	Routing	Invert	Outlet Devices		
#1	Primary	698.43'	18.0" Round C	Culvert	
	2		L= 138.0' CMF	^{>} , square edge h	eadwall, Ke= 0.500
					6.24' S= 0.0159 '/' Cc= 0.900
			n= 0.013, Flow	/ Area= 1.77 sf	
#2	Device 1	696.43'	,		.620 Limited to weir flow at low heads
#3	Device 1	709.40'	16.0" W x 3.0"	H Vert. Orifice/G	rate C= 0.620
			@ 14.82 hrs HW		Discharge)

1=Culvert (Passes 0.61 cfs of 24.10 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.61 cfs @ 16.45 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

Hydrograph 3.52 cfs Inflow Primary Inflow Area=349,468 sf Peak Elev=709.36' 3-Storage=13,293 cf Flow (cfs) 2 1 0.61 cfs 0-5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 Ó Time (hours)

Pond Pond A: Pond A

Summary for Pond Pond B: Pond B

Inflow Area	a =	294,223 sf, 54.40% Impervious, Inflow Depth = 1.90" for 2-Year event
Inflow	=	3.08 cfs @ 7.91 hrs, Volume= 46,591 cf
Outflow	=	0.67 cfs @ 11.01 hrs, Volume= 46,591 cf, Atten= 78%, Lag= 186.0 min
Primary	=	0.67 cfs @ 11.01 hrs, Volume= 46,591 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 709.12' @ 11.01 hrs Surf.Area= 5,248 sf Storage= 9,227 cf

Plug-Flow detention time= 143.1 min calculated for 46,587 cf (100% of inflow) Center-of-Mass det. time= 143.1 min (856.4 - 713.3)

Volume	Inver	t Avail.Sto	rage Storage [Description	
#1	707.00)' 36,8^	13 cf Custom	Stage Data (Pri	ismatic) Listed below (Recalc)
El su setti su			In a Otana	Ourse Otherse	
Elevatio		Surf.Area	Inc.Store	Cum.Store	
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	
707.00	C	3,493	0	0	
708.00	C	4,288	3,891	3,891	
709.00	C	5,139	4,714	8,604	
710.00	C	6,046	5,593	14,197	
711.00	C	7,010	6,528	20,725	
712.00	C	8,030	7,520	28,245	
713.00	C	9,107	8,569	36,813	
Device	Routing	Invert	Outlet Devices	;	
#1	Primary	705.04'	18.0" Round	Culvert	
	,		L= 339.0' CM	P. square edge	e headwall, Ke= 0.500
					701.65' S= 0.0100 '/' Cc= 0.900
				v Area= 1.77 sf	
#2	Device 1	703.04'	3.5" Horiz. Ori		
#3	Device 1	709.15'	14.0 VV X 3.0"	H Vert. Orifice	/Grate C= 0.620

Primary OutFlow Max=0.67 cfs @ 11.01 hrs HW=709.12' (Free Discharge)

-1=Culvert (Passes 0.67 cfs of 12.49 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.67 cfs @ 10.05 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

Bull Run Conveyance

Hydrograph 3.08 cfs Inflow Primary 3-Inflow Area=294,223 sf Peak Elev=709.12' Storage=9,227 cf 2 Flow (cfs) 1 0.67 cfs 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 Ó Time (hours)

Pond Pond B: Pond B

Summary for Pond Pond C: Pond C

Inflow Area	a =	744,337 sf,	9.68% Impervious,	Inflow Depth > 7.28" for 2-Year event
Inflow	=	5.43 cfs @	7.98 hrs, Volume=	451,597 cf, Incl. 0.80 cfs Base Flow
Outflow	=	1.85 cfs @ 1	15.18 hrs, Volume=	451,252 cf, Atten= 66%, Lag= 431.7 min
Primary	=	1.85 cfs @ 1	15.18 hrs, Volume=	451,252 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 704.77' @ 15.18 hrs Surf.Area= 20,253 sf Storage= 14,924 cf

Plug-Flow detention time= 35.8 min calculated for 451,247 cf (100% of inflow) Center-of-Mass det. time= 32.3 min (3,403.1 - 3,370.8)

Volume	Inver	t Avail.Sto	rage Storage	e Description		
#1	704.00	' 95,41	l6 cf Custon	n Stage Data (Pr	ismatic) Listed belo	ow (Recalc)
	-	C A				
Elevatior	ı S	urf.Area	Inc.Store	Cum.Store		
(feet)		(sq-ft)	(cubic-feet)	(cubic-feet)		
704.00		18,277	0	0		
705.00)	20,828	19,553	19,553		
706.00)	23,671	22,250	41,802		
707.00)	26,743	25,207	67,009		
708.00)	30,070	28,407	95,416		
Device	Routing	Invert	Outlet Device	es		
-	Primary	697.90'	12.0" Round	d Culvert		
	,, ,				headwall, Ke= 0.50	00
				· · · ·	695.55' S= 0.044	
				ow Area= 0.79 st		
#2	Device 1	693.90'	,			weir flow at low heads
	Device 1	704.85')" H Vert. Orifice		

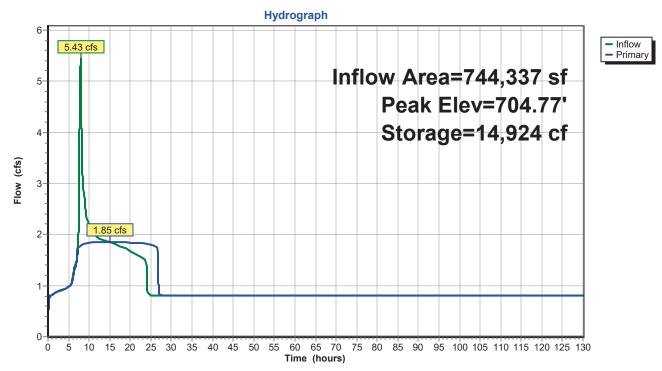
Primary OutFlow Max=1.85 cfs @ 15.18 hrs HW=704.77' (Free Discharge)

-1=Culvert (Passes 1.85 cfs of 9.55 cfs potential flow)

-2=Orifice/Grate (Orifice Controls 1.85 cfs @ 13.05 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

Bull Run Conveyance

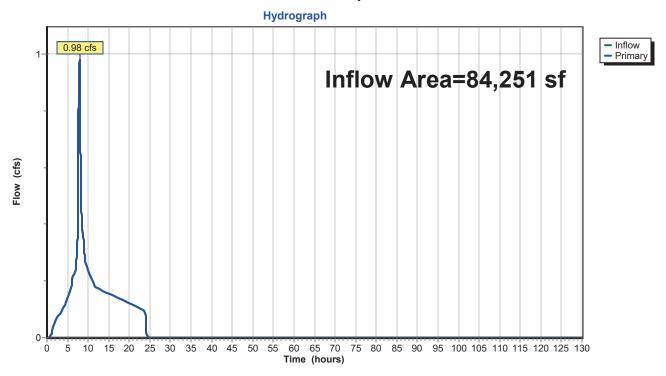


Pond Pond C: Pond C

Summary for Link L1: Pipe 4

Inflow Are	a =	84,251 sf,	66.47% Impervious,	Inflow Depth = 2.08"	for 2-Year event
Inflow	=	0.98 cfs @	7.90 hrs, Volume=	14,584 cf	
Primary	=	0.98 cfs @	7.90 hrs, Volume=	14,584 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs

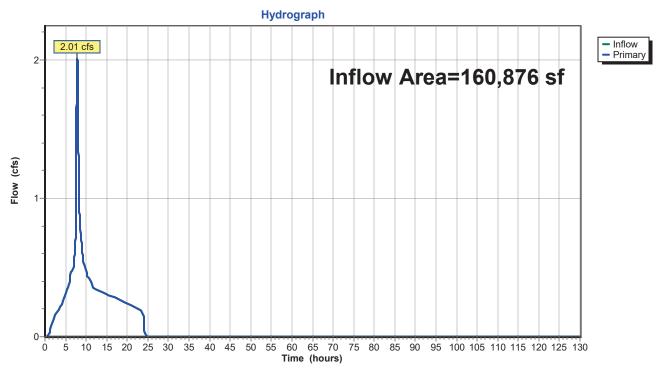


Link L1: Pipe 4

Summary for Link L2: Pipe 6

Inflow Area	a =	160,876 sf,	75.25% Impervious,	Inflow Depth = 2.21"	for 2-Year event
Inflow	=	2.01 cfs @	7.90 hrs, Volume=	29,575 cf	
Primary	=	2.01 cfs @	7.90 hrs, Volume=	29,575 cf, Atter	n= 0%, Lag= 0.0 min

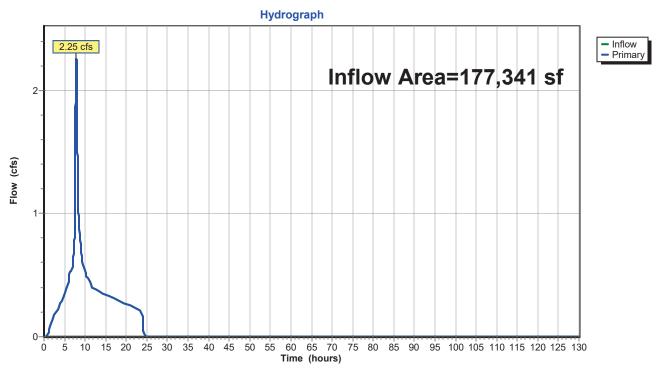
Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs



Link L2: Pipe 6

Inflow Area	a =	177,341 sf,	77.55% Impervious,	Inflow Depth = 2.24"	for 2-Year event
Inflow	=	2.25 cfs @	7.89 hrs, Volume=	33,101 cf	
Primary	=	2.25 cfs @	7.89 hrs, Volume=	33,101 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs



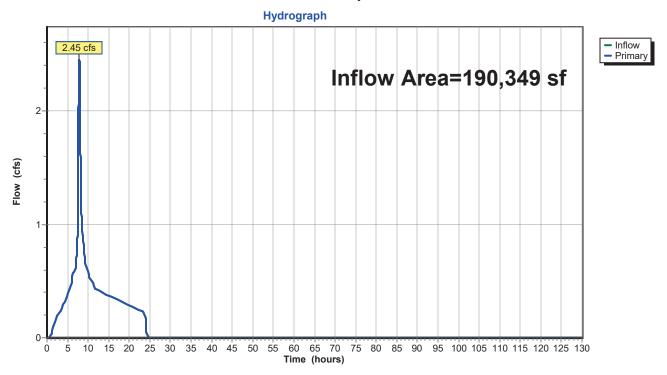
Link L3: Pipe 7

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Summary for Link L4: Pipe 9

Inflow Area	a =	190,349 sf,	79.08% Impervious,	Inflow Depth = 2.26"	for 2-Year event
Inflow	=	2.45 cfs @	7.89 hrs, Volume=	35,886 cf	
Primary	=	2.45 cfs @	7.89 hrs, Volume=	35,886 cf, Atte	n= 0%, Lag= 0.0 min

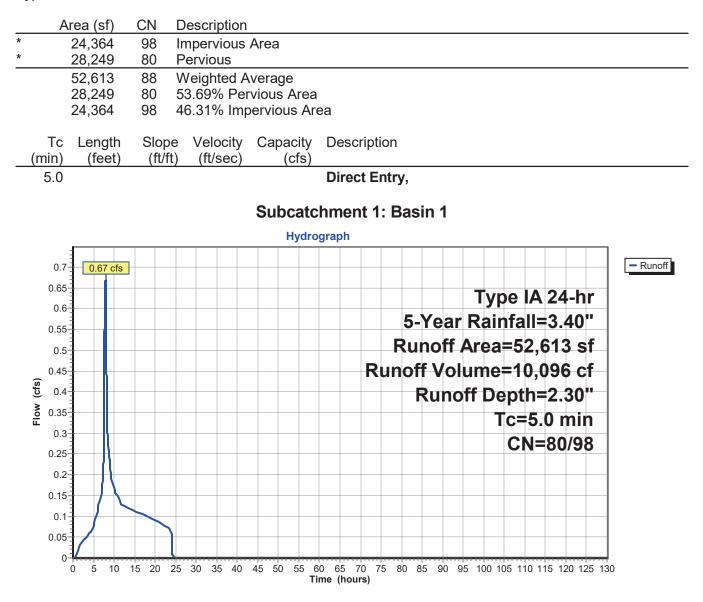
Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs



Link L4: Pipe 9

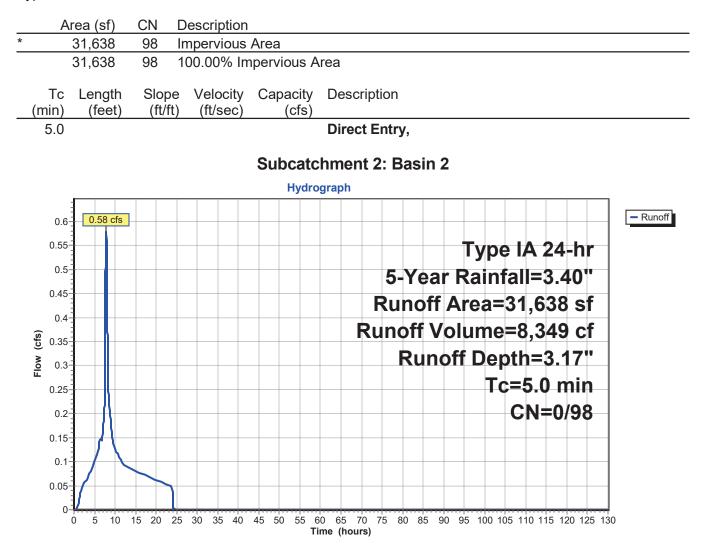
Summary for Subcatchment 1: Basin 1

Runoff = 0.67 cfs @ 7.92 hrs, Volume= 10,096 cf, Depth= 2.30"



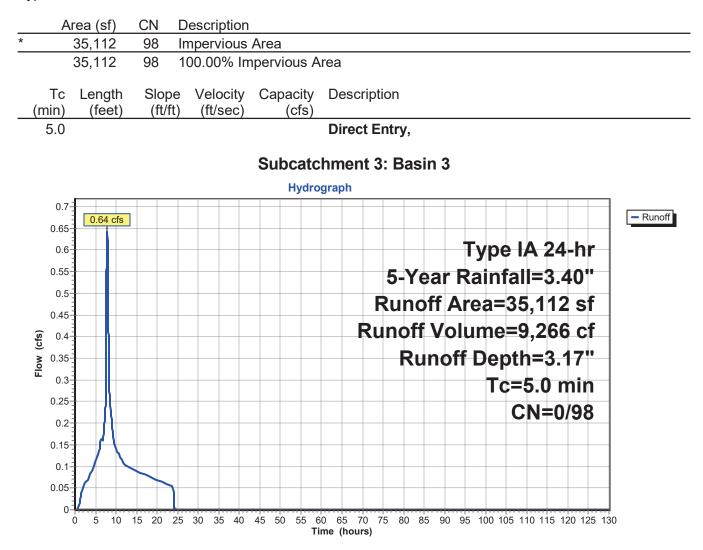
Summary for Subcatchment 2: Basin 2

Runoff = 0.58 cfs @ 7.88 hrs, Volume= 8,349 cf, Depth= 3.17"



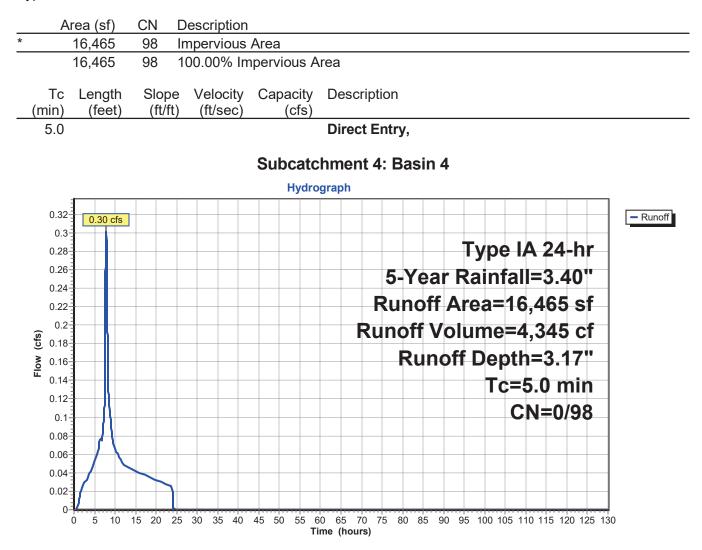
Summary for Subcatchment 3: Basin 3

Runoff = 0.64 cfs @ 7.88 hrs, Volume= 9,266 cf, Depth= 3.17"



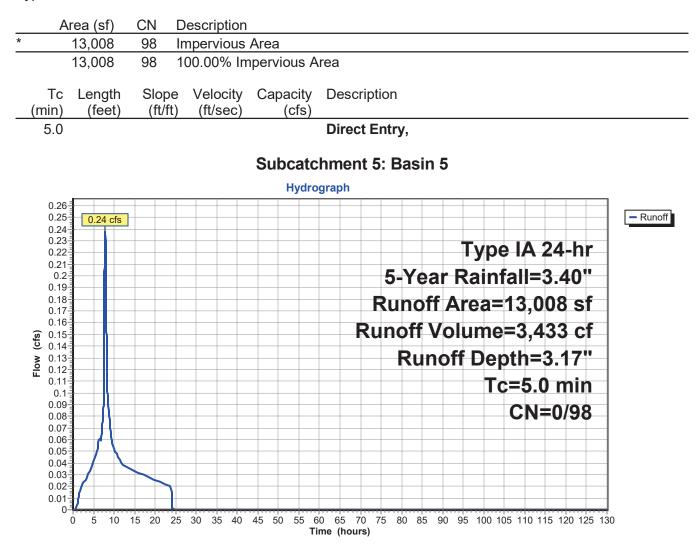
Summary for Subcatchment 4: Basin 4

Runoff = 0.30 cfs @ 7.88 hrs, Volume= 4,345 cf, Depth= 3.17"



Summary for Subcatchment 5: Basin 5

Runoff = 0.24 cfs @ 7.88 hrs, Volume= 3,433 cf, Depth= 3.17"



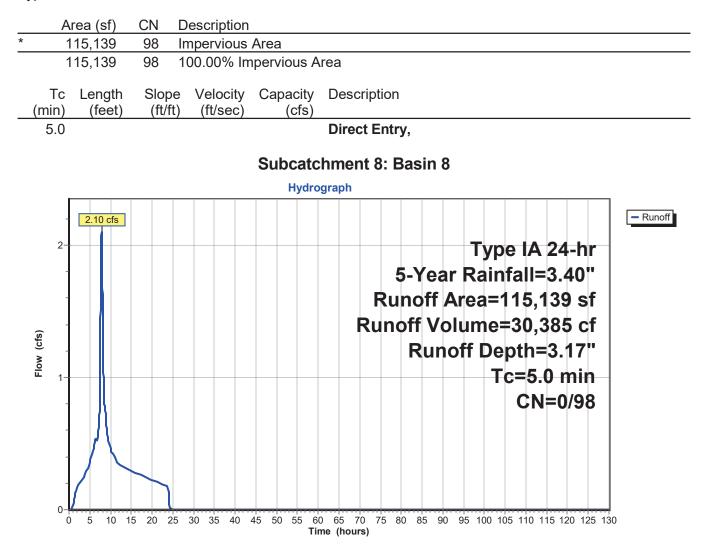
Summary for Subcatchment 6: Basin 6

Runoff = 1.54 cfs @ 7.96 hrs, Volume= 24,090 cf, Depth= 1.82"

Area (sf)	CN Description
,	80 Pervious
159,119 133,479	98Impervious83Weighted Average8083.89% Pervious Area9816.11% Impervious Area
Tc Length (min) (feet)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)
5.0	Direct Entry,
	Subcatchment 6: Basin 6
	Hydrograph
(s;) mol	Type IA 24-hr 5-Year Rainfall=3.40" Runoff Area=159,119 sf Runoff Volume=24,090 cf Runoff Depth=1.82" Tc=5.0 min CN=80/98

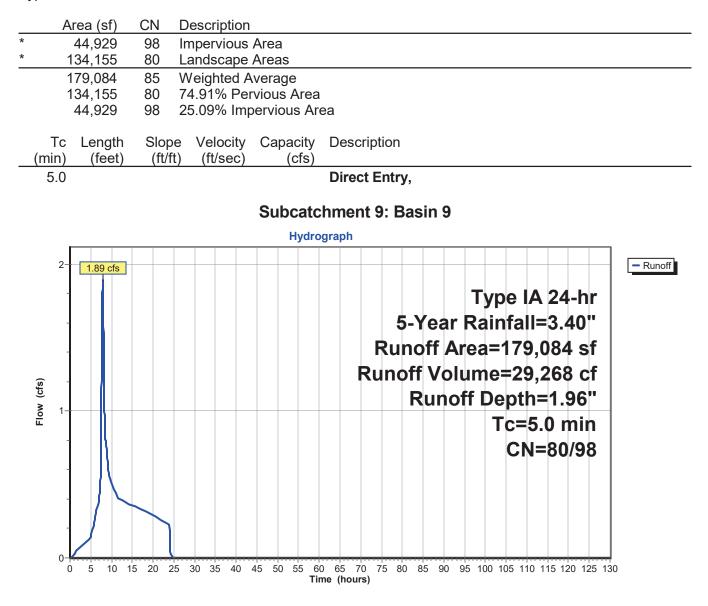
Summary for Subcatchment 8: Basin 8

Runoff = 2.10 cfs @ 7.88 hrs, Volume= 30,385 cf, Depth= 3.17"



Summary for Subcatchment 9: Basin 9

Runoff = 1.89 cfs @ 7.94 hrs, Volume= 29,268 cf, Depth= 1.96"



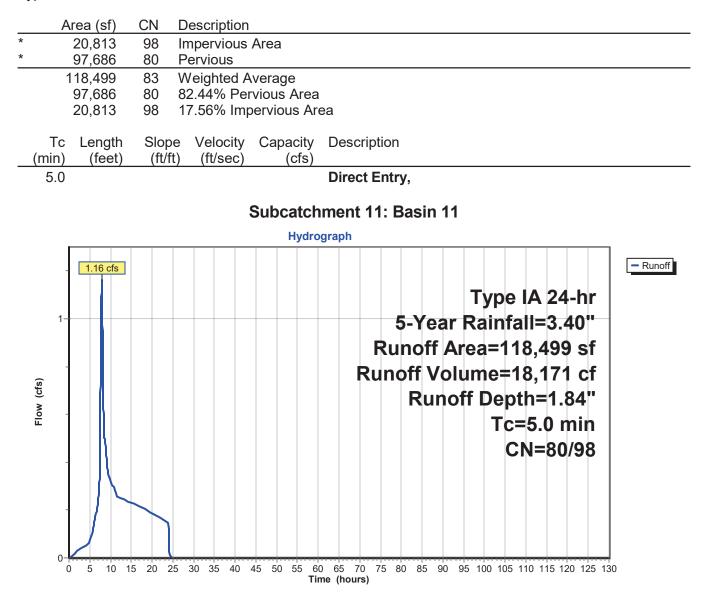
Summary for Subcatchment 10: Basin 10

Runoff = 5.55 cfs @ 7.97 hrs, Volume= 88,095 cf, Depth= 1.69"

*	Ļ	<u>vrea (sf)</u> 51,234 574,604 525,838	98 80	Description Impervious Pervious Weighted A	Area	
		574,604		91.81% Per		a
		51,234	98	8.19% Impe	ervious Are	a
_(Tc min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description
	5.0					Direct Entry,
				:	Subcatch	hment 10: Basin 10
					Hydro	ograph
	6-	5.55 cfs				Type IA 24-hr
	5					5-Year Rainfall=3.40" Runoff Area=625,838 sf
Elour (ofo)	4- (SLD) M					Runoff Volume=88,095 cf Runoff Depth=1.69"
Ū	2-					Tc=5.0 min CN=80/98
	- - - 1-					
	-					
	(0 5 10 1	5 20 25	30 35 40		60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 ime (hours)

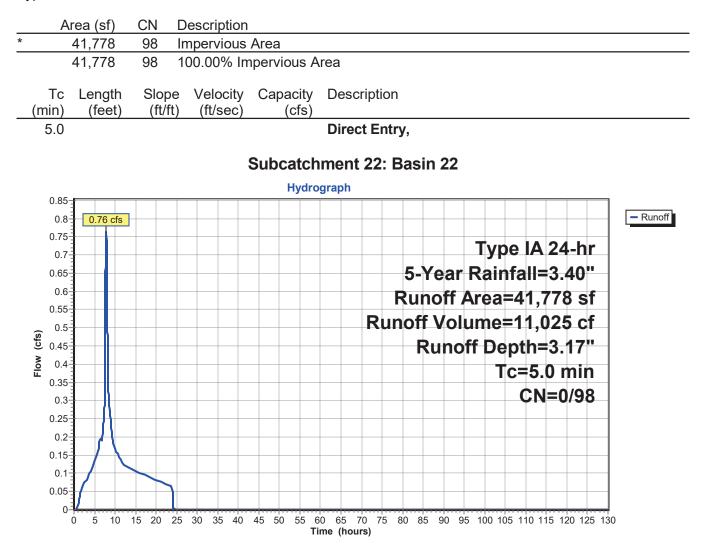
Summary for Subcatchment 11: Basin 11

Runoff = 1.16 cfs @ 7.95 hrs, Volume= 18,171 cf, Depth= 1.84"



Summary for Subcatchment 22: Basin 22

Runoff = 0.76 cfs @ 7.88 hrs, Volume= 11,025 cf, Depth= 3.17"



Summary for Subcatchment 23: Basin 23

Runoff = 0.64 cfs @ 7.89 hrs, Volume= 9,404 cf, Depth= 2.72"

	Aı	rea (sf)	CN	Description										
*		29,948	98	Impervious	Area									
*		11,565	80	Pervious										
		41,513		Weighted A										
		11,565		27.86% Per										
		29,948	98	72.14% Imp	bervious Ar	ea								
	Tc	Length	Slope			Descr	iption							
(<u>min)</u> 5.0	(feet)	(ft/ft) (ft/sec)	(cfs)	Direc	t Entry,	,						
									••					
					Subcatch		23: Ba	asın	23					
	0.7-				Hydro	ograph		1	1	1		1 1		
	0.65-	0.64 cfs												- Runoff
	0.6								T	уре	IA	24-	hr	
	0.55-						5-`	Yea	r Ra					
	0.5						Rur				-			
	0.45													
	(s) 0.4-						Runo				•			
-	(\$1) 0.4 0.35						R	Run	off I	Dep	th=	2.7	2"	
	0.3-									Тс	=5.	0 m	in	
	0.25									C	N=	80/	98	
	0.2- 0.15-													
	0.15-													
	0.05-		\searrow											
	0-	0 5 10	15 20 2	25 30 35 40		60 65 70 60 65 70		85 9	90 95	100 105	5 110 1	115 120) 125 13	0

Summary for Reach R4: Ditch 4

 Inflow Area =
 1,080,338 sf, 25.35% Impervious, Inflow Depth > 6.12" for 5-Year event

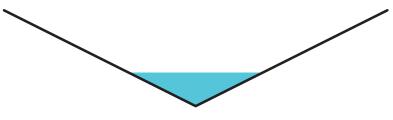
 Inflow =
 3.69 cfs @
 9.77 hrs, Volume=
 551,028 cf

 Outflow =
 3.69 cfs @
 9.77 hrs, Volume=
 551,003 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 4.90 fps, Min. Travel Time= 0.3 min Avg. Velocity = 3.57 fps, Avg. Travel Time= 0.5 min

Peak Storage= 75 cf @ 9.77 hrs Average Depth at Peak Storage= 0.61' Bank-Full Depth= 1.75' Flow Area= 6.1 sf, Capacity= 60.27 cfs

Custom cross-section, Length= 100.0' Slope= 0.0380 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 694.00', Outlet Invert= 690.20'



Offset	Elevation	Chan.Depth	
(feet)	(feet)	(feet)	
-3.50	1.75	0.00	
0.00	0.00	1.75	
3.50	1.75	0.00	

Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	0.0	0	0.00
1.75	6.1	7.8	613	60.27

Hydrograph 4 3.69 cfs Inflow - Outflow Inflow Area=1,080,338 sf Avg. Flow Depth=0.61' 3-Max Vel=4.90 fps n=0.025 Flow (cfs) L=100.0' 2 S=0.0380 '/' Capacity=60.27 cfs 1 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 Ó Time (hours)

Reach R4: Ditch 4

Summary for Pond Pond A: Pond A

Inflow Area	a =	349,468 sf,	50.41% Impervious,	Inflow Depth = 2.37" for 5-Year event
Inflow	=	4.59 cfs @	7.91 hrs, Volume=	68,983 cf
Outflow	=	1.18 cfs @	9.84 hrs, Volume=	68,983 cf, Atten= 74%, Lag= 115.6 min
Primary	=	1.18 cfs @	9.84 hrs, Volume=	68,983 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 709.65' @ 9.84 hrs Surf.Area= 8,451 sf Storage= 15,736 cf

Plug-Flow detention time= 240.8 min calculated for 68,983 cf (100% of inflow) Center-of-Mass det. time= 240.8 min (953.4 - 712.7)

Volume	Invert	Avail.Sto	rage Storage [Description	
#1	707.50	70,55	55 cf Custom	Stage Data (Prism	natic) Listed below (Recalc)
	-				
Elevation		urf.Area	Inc.Store	Cum.Store	
(feet)		(sq-ft)	(cubic-feet)	(cubic-feet)	
707.50		6,107	0	0	
708.00		6,714	3,205	3,205	
709.00		7,742	7,228	10,433	
710.00		8,825	8,284	18,717	
711.00		9,967	9,396	28,113	
712.00		11,161	10,564	38,677	
713.00		12,412	11,787	50,463	
714.00		13,717	13,065	63,528	
714.50		14,392	7,027	70,555	
Device F	Routing	Invert	Outlet Devices		
#1 F	Primary	698.43'	18.0" Round (Culvert	
			L= 138.0' CM	P, square edge he	adwall, Ke= 0.500
			Inlet / Outlet In	vert= 698.43' / 696	6.24' S= 0.0159 '/' Cc= 0.900
			n= 0.013, Flov	v Area= 1.77 sf	
#2 [Device 1	696.43'	2.6" Horiz. Ori	fice/Grate C= 0.	620 Limited to weir flow at low heads
#3 [Device 1	709.40'	16.0" W x 3.0"	H Vert. Orifice/G	rate C= 0.620
· · · · ·			@ 9.84 hrs HW=	=709.65' (Free Di	scharge)

1=Culvert (Passes 1.18 cfs of 24.40 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.61 cfs @ 16.67 fps)

-3=Orifice/Grate (Orifice Controls 0.57 cfs @ 1.70 fps)

Hydrograph 5 4.59 cfs Inflow Primary Inflow Area=349,468 sf 4 Peak Elev=709.65' Storage=15,736 cf 3-Flow (cfs) 2-1.18 cfs 1 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 Ó Time (hours)

Pond Pond A: Pond A

Summary for Pond Pond B: Pond B

Inflow Area	ı =	294,223 sf,	54.40% Impervious,	Inflow Depth = 2.43" for 5-Year event
Inflow	=	3.98 cfs @	7.91 hrs, Volume=	59,653 cf
Outflow	=	1.34 cfs @	8.97 hrs, Volume=	59,653 cf, Atten= 66%, Lag= 63.7 min
Primary	=	1.34 cfs @	8.97 hrs, Volume=	59,653 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 709.48' @ 8.97 hrs Surf.Area= 5,573 sf Storage= 11,166 cf

Plug-Flow detention time= 157.4 min calculated for 59,648 cf (100% of inflow) Center-of-Mass det. time= 157.4 min (864.9 - 707.5)

Volume	Inver	t Avail.Sto	age Storage Description				
#1	707.00)' 36,8^	13 cf Custom	Stage Data (Pri	ismatic) Listed below (Recalc)		
El su setti su			In a Otana	Ourse Otherse			
Elevatio		Surf.Area	Inc.Store	Cum.Store			
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)			
707.00	C	3,493	0	0			
708.00	C	4,288	3,891	3,891			
709.00	C	5,139	4,714	8,604			
710.00	C	6,046	5,593	14,197			
711.00	C	7,010	6,528	20,725			
712.00	C	8,030	7,520	28,245			
713.00	C	9,107	8,569	36,813			
Device	Routing	Invert	Outlet Devices	;			
#1	Primary	705.04'	18.0" Round	Culvert			
	,		L= 339.0' CM	P. square edge	e headwall, Ke= 0.500		
					701.65' S= 0.0100 '/' Cc= 0.900		
				v Area= 1.77 sf			
#2	Device 1	703.04'	3.5" Horiz. Ori				
#3	Device 1	709.15'	14.0 VV X 3.0"	H Vert. Orifice	/Grate C= 0.620		

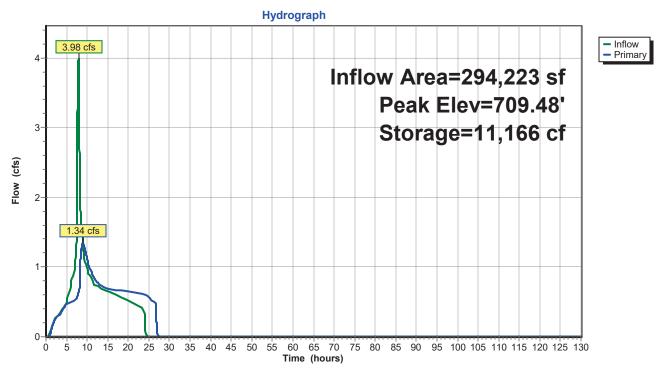
Primary OutFlow Max=1.34 cfs @ 8.97 hrs HW=709.48' (Free Discharge)

-**1=Culvert** (Passes 1.34 cfs of 12.86 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.70 cfs @ 10.48 fps)

-3=Orifice/Grate (Orifice Controls 0.64 cfs @ 2.21 fps)

Bull Run Conveyance



Pond Pond B: Pond B

Summary for Pond Pond C: Pond C

Inflow Area	=	744,337 sf,	9.68% Impervious,	Inflow Depth > 7.75" for 5-Year event
Inflow =	=	7.52 cfs @	7.97 hrs, Volume=	480,694 cf, Incl. 0.80 cfs Base Flow
Outflow =	=	2.48 cfs @	11.23 hrs, Volume=	480,349 cf, Atten= 67%, Lag= 195.8 min
Primary =	=	2.48 cfs @	11.23 hrs, Volume=	480,349 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 705.11' @ 11.23 hrs Surf.Area= 21,150 sf Storage= 21,926 cf

Plug-Flow detention time= 49.4 min calculated for 480,345 cf (100% of inflow) Center-of-Mass det. time= 46.1 min (3,257.9 - 3,211.9)

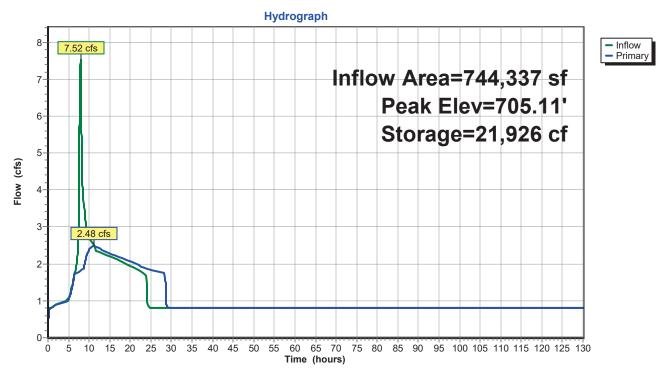
Volume	Invei	t Avail.Sto	rage Storage	e Description		
#1	704.00)' 95,47	16 cf Custon	n Stage Data (Pr	ismatic) Listed below (Recalc)	
				0		
Elevatio		Surf.Area	Inc.Store	Cum.Store		
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)		
704.0	00	18,277	0	0		
705.0	00	20,828	19,553	19,553		
706.0	00	23,671	22,250	41,802		
707.0	00	26,743	25,207	67,009		
708.0	00	30,070	28,407	95,416		
Device	Routing	Invert	Outlet Device	es		
#1	Primary	697.90'	12.0" Round	d Culvert		
	L= 53.0' CMP, square edge headwall, Ke= 0.500					
					695.55' S= 0.0443 '/' Cc= 0.900	
			n= 0.013. Fl	ow Area= 0.79 sf		
#2	Device 1	693.90'	5.1" Horiz. Orifice/Grate C= 0.620 Limited to weir flow at low heads			
#3	Device 1	704.85'				

Primary OutFlow Max=2.49 cfs @ 11.23 hrs HW=705.11' (Free Discharge)

-1=Culvert (Passes 2.49 cfs of 9.80 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.90 cfs @ 13.36 fps)

-3=Orifice/Grate (Orifice Controls 0.59 cfs @ 1.77 fps)

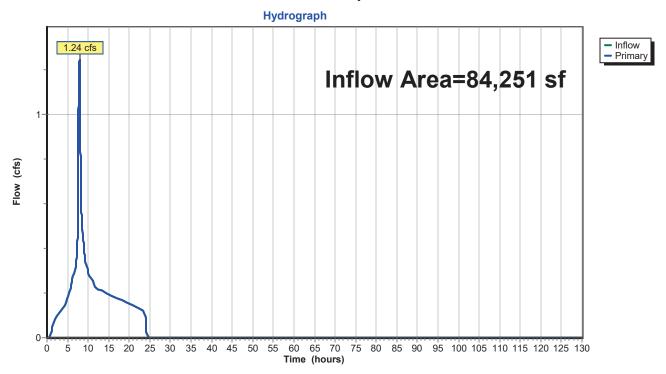


Pond Pond C: Pond C

Summary for Link L1: Pipe 4

Inflow Area =		84,251 sf,	66.47% Impervious,	Inflow Depth = 2.63"	for 5-Year event
Inflow	=	1.24 cfs @	7.90 hrs, Volume=	18,445 cf	
Primary	=	1.24 cfs @	7.90 hrs, Volume=	18,445 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs

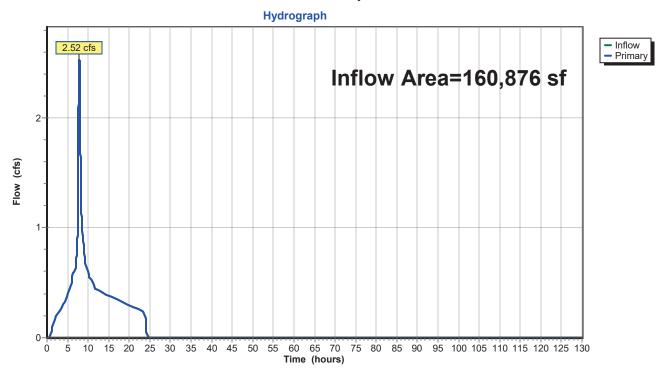


Link L1: Pipe 4

Summary for Link L2: Pipe 6

Inflow Area	a =	160,876 sf,	75.25% Impervious,	Inflow Depth = 2.77"	for 5-Year event
Inflow	=	2.52 cfs @	7.89 hrs, Volume=	37,115 cf	
Primary	=	2.52 cfs @	7.89 hrs, Volume=	37,115 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs

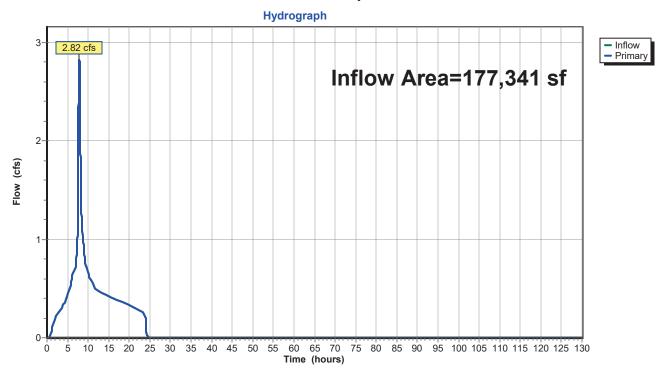


Link L2: Pipe 6

Summary for Link L3: Pipe 7

Inflow Are	a =	177,341 sf,	77.55% Impervious,	Inflow Depth = 2.81"	for 5-Year event
Inflow	=	2.82 cfs @	7.89 hrs, Volume=	41,460 cf	
Primary	=	2.82 cfs @	7.89 hrs, Volume=	41,460 cf, Atte	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs

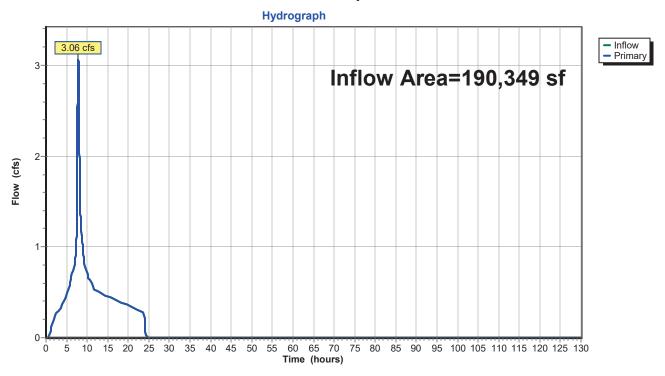


Link L3: Pipe 7

Summary for Link L4: Pipe 9

Inflow Area =		190,349 sf,	79.08% Impervious,	Inflow Depth = 2.83"	for 5-Year event
Inflow	=	3.06 cfs @	7.89 hrs, Volume=	44,893 cf	
Primary	=	3.06 cfs @	7.89 hrs, Volume=	44,893 cf, Atte	en= 0%, Lag= 0.0 min

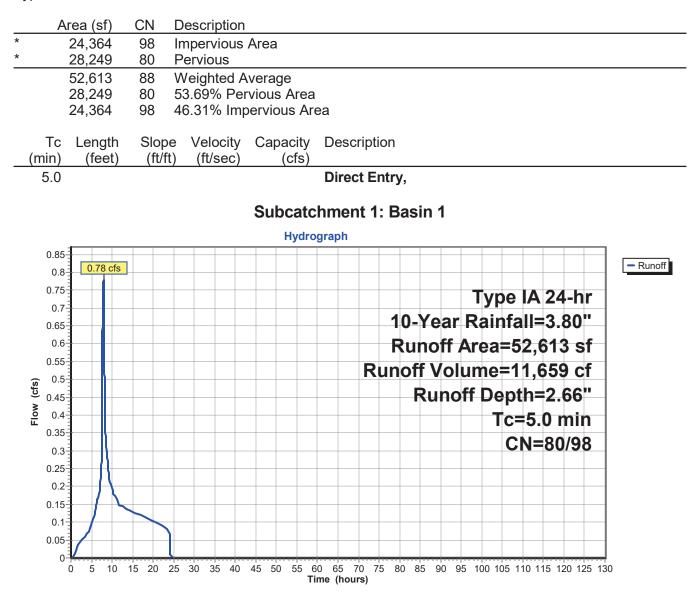
Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs



Link L4: Pipe 9

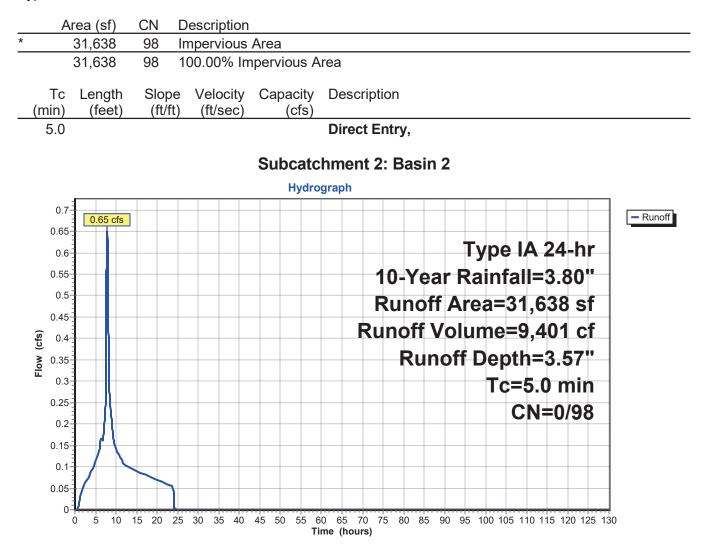
Summary for Subcatchment 1: Basin 1

Runoff = 0.78 cfs @ 7.91 hrs, Volume= 11,659 cf, Depth= 2.66"



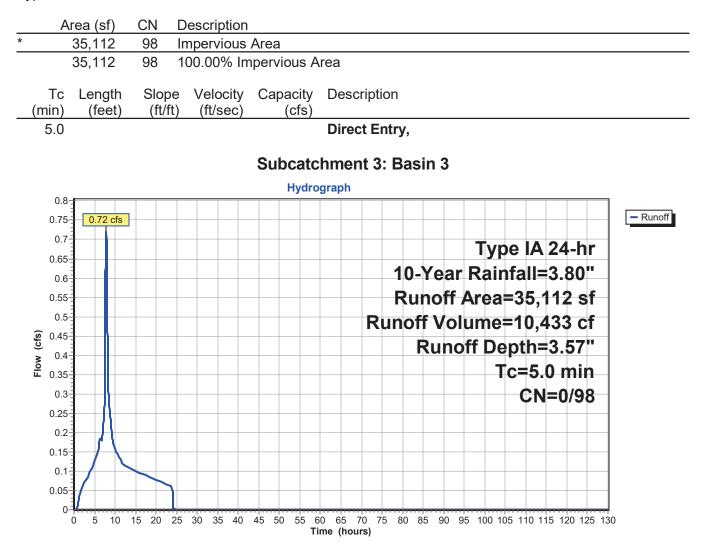
Summary for Subcatchment 2: Basin 2

Runoff = 0.65 cfs @ 7.88 hrs, Volume= 9,401 cf, Depth= 3.57"



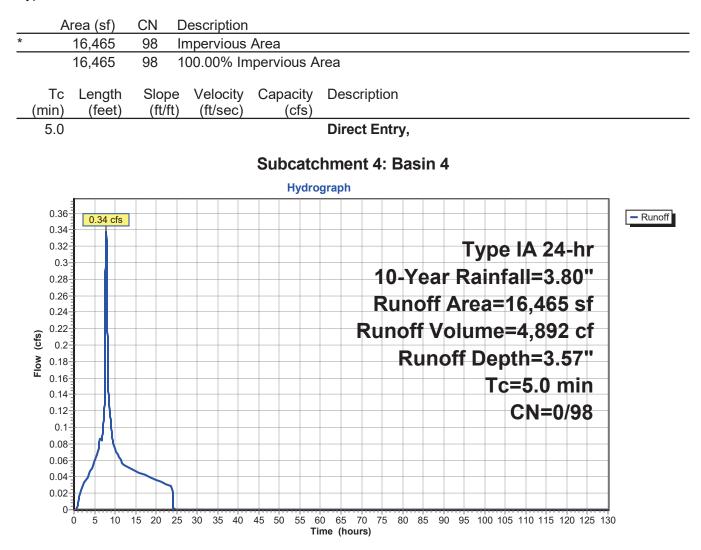
Summary for Subcatchment 3: Basin 3

Runoff = 0.72 cfs @ 7.88 hrs, Volume= 10,433 cf, Depth= 3.57"



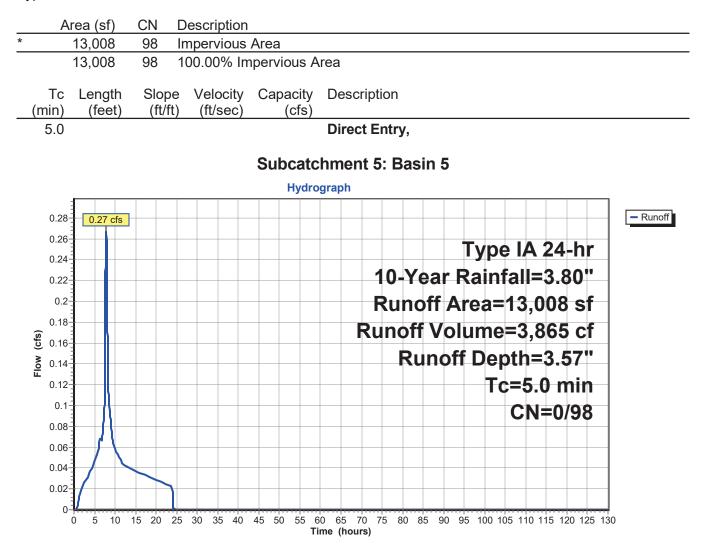
Summary for Subcatchment 4: Basin 4

Runoff = 0.34 cfs @ 7.88 hrs, Volume= 4,892 cf, Depth= 3.57"



Summary for Subcatchment 5: Basin 5

Runoff = 0.27 cfs @ 7.88 hrs, Volume= 3,865 cf, Depth= 3.57"



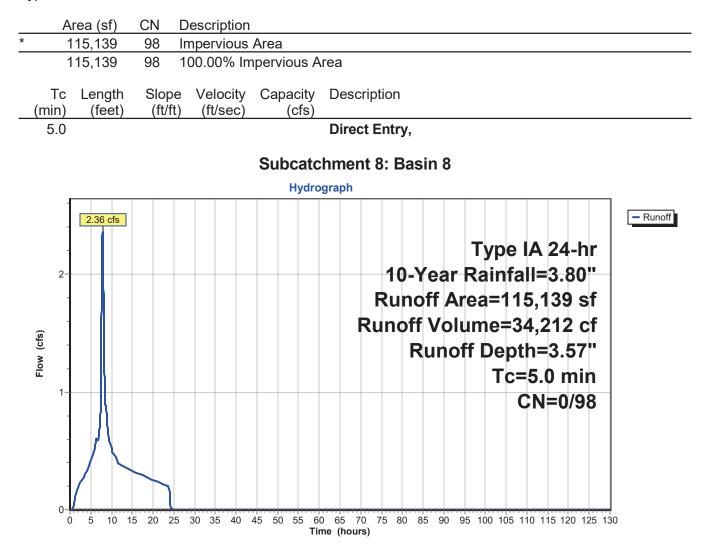
Summary for Subcatchment 6: Basin 6

Runoff = 1.85 cfs @ 7.95 hrs, Volume= 28,503 cf, Depth= 2.15"

Α	vrea (sf)	CN E	escription		
* *	133,479		Pervious		
	<u>25,640</u> 159,119		mpervious Veighted A	Verage	
	133,479			vious Area	a
	25,640	98 1	6.11% Imp	ervious Ar	ea
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
5.0					Direct Entry,
				Subcato	chment 6: Basin 6
				Hydro	ograph
2-	1.85 cfs				
-	1.00 013				
					Type IA 24-hr
-					10-Year Rainfall=3.80"
-					Runoff Area=159,119 sf
(Runoff Volume=28,503 cf
Flow (cfs)					Runoff Depth=2.15"
Flov					Tc=5.0 min
-					CN=80/98
-					
_					
-					
0-	0 5 10 1	5 20 25	30 35 40	45 50 55 6	0 65 70 75 80 85 90 95 100 105 110 115 120 125 130
· · · · · ·		20 20			me (hours)

Summary for Subcatchment 8: Basin 8

Runoff = 2.36 cfs @ 7.88 hrs, Volume= 34,212 cf, Depth= 3.57"



Summary for Subcatchment 9: Basin 9

Runoff = 2.25 cfs @ 7.94 hrs, Volume= 34,341 cf, Depth= 2.30"

Area (sf) CN Description	
* 44,929 98 Impervious Area	
* 134,155 80 Landscape Areas	
179,084 85 Weighted Average 134,155 80 74.91% Pervious Area	
44,929 98 25.09% Impervious Area	a
The logistic Olympic Materiae Operation	Description
Tc Length Slope Velocity Capacity ((min) (feet) (ft/ft) (ft/sec) (cfs)	Description
	Direct Entry,
Subcatch	iment 9: Basin 9
Hydrog	raph
2.25 cfs	- Runoff
	Type IA 24-hr
2-	10-Year Rainfall=3.80"
	Runoff Area=179,084 sf
ω.	Runoff Volume=34,341 cf
Flow (cfs)	Runoff Depth=2.30"
	Tc=5.0 min
	CN=80/98
0	
0 5 10 15 20 25 30 35 40 45 50 55 60 Time	65 70 75 80 85 90 95 100 105 110 115 120 125 130 e (hours)

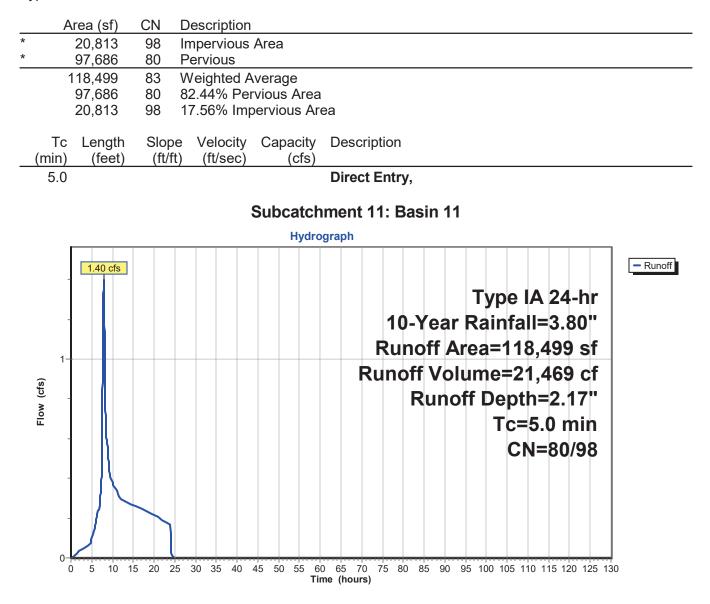
Summary for Subcatchment 10: Basin 10

Runoff = 6.79 cfs @ 7.96 hrs, Volume= 105,129 cf, Depth= 2.02"

* * (mir 5.	5 62 5 5	rea (sf) 51,234 74,604 25,838 74,604 51,234 Length (feet)	98 80 81 80		Area verage rvious Area		
0.	0						
						nment 10: Basin 10	
	7-	6.79 cfs			Hydro	ograph	- Runoff
	- - 6					Type IA 24-hr	
	-					10-Year Rainfall=3.80" Runoff Area=625,838 sf	
	5					Runoff Volume=105,129 cf	
Flow (cfs)	4					Runoff Depth=2.02"	
_	- - 3					Tc=5.0 min	
	- - -					CN=80/98	
	2						
	1		\searrow				
	-						
	0	5 10 1	5 20 25	30 35 40		0 65 70 75 80 85 90 95 100 105 110 115 120 125 130 me (hours)	

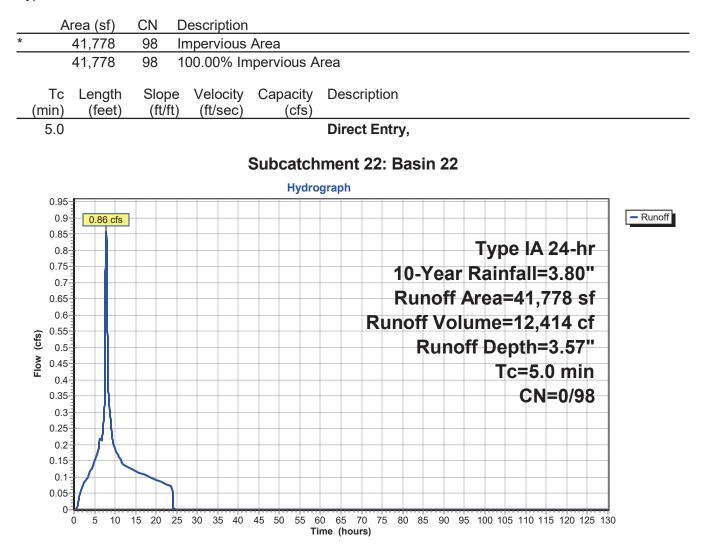
Summary for Subcatchment 11: Basin 11

Runoff = 1.40 cfs @ 7.95 hrs, Volume= 21,469 cf, Depth= 2.17"



Summary for Subcatchment 22: Basin 22

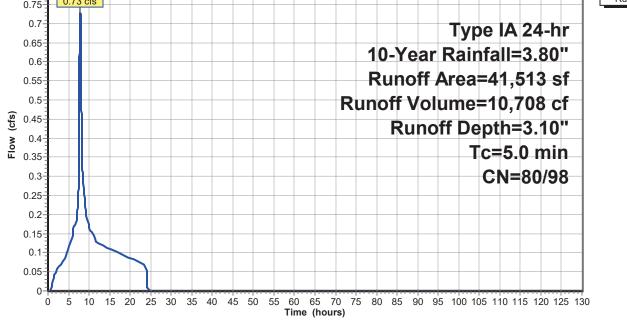
Runoff = 0.86 cfs @ 7.88 hrs, Volume= 12,414 cf, Depth= 3.57"



Summary for Subcatchment 23: Basin 23

Runoff = 0.73 cfs @ 7.89 hrs, Volume= 10,708 cf, Depth= 3.10"

	A	rea (sf)	CN	Description				
*		29,948	98	Impervious	Area			
*		11,565	80	Pervious				
		41,513	93	Weighted A	verage			
		11,565	80	27.86% Pe	rvious Area			
		29,948	98	72.14% Imp	pervious Are	ea		
	_		~		• •			
	Tc	Length	Slop		Capacity	Description		
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)			
	5.0					Direct Entry,		
					Subcatch	ment 23: Ba	sin 23	
					Hydro	graph		
	0.8							
	0.75	0.73 cfs						- Runoff
	0.7						Type IA 24-hr	



Summary for Reach R4: Ditch 4

 Inflow Area =
 1,080,338 sf, 25.35% Impervious, Inflow Depth > 6.46" for 10-Year event

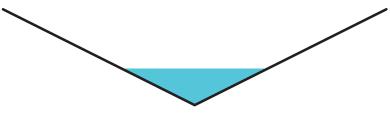
 Inflow =
 4.63 cfs @
 9.05 hrs, Volume=
 581,648 cf

 Outflow =
 4.63 cfs @
 9.06 hrs, Volume=
 581,624 cf, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 5.18 fps, Min. Travel Time= 0.3 min Avg. Velocity = 3.60 fps, Avg. Travel Time= 0.5 min

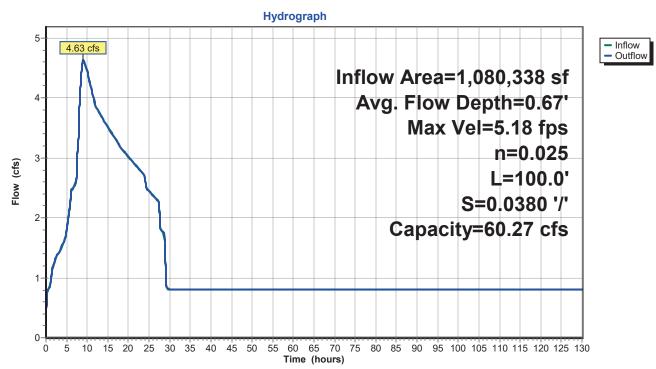
Peak Storage= 89 cf @ 9.06 hrs Average Depth at Peak Storage= 0.67' Bank-Full Depth= 1.75' Flow Area= 6.1 sf, Capacity= 60.27 cfs

Custom cross-section, Length= 100.0' Slope= 0.0380 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 694.00', Outlet Invert= 690.20'



	Offset	Elevation	Chan.Depth	
_	(feet)	(feet)	(feet)	
_	-3.50	1.75	0.00	
	0.00	0.00	1.75	
	3.50	1.75	0.00	

Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	0.0	0	0.00
1.75	6.1	7.8	613	60.27



Reach R4: Ditch 4

Summary for Pond Pond A: Pond A

Inflow Area	ı =	349,468 sf,	50.41% Impervious,	Inflow Depth = 2.73" for 10-Year event
Inflow	=	5.32 cfs @	7.91 hrs, Volume=	79,462 cf
Outflow	=	1.56 cfs @	9.22 hrs, Volume=	79,462 cf, Atten= 71%, Lag= 78.7 min
Primary	=	1.56 cfs @	9.22 hrs, Volume=	79,462 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 709.85' @ 9.22 hrs Surf.Area= 8,661 sf Storage= 17,391 cf

Plug-Flow detention time= 223.3 min calculated for 79,456 cf (100% of inflow) Center-of-Mass det. time= 223.3 min (932.4 - 709.1)

Volume	Inver	t Avail.Sto	age Storage Description			
#1	707.50	י' 70,5 <u>5</u>	55 cf Custom	Stage Data (Prismat	t ic) Listed below (Recalc)	
	_					
Elevatio		Surf.Area	Inc.Store	Cum.Store		
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)		
707.5	50	6,107	0	0		
708.0	00	6,714	3,205	3,205		
709.0	00	7,742	7,228	10,433		
710.0	00	8,825	8,284	18,717		
711.0	00	9,967	9,396	28,113		
712.0	00	11,161	10,564	38,677		
713.0	00	12,412	11,787	50,463		
714.0	00	13,717	13,065	63,528		
714.5	50	14,392	7,027	70,555		
Device	Routing	Invert	Outlet Devices	6		
#1	Primary	698.43'	18.0" Round	Culvert		
			L= 138.0' CN	IP, square edge head	dwall, Ke= 0.500	
			Inlet / Outlet Ir	nvert= 698.43' / 696.2	24' S= 0.0159 '/' Cc= 0.900	
			n= 0.013, Flo	w Area= 1.77 sf		
#2	Device 1	696.43'	2.6" Horiz. Or	ifice/Grate C= 0.62	20 Limited to weir flow at low heads	
#3	Device 1	709.40'	16.0" W x 3.0'	' H Vert. Orifice/Grat	te C= 0.620	
			9.22 hrs HW 24 60 cfs poter	=709.85' (Free Disc	harge)	

__1=Culvert (Passes 1.56 cfs of 24.60 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.62 cfs @ 16.81 fps)

-3=Orifice/Grate (Orifice Controls 0.94 cfs @ 2.81 fps)

Bull Run Conveyance

0-

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5

10 15 20 25 30 35 40 45 50 55

Hydrograph 5.32 cfs Inflow Primary Inflow Area=349,468 sf 5 Peak Elev=709.85' Storage=17,391 cf 4 Flow (cfs) 3-2-1.56 cfs 1

Time (hours)

60 65 70 75 80 85 90 95 100 105 110 115 120 125 130

Pond Pond A: Pond A

Summary for Pond Pond B: Pond B

Inflow Area =		294,223 sf,	54.40% Impervious,	Inflow Depth = 2.80" for 10-Year event
Inflow	=	4.60 cfs @	7.91 hrs, Volume=	68,552 cf
Outflow	=	1.70 cfs @	8.80 hrs, Volume=	68,552 cf, Atten= 63%, Lag= 53.7 min
Primary	=	1.70 cfs @	8.80 hrs, Volume=	68,552 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 709.73' @ 8.80 hrs Surf.Area= 5,802 sf Storage= 12,600 cf

Plug-Flow detention time= 154.7 min calculated for 68,552 cf (100% of inflow) Center-of-Mass det. time= 154.7 min (858.9 - 704.2)

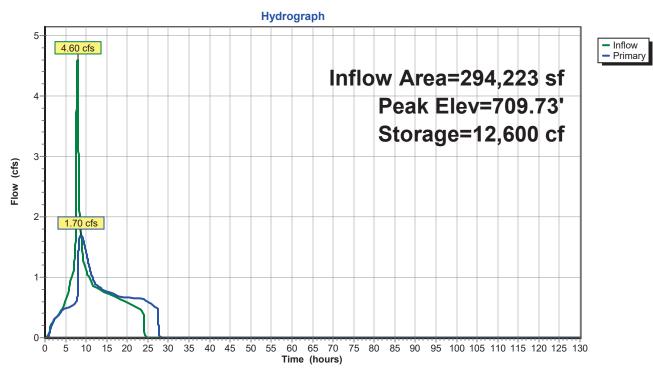
Volume	Inve	rt Avail.Sto	rage Storage [Description	
#1	707.0	D' 36,8 ⁻	13 cf Custom	Stage Data (Pri	smatic) Listed below (Recalc)
Flovetia	b	Curf Area	Inc Store	Cum Store	
Elevatio		Surf.Area	Inc.Store	Cum.Store	
(fee	t)	(sq-ft)	(cubic-feet)	(cubic-feet)	
707.0	0	3,493	0	0	
708.0	0	4,288	3,891	3,891	
709.0	0	5,139	4,714	8,604	
710.0	0	6,046	5,593	14,197	
711.0		7,010	6,528	20,725	
712.0	-	8,030	7,520	28,245	
713.0		9,107	8,569	36,813	
1 1010	0	0,101	0,000	00,010	
Device	Routing	Invert	Outlet Devices		
#1	Primary	705.04'	18.0" Round (Culvert	
	,		L= 339.0' CM	P. square edge	headwall, Ke= 0.500
					701.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flov		
#2	Device 1	703.04'	3.5" Horiz. Ori		
#3	Device 1	709.15'	14.0 VV X 3.0	H Vert. Orifice	/Grate C= 0.620

Primary OutFlow Max=1.70 cfs @ 8.80 hrs HW=709.73' (Free Discharge)

-1=Culvert (Passes 1.70 cfs of 13.11 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.72 cfs @ 10.78 fps)

-3=Orifice/Grate (Orifice Controls 0.98 cfs @ 3.35 fps)



Pond Pond B: Pond B

Summary for Pond Pond C: Pond C

Inflow Area =		744,337 sf,	9.68% Impervious,	Inflow Depth > 8.08" for 10-Year event
Inflow =	=	8.99 cfs @	7.96 hrs, Volume=	501,027 cf, Incl. 0.80 cfs Base Flow
Outflow =	=	2.87 cfs @	10.95 hrs, Volume=	500,682 cf, Atten= 68%, Lag= 179.2 min
Primary =	=	2.87 cfs @	10.95 hrs, Volume=	500,682 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 705.31' @ 10.95 hrs Surf.Area= 21,701 sf Storage= 26,080 cf

Plug-Flow detention time= 53.6 min calculated for 500,677 cf (100% of inflow) Center-of-Mass det. time= 50.3 min (3,161.6 - 3,111.3)

Volume	Inver	t Avail.Sto	rage Storage	e Description		
#1	704.00)' 95,4^	16 cf Custom	n Stage Data (Pr	rismatic) Listed below (Recalc)	
Elevetia		Sumf Anon	In a Starra	Curra Starra		
Elevatio		Surf.Area	Inc.Store	Cum.Store		
(fee	,	(sq-ft)	(cubic-feet)	(cubic-feet)		
704.0	00	18,277	0	0		
705.0	00	20,828	19,553	19,553		
706.0	00	23,671	22,250	41,802		
707.0	00	26,743	25,207	67,009		
708.0	00	30,070	28,407	95,416		
Device	Routing	Invert	Outlet Device	es		
#1	Primary	697.90'	12.0" Round	d Culvert		
	,		L= 53.0' CN	IP. square edge	headwall, Ke= 0.500	
					/ 695.55' S= 0.0443 '/' Cc= 0.900	
				ow Area= 0.79 sf		
#2	Device 1	693.90'	,		C= 0.620 Limited to weir flow at low heads	S
#3	Device 1	704.85')" H Vert. Orifice		0
π0	Device 1	104.00	10.0 11 × 0.0			

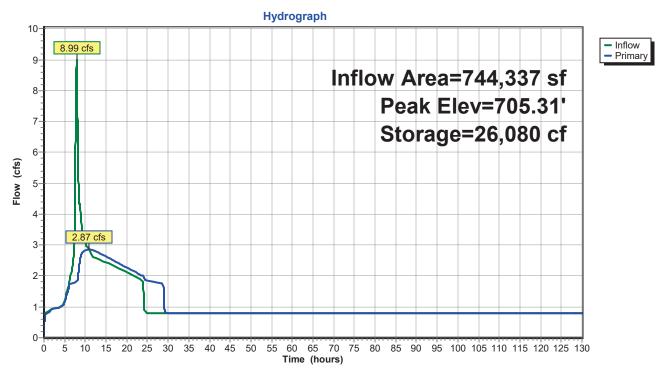
Primary OutFlow Max=2.87 cfs @ 10.95 hrs HW=705.31' (Free Discharge)

-1=Culvert (Passes 2.87 cfs of 9.94 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.92 cfs @ 13.54 fps)

-3=Orifice/Grate (Orifice Controls 0.95 cfs @ 2.85 fps)

Bull Run Conveyance

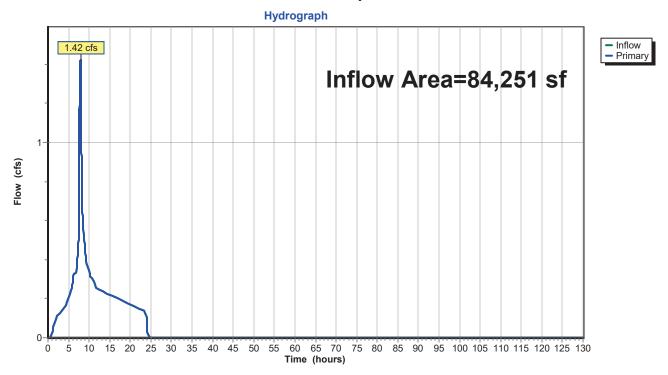


Pond Pond C: Pond C

Summary for Link L1: Pipe 4

Inflow Area	a =	84,251 sf,	66.47% Impervious,	Inflow Depth = 3.00"	for 10-Year event
Inflow	=	1.42 cfs @	7.90 hrs, Volume=	21,060 cf	
Primary	=	1.42 cfs @	7.90 hrs, Volume=	21,060 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs

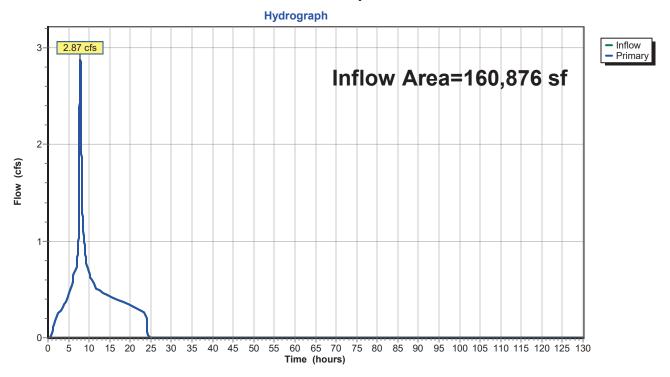


Link L1: Pipe 4

Summary for Link L2: Pipe 6

Inflow Are	a =	160,876 sf,	75.25% Impervious,	Inflow Depth = 3.15 "	for 10-Year event
Inflow	=	2.87 cfs @	7.89 hrs, Volume=	42,201 cf	
Primary	=	2.87 cfs @	7.89 hrs, Volume=	42,201 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs

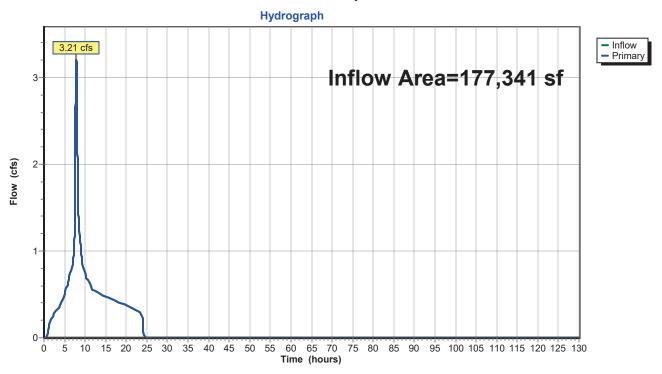


Link L2: Pipe 6

Summary for Link L3: Pipe 7

Inflow Area	a =	177,341 sf,	77.55% Impervious,	Inflow Depth = 3.19"	for 10-Year event
Inflow	=	3.21 cfs @	7.89 hrs, Volume=	47,093 cf	
Primary	=	3.21 cfs @	7.89 hrs, Volume=	47,093 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs

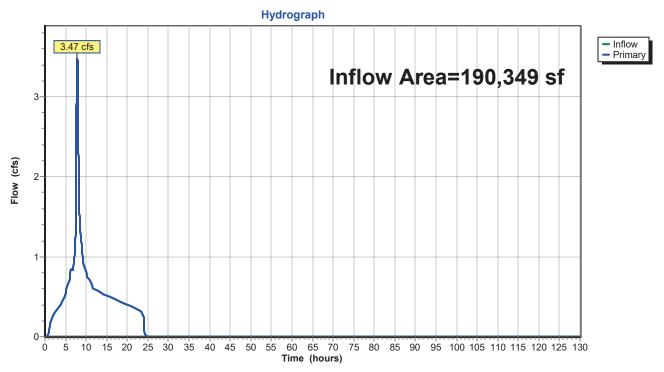


Link L3: Pipe 7

Summary for Link L4: Pipe 9

Inflow Area	a =	190,349 sf,	79.08% Impervious,	Inflow Depth = 3.21"	for 10-Year event
Inflow	=	3.47 cfs @	7.89 hrs, Volume=	50,959 cf	
Primary	=	3.47 cfs @	7.89 hrs, Volume=	50,959 cf, Atte	n= 0%, Lag= 0.0 min

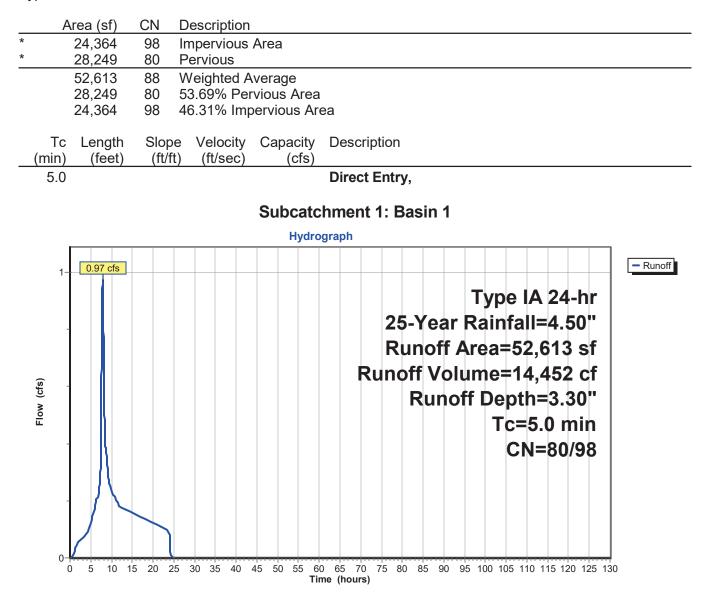
Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs



Link L4: Pipe 9

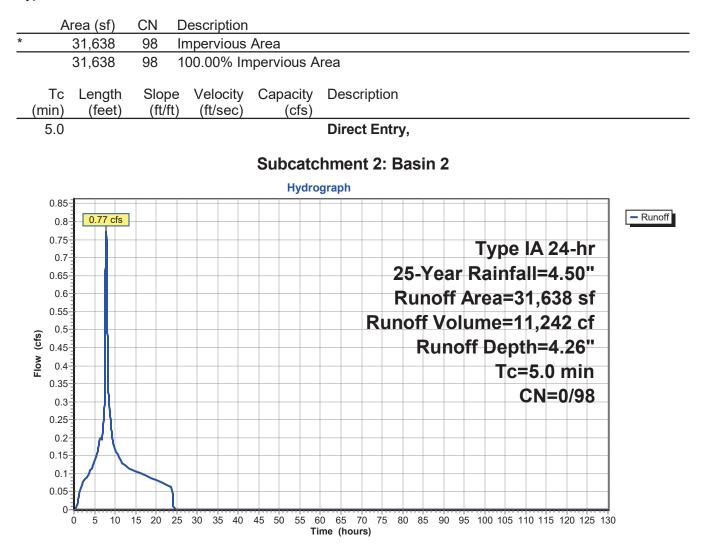
Summary for Subcatchment 1: Basin 1

Runoff = 0.97 cfs @ 7.91 hrs, Volume= 14,452 cf, Depth= 3.30"



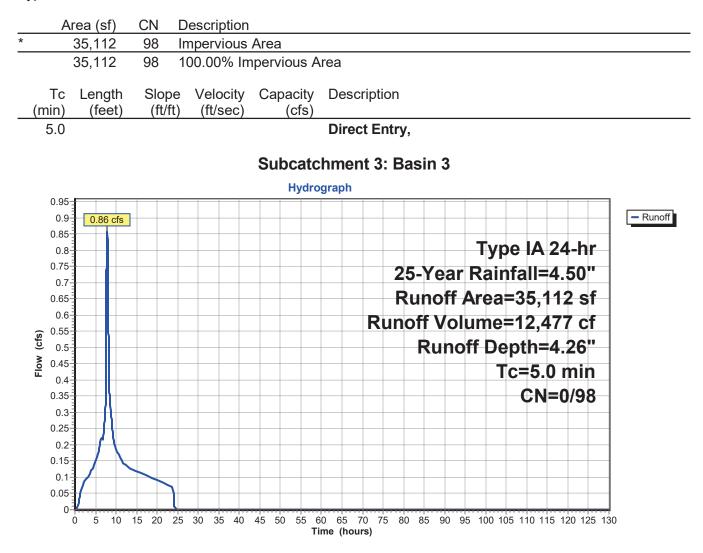
Summary for Subcatchment 2: Basin 2

Runoff = 0.77 cfs @ 7.88 hrs, Volume= 11,242 cf, Depth= 4.26"



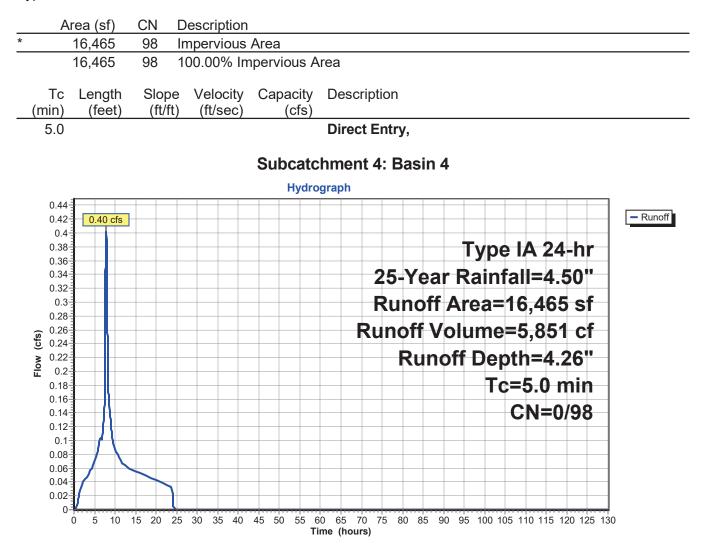
Summary for Subcatchment 3: Basin 3

Runoff = 0.86 cfs @ 7.88 hrs, Volume= 12,477 cf, Depth= 4.26"



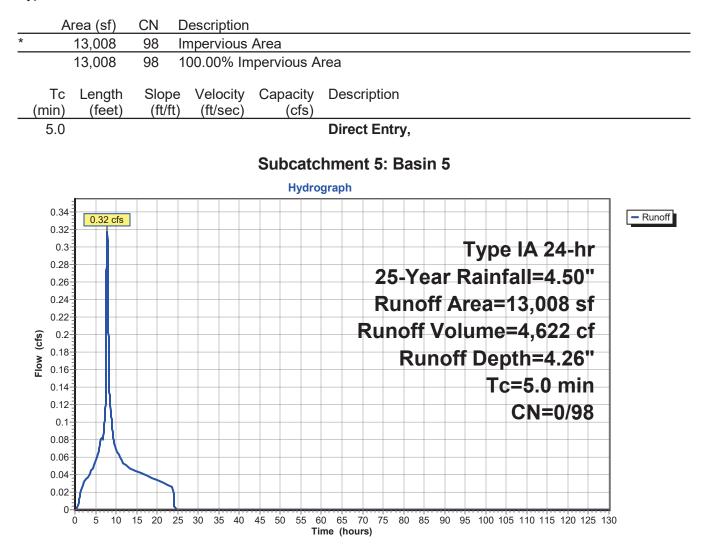
Summary for Subcatchment 4: Basin 4

Runoff = 0.40 cfs @ 7.88 hrs, Volume= 5,851 cf, Depth= 4.26"



Summary for Subcatchment 5: Basin 5

Runoff = 0.32 cfs @ 7.88 hrs, Volume= 4,622 cf, Depth= 4.26"



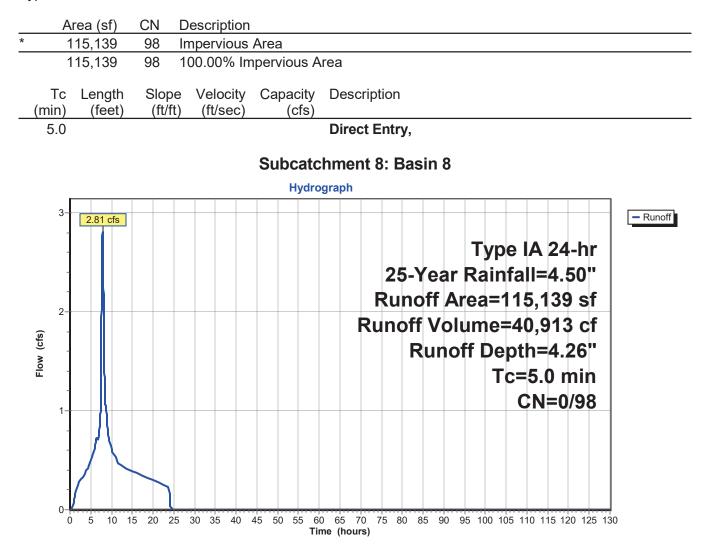
Summary for Subcatchment 6: Basin 6

Runoff = 2.43 cfs @ 7.94 hrs, Volume= 36,491 cf, Depth= 2.75"

Area (sf) CN Description	
* 133,479 80 Pervious	
<u>* 25,640 98 Impervious</u> 159,119 83 Weighted Average	
133,479 80 83.89% Pervious Area	
25,640 98 16.11% Impervious Area	
Tc Length Slope Velocity Capacity Description	
(min) (feet) (ft/ft) (ft/sec) (cfs)	
5.0 Direct Entry,	
Subcatchment 6: Basin 6	
Hydrograph	
]
2.43 cfs	- Runoff
Type IA 24-hr	
25-Year Rainfall=4.50"	
Runoff Area=159,119 sf	
Runoff Volume=36,491 cf	
Runoff Depth=2.75"	
Tc=5.0 min	
¹ CN=80/98	
0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 1 Time (hours)	30

Summary for Subcatchment 8: Basin 8

Runoff = 2.81 cfs @ 7.88 hrs, Volume= 40,913 cf, Depth= 4.26"



Summary for Subcatchment 9: Basin 9

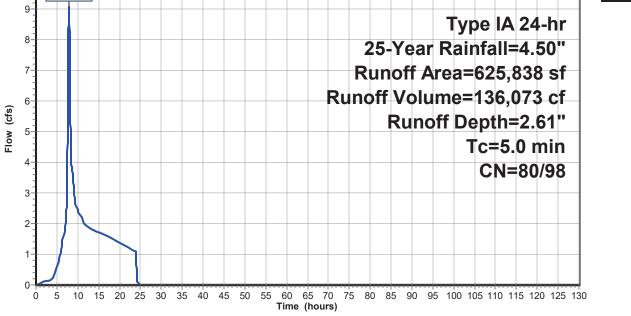
Runoff = 2.91 cfs @ 7.93 hrs, Volume= 43,484 cf, Depth= 2.91"

	Α	rea (sf)	CN [Description						
*		44,929		mpervious						
×		<u>34,155</u> 79,084		<u>andscape.</u> Veighted A						
		34,155			vious Area	l				
		44,929		25.09% Imp	pervious Ar	ea				
	Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Becomption				
	5.0					Direct Entry	у,			
					Subcato	hment 9: B	lasin 9			
						ograph	Jasin J			
	ſ				Tiyare	угарп				
	3–	2.91 cfs								- Runoff
	1							Type IA 24	1-hr	
	1					25		ainfall=4.		
]							a=179,08		
	2-									
(ofe						Rund		me=43,48		
Elour (ofe)	Å I						Runott	Depth=2.		
ū								Tc=5.0	min	
	1							CN=80)/98	
	ľ	Ν								
	-									
	-									
	- 1									
	0- <mark>+</mark> 0	5 10 1	5 20 25	30 35 40			0 85 90 95	100 105 110 115 1	20 125 13	0
					11	me (hours)				

Summary for Subcatchment 10: Basin 10

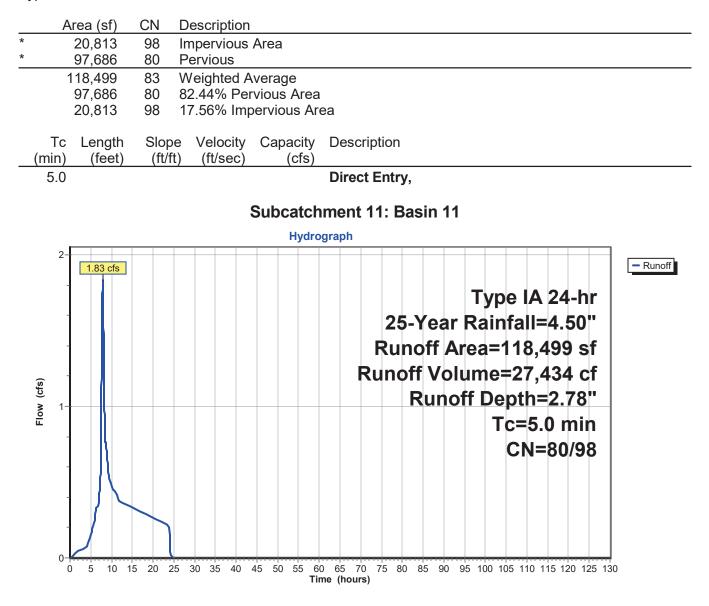
Runoff = 9.05 cfs @ 7.95 hrs, Volume= 136,073 cf, Depth= 2.61"

	Area (sf)	CN	Description	
*	51,234	98	Impervious Area	
*	574,604	80	Pervious	
	625,838	81	Weighted Average	
	574,604	80	91.81% Pervious Area	
	51,234	98	8.19% Impervious Area	
	Tc Length (min) (feet) 5.0	Slop (ft/i		
			Subcatchment 10: Basin 10	
	10-		Hydrograph	
	9.05 cfs			- Runoff
	9			4



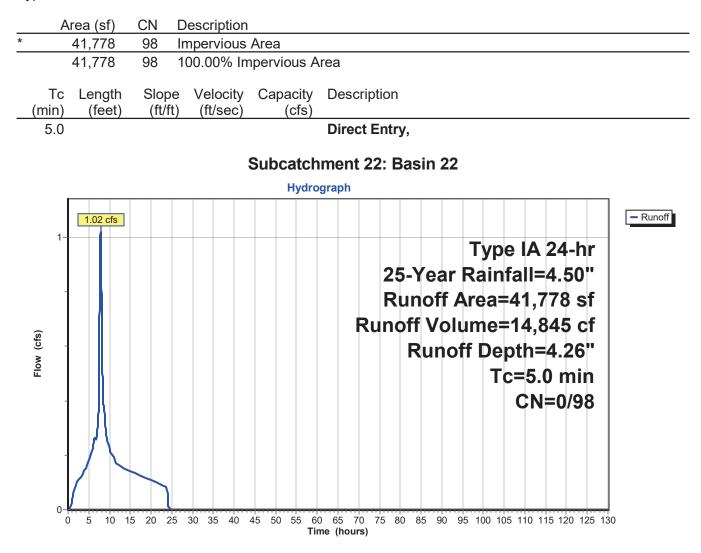
Summary for Subcatchment 11: Basin 11

Runoff = 1.83 cfs @ 7.94 hrs, Volume= 27,434 cf, Depth= 2.78"



Summary for Subcatchment 22: Basin 22

Runoff = 1.02 cfs @ 7.88 hrs, Volume= 14,845 cf, Depth= 4.26"



0.5°

0.4

0.35-0.25-0.25-0.15-0.15-0.05-0-0-

0

Tc=5.0 min

CN=80/98

Summary for Subcatchment 23: Basin 23

Runoff = 0.88 cfs @ 7.89 hrs, Volume= 13,014 cf, Depth= 3.76"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

_	A	rea (sf)	CN	Description						
*		29,948	98	Impervious Pervious	Area					
_		<u>11,565</u> 41,513	80 93	Weighted A	verage					
		11,565	80	27.86% Per	rvious Area					
		29,948	98	72.14% Imp	pervious Ar	ea				
	Тс	Length	Slope		Capacity	Description				
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
	5.0					Direct Entry	,			
				:	Subcatch	ment 23: Ba	asin 23			
					Hydro	graph				
	0.95-	0.88 cfs								- Runoff
	0.9-									
	0.85- 0.8-							Type IA	24-hr	
	0.8-					2	5-Year F			
	0.7-									
	0.65-						unoff A	1		
	0.6-	3 1 1				Rune	off Volu	me=13,(014 cf	
	(s) 0.55						Runoff	Depth=	=3.76"	
	≥ 0.5									

5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130

Time (hours)

Summary for Reach R4: Ditch 4

 Inflow Area =
 1,080,338 sf, 25.35% Impervious, Inflow Depth > 7.07" for 25-Year event

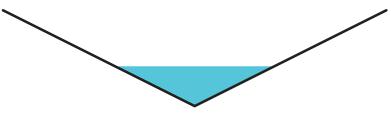
 Inflow =
 5.83 cfs @
 8.89 hrs, Volume=
 636,832 cf

 Outflow =
 5.83 cfs @
 8.89 hrs, Volume=
 636,808 cf, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 5.49 fps, Min. Travel Time= 0.3 min Avg. Velocity = 3.64 fps, Avg. Travel Time= 0.5 min

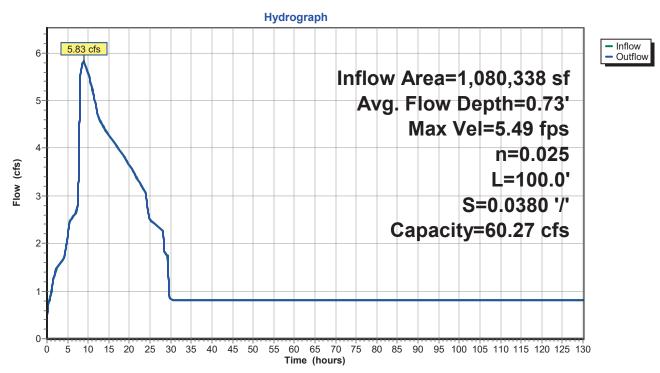
Peak Storage= 106 cf @ 8.89 hrs Average Depth at Peak Storage= 0.73' Bank-Full Depth= 1.75' Flow Area= 6.1 sf, Capacity= 60.27 cfs

Custom cross-section, Length= 100.0' Slope= 0.0380 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 694.00', Outlet Invert= 690.20'



Offset	Elevation	Chan.Depth	
(feet)	(feet)	(feet)	
-3.50	1.75	0.00	
0.00	0.00	1.75	
3.50	1.75	0.00	

Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	0.0	0	0.00
1.75	6.1	7.8	613	60.27



Reach R4: Ditch 4

Summary for Pond Pond A: Pond A

Inflow Area	a =	349,468 sf,	50.41% Impervious,	Inflow Depth = 3.37" for 25-Year event
Inflow	=	6.62 cfs @	7.91 hrs, Volume=	98,149 cf
Outflow	=	2.11 cfs @	9.05 hrs, Volume=	98,149 cf, Atten= 68%, Lag= 68.5 min
Primary	=	2.11 cfs @	9.05 hrs, Volume=	98,149 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 710.32' @ 9.05 hrs Surf.Area= 9,193 sf Storage= 21,619 cf

Plug-Flow detention time= 200.9 min calculated for 98,141 cf (100% of inflow) Center-of-Mass det. time= 200.9 min (904.6 - 703.7)

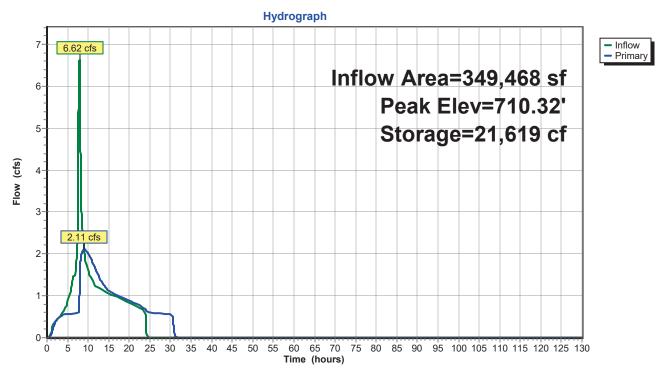
Volume	Inver	t Avail.Sto	rage Storage D	escription	
#1	707.50	' 70,5	55 cf Custom S	tage Data (Prisr	natic) Listed below (Recalc)
_					
Elevatio		Surf.Area	Inc.Store	Cum.Store	
(fee	t)	(sq-ft)	(cubic-feet)	(cubic-feet)	
707.5	0	6,107	0	0	
708.0	0	6,714	3,205	3,205	
709.0	0	7,742	7,228	10,433	
710.0	0	8,825	8,284	18,717	
711.0	0	9,967	9,396	28,113	
712.0	0	11,161	10,564	38,677	
713.0	0	12,412	11,787	50,463	
714.0	0	13,717	13,065	63,528	
714.5	0	14,392	7,027	70,555	
Device	Routing	Invert	Outlet Devices		
#1	Primary	698.43'	18.0" Round C	ulvert	
	,		L= 138.0' CMF	. square edge h	eadwall, Ke= 0.500
					6.24' S= 0.0159 '/' Cc= 0.900
			n= 0.013, Flow		
#2	Device 1	696.43'	,		.620 Limited to weir flow at low heads
#3	Device 1	709.40'		H Vert. Orifice/G	
			@ 9.05 hrs HW=		lischarge)

1=Culvert (Passes 2.11 cfs of 25.08 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.63 cfs @ 17.16 fps)

-3=Orifice/Grate (Orifice Controls 0.03 cfs @ 4.44 fps)

Bull Run Conveyance



Pond Pond A: Pond A

Summary for Pond Pond B: Pond B

Inflow Area	a =	294,223 sf,	54.40% Impervious,	Inflow Depth = 3.44" for 25-Year event
Inflow	=	5.70 cfs @	7.90 hrs, Volume=	84,397 cf
Outflow	=	2.22 cfs @	8.68 hrs, Volume=	84,397 cf, Atten= 61%, Lag= 46.5 min
Primary	=	2.22 cfs @	8.68 hrs, Volume=	84,397 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 710.29' @ 8.68 hrs Surf.Area= 6,322 sf Storage= 15,968 cf

Plug-Flow detention time= 146.4 min calculated for 84,397 cf (100% of inflow) Center-of-Mass det. time= 146.4 min (845.4 - 699.0)

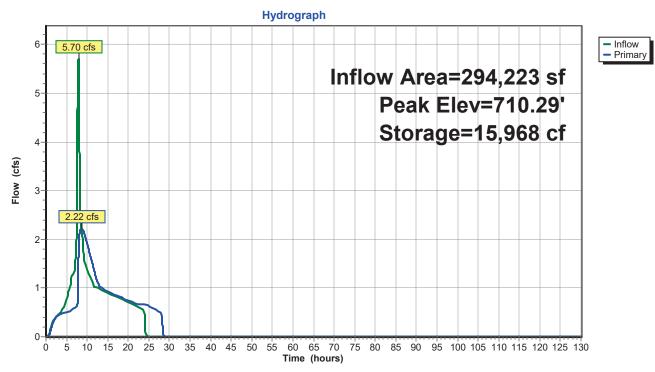
Volume	Inver	t Avail.Sto	rage Storage D	Description	
#1	707.00)' 36,8^	13 cf Custom S	Stage Data (Pri	ismatic) Listed below (Recalc)
-					
Elevatio		Surf.Area	Inc.Store	Cum.Store	
(feet	t)	(sq-ft)	(cubic-feet)	(cubic-feet)	
707.0	0	3,493	0	0	
708.0	0	4,288	3,891	3,891	
709.0	0	5,139	4,714	8,604	
710.0	0	6,046	5,593	14,197	
711.0	0	7,010	6,528	20,725	
712.0	0	8,030	7,520	28,245	
713.0	0	9,107	8,569	36,813	
Device	Routing	Invert	Outlet Devices		
#1	Primary	705.04'	18.0" Round C	Culvert	
	-		L= 339.0' CMF	P, square edge	headwall, Ke= 0.500
					701.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow		
#2	Device 1	703.04'	,		= 0.620 Limited to weir flow at low heads
#3	Device 1	709.15'	14.0" W x 3.0"		

Primary OutFlow Max=2.22 cfs @ 8.68 hrs HW=710.29' (Free Discharge)

-**1=Culvert** (Passes 2.22 cfs of 13.65 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.76 cfs @ 11.40 fps)

-3=Orifice/Grate (Orifice Controls 1.46 cfs @ 5.00 fps)



Pond Pond B: Pond B

Summary for Pond Pond C: Pond C

Inflow Are	a =	744,337 sf,	9.68% Impervious,	Inflow Depth > 8.67" for 25-Year event
Inflow	=	11.68 cfs @	7.95 hrs, Volume=	537,935 cf, Incl. 0.80 cfs Base Flow
Outflow	=	3.45 cfs @	10.90 hrs, Volume=	537,590 cf, Atten= 70%, Lag= 177.5 min
Primary	=	3.45 cfs @	10.90 hrs, Volume=	537,590 cf

Routing by Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 705.76' @ 10.90 hrs Surf.Area= 22,994 sf Storage= 36,249 cf

Plug-Flow detention time= 64.3 min calculated for 537,586 cf (100% of inflow) Center-of-Mass det. time= 61.1 min (3,008.5 - 2,947.4)

Volume	Inver	t Avail.Sto	rage Storage	Description	
#1	704.00)' 95,47	16 cf Custom	Stage Data (Pr	ismatic) Listed below (Recalc)
F lavestia				Ourse Otherse	
Elevatio		Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
704.0	00	18,277	0	0	
705.0	00	20,828	19,553	19,553	
706.0	00	23,671	22,250	41,802	
707.0	00	26,743	25,207	67,009	
708.0	00	30,070	28,407	95,416	
Device	Routing	Invert	Outlet Device:	S	
#1	Primary	697.90'	12.0" Round	Culvert	
	,		L= 53.0' CM	⊃. square edge	headwall, Ke= 0.500
					695.55' S= 0.0443 '/' Cc= 0.900
				w Area= 0.79 st	
#2	Device 1	693.90'	,		= 0.620 Limited to weir flow at low heads
#3	Device 1	704.85'			Grate C= 0.620

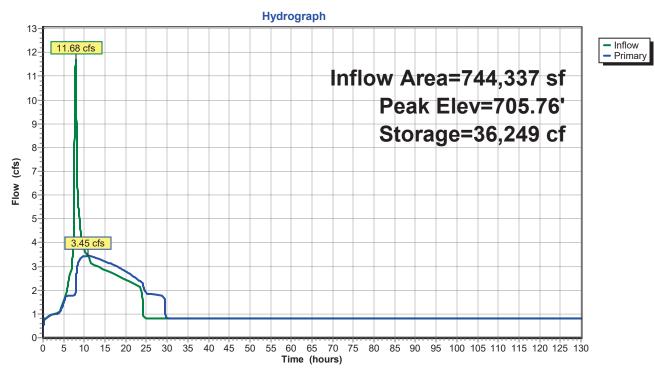
Primary OutFlow Max=3.45 cfs @ 10.90 hrs HW=705.76' (Free Discharge)

-**1=Culvert** (Passes 3.45 cfs of 10.26 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.98 cfs @ 13.95 fps)

-3=Orifice/Grate (Orifice Controls 1.47 cfs @ 4.41 fps)

Bull Run Conveyance

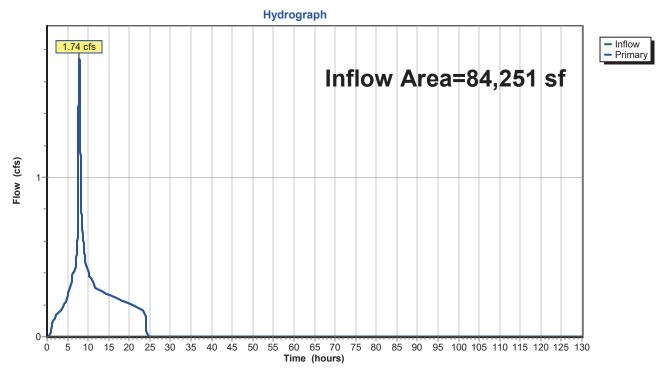


Pond Pond C: Pond C

Summary for Link L1: Pipe 4

Inflow Area	a =	84,251 sf,	66.47% Impervious,	Inflow Depth = 3.66"	for 25-Year event
Inflow	=	1.74 cfs @	7.89 hrs, Volume=	25,694 cf	
Primary	=	1.74 cfs @	7.89 hrs, Volume=	25,694 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs

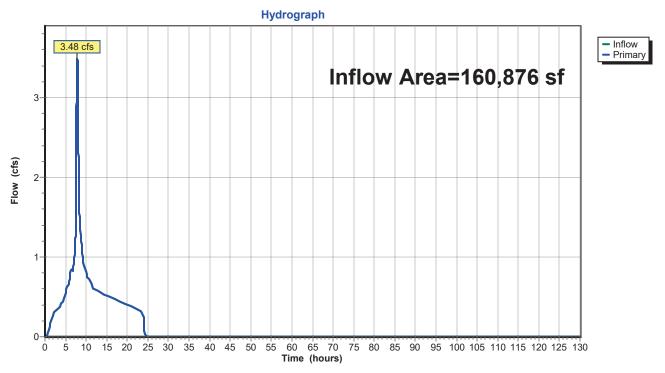


Link L1: Pipe 4

Summary for Link L2: Pipe 6

Inflow Are	a =	160,876 sf,	75.25% Impervious,	Inflow Depth = 3.82 "	for 25-Year event
Inflow	=	3.48 cfs @	7.89 hrs, Volume=	51,185 cf	
Primary	=	3.48 cfs @	7.89 hrs, Volume=	51,185 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs

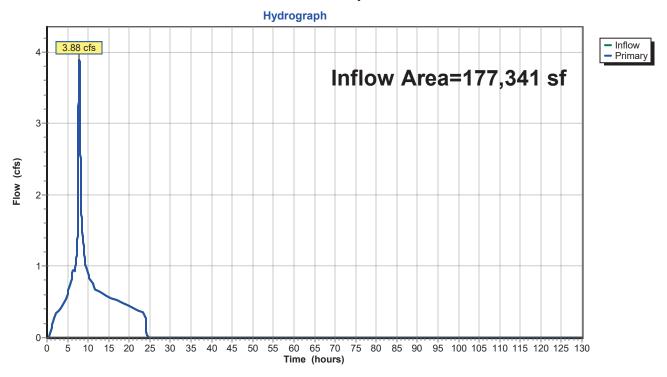


Link L2: Pipe 6

Summary for Link L3: Pipe 7

Inflow Area	a =	177,341 sf,	77.55% Impervious,	Inflow Depth = $3.86"$	for 25-Year event
Inflow	=	3.88 cfs @	7.89 hrs, Volume=	57,035 cf	
Primary	=	3.88 cfs @	7.89 hrs, Volume=	57,035 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs

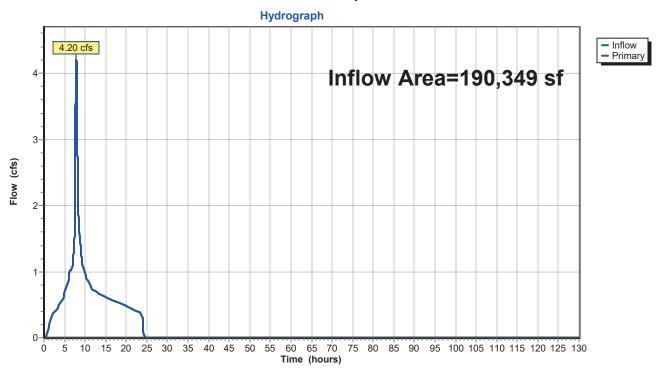


Link L3: Pipe 7

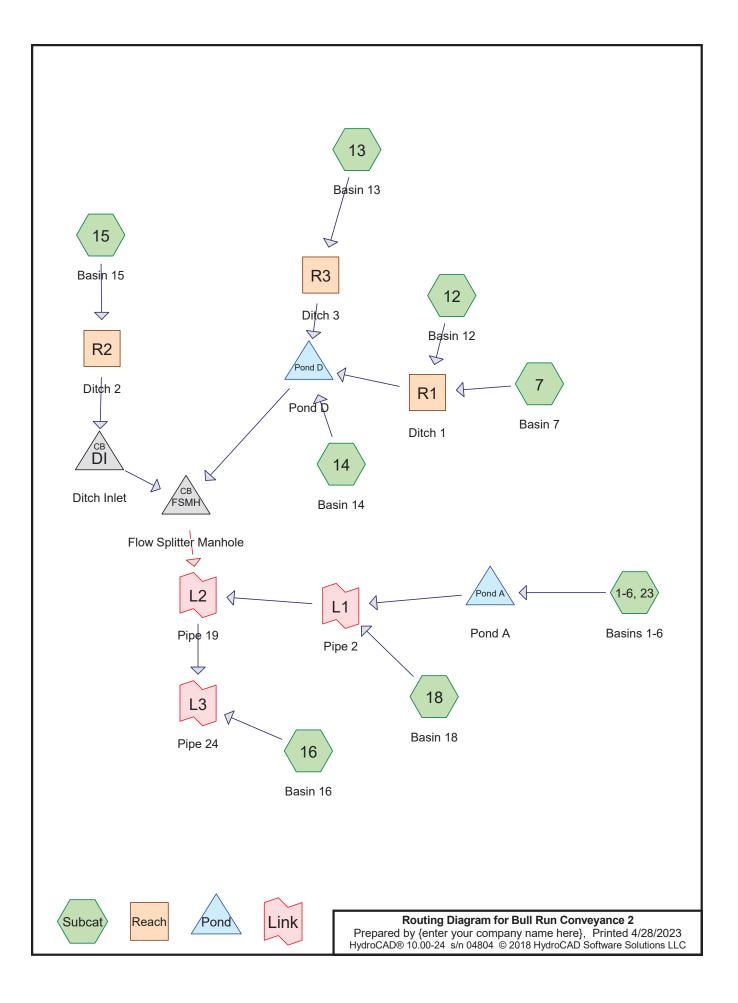
Summary for Link L4: Pipe 9

Inflow Area	a =	190,349 sf,	79.08% Impervious,	Inflow Depth = 3.89"	for 25-Year event
Inflow	=	4.20 cfs @	7.89 hrs, Volume=	61,657 cf	
Primary	=	4.20 cfs @	7.89 hrs, Volume=	61,657 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs



Link L4: Pipe 9



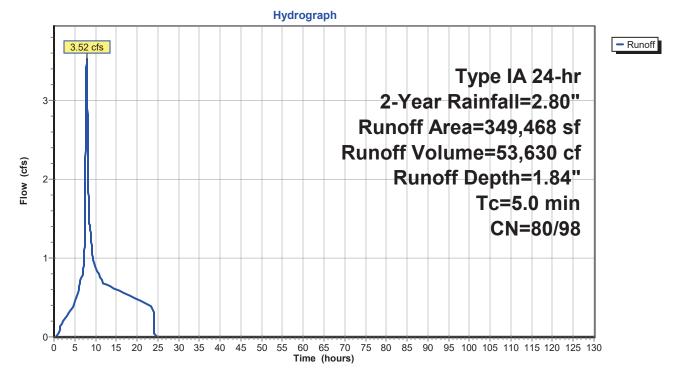
Summary for Subcatchment 1-6, 23: Basins 1-6

Runoff = 3.52 cfs @ 7.92 hrs, Volume= 53,630 cf, Depth= 1.84"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

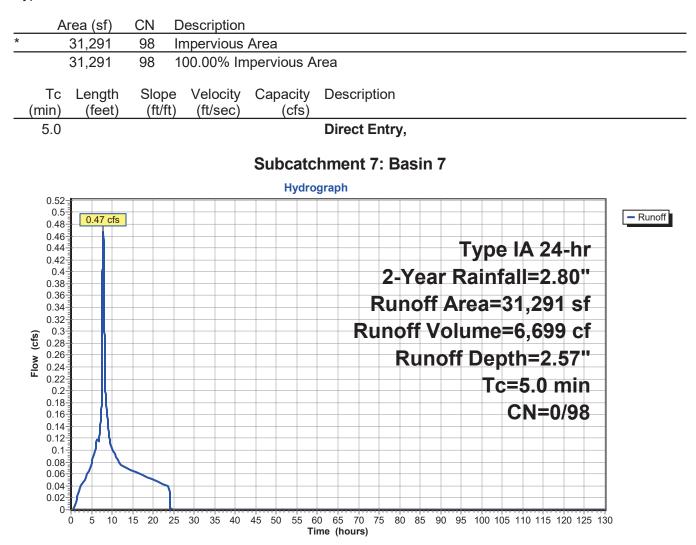
_	A	rea (sf)	CN	Description					
*	1	76,142	98	Impervious Area					
_	1	73,326	80	>75% Gras	s cover, Go	ood, HSG D			
	3	49,468	89	89 Weighted Average					
	1	73,326	326 80 49.60% Pervious Area			3			
	1	76,142	98	50.40% Imp	pervious Ar	ea			
_	Tc (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description			
	5.0					Direct Entry,			

Subcatchment 1-6, 23: Basins 1-6



Summary for Subcatchment 7: Basin 7

Runoff = 0.47 cfs @ 7.88 hrs, Volume= 6,699 cf, Depth= 2.57"



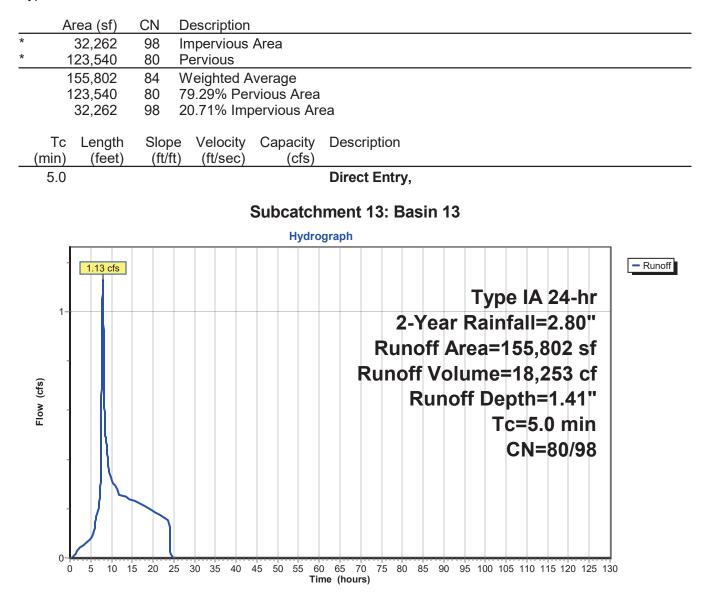
Summary for Subcatchment 12: Basin 12

Runoff = 0.54 cfs @ 7.94 hrs, Volume= 8,458 cf, Depth= 1.56"

А	rea (sf)	CN I	Description								
*	20,370	98	mpervious	Area							
*	44,610		Pervious								
	64,980		Weighted A								
	44,610		68.65% Per								
	20,370	98 3	31.35% Imp	bervious Ar	ea						
Тс	Length	Slope		Capacity	Descriptio	n					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
5.0					Direct Ent	ry,					
			:	Subcatch	ment 12:	Basin 1	2				
0.6	_			Hydro	ograph						
0.6	0.54 cfs										- Runoff
0.55											
0.5							Ту	pe l/	4 24-ł	1 r	
0.45						2-Year	Ra	infal	I=2.80) • •	
0.4						unoff	_	_			
	-								•		
0.35 ^{0.35}					Rur	off Vo	olun	ne=8	,458 (CT	
Elow (cfs)						Runo	off D	epth	n=1.56	5"	
œ 0.25								Tc=	5.0 mi	n	
0.2									=80/9		
0.15									-00/3	0	
0.1											
0.05											
0	0 5 10		5 30 35 40		60 65 70 75				0 445 400		,
	0 5 10	10 20 20	5 50 55 40		ime (hours)	00 00 90	90 10	00 100 11	0 113 120	120 130	1

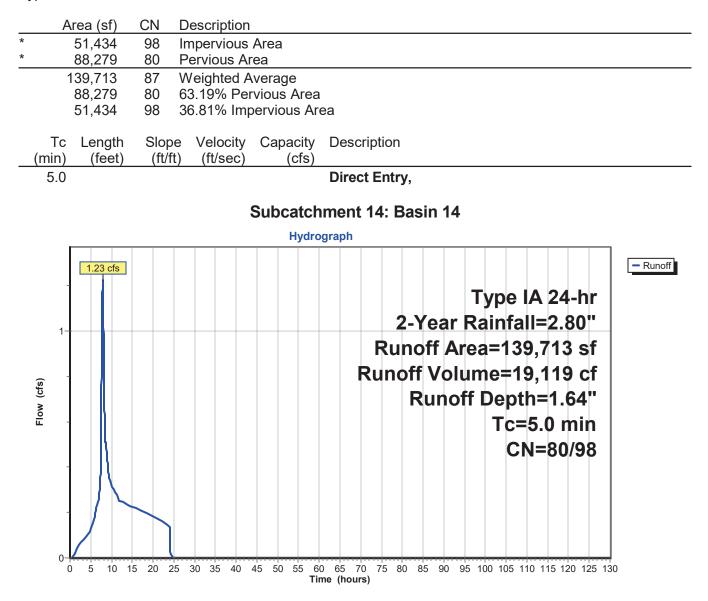
Summary for Subcatchment 13: Basin 13

Runoff = 1.13 cfs @ 7.96 hrs, Volume= 18,253 cf, Depth= 1.41"



Summary for Subcatchment 14: Basin 14

Runoff = 1.23 cfs @ 7.94 hrs, Volume= 19,119 cf, Depth= 1.64"



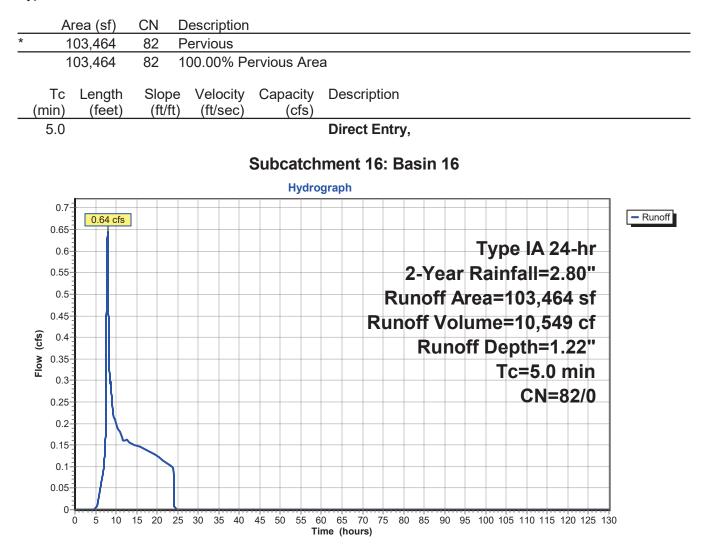
Summary for Subcatchment 15: Basin 15

Runoff = 0.59 cfs @ 7.99 hrs, Volume= 9,892 cf, Depth= 1.18"

 * 5,624 98 Impervious Area * 94,598 80 Pervious 100,222 81 Weighted Average 94,598 80 94.39% Pervious Area 5,624 98 5.61% Impervious Area Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs) 5.0 Direct Entry, 	
100,22281Weighted Average94,5988094.39% Pervious Area5,624985.61% Impervious AreaTcLengthSlopeVelocity(min)(feet)(ft/ft)(ft/sec)(cfs)	
94,598 80 94.39% Pervious Area 5,624 98 5.61% Impervious Area Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
5,624 98 5.61% Impervious Area Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
(min) (feet) (ft/ft) (ft/sec) (cfs)	
5.0 Direct Entry.	
Subcatchment 15: Basin 15	
Hydrograph	
	Runoff
	tunon
0.55 Type IA 24-hr	
0.5 2-Year Rainfall=2.80''	
^{0.45} Runoff Area=100,222 sf	
رق 0.35 ق 0.35	
Tc=5.0 min	
0.2 CN=80/98	
0.1	
0.05	
0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 Time (hours)	

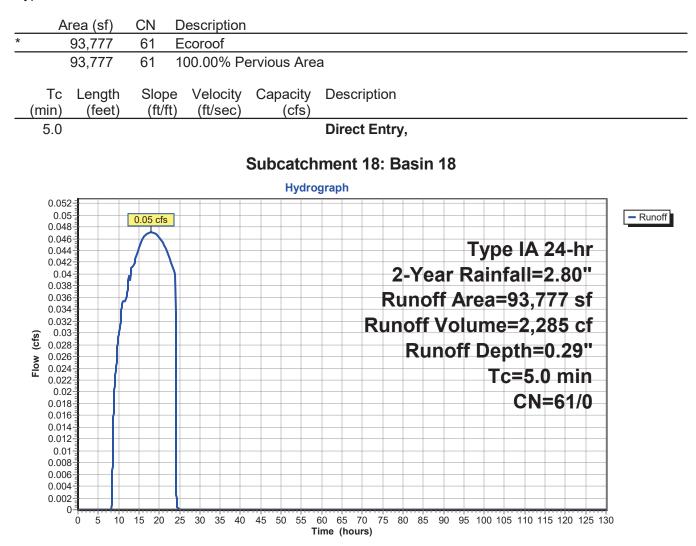
Summary for Subcatchment 16: Basin 16

Runoff = 0.64 cfs @ 7.99 hrs, Volume= 10,549 cf, Depth= 1.22"



Summary for Subcatchment 18: Basin 18

Runoff = 0.05 cfs @ 18.05 hrs, Volume= 2,285 cf, Depth= 0.29"



Summary for Reach R1: Ditch 1

15.74

561

96,271 sf, 53.66% Impervious, Inflow Depth = 1.89" Inflow Area = for 2-Year event Inflow = 1.00 cfs @ 7.91 hrs, Volume= 15.157 cf 7.93 hrs, Volume= Outflow 1.00 cfs @ 15,157 cf, Atten= 0%, Lag= 0.9 min =

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 2.23 fps, Min. Travel Time= 1.3 min Avg. Velocity = 1.15 fps, Avg. Travel Time= 2.6 min

Peak Storage= 79 cf @ 7.93 hrs Average Depth at Peak Storage= 0.18' Bank-Full Depth= 0.75' Flow Area= 3.2 sf, Capacity= 15.74 cfs

Custom cross-section, Length= 176.0' Slope= 0.0187 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 702.30', Outlet Invert= 699.00'

‡

0.75

	Offse	et Eleva	tion	Cha	n.Depth		
_	(feet	t) (f	eet)		(feet)		
	-3.2	5 ().75		0.00		
	-1.0	0 (00.0		0.75		
	1.0	0 0	0.00		0.75		
	3.2	5 ().75		0.00		
	Depth E	nd Area	Pe	rim.	S	Storage	Discharge
_	(feet)	(sq-ft)	(f	eet)	(cub	ic-feet)	(cfs)
	0.00	0.0		2.0		0	0.00

6.7

3.2

Hydrograph Inflow 1.00 cfs Outflow Inflow Area=96,271 sf Avg. Flow Depth=0.18' Max Vel=2.23 fps n=0.025 Flow (cfs) L=176.0' S=0.0187 '/' Capacity=15.74 cfs 0 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 5 Ó Time (hours)

Reach R1: Ditch 1

Summary for Reach R2: Ditch 2

Inflow Area =100,222 sf,5.61% Impervious, Inflow Depth =1.18" for 2-Year eventInflow =0.59 cfs @7.99 hrs, Volume=9,892 cfOutflow =0.58 cfs @8.01 hrs, Volume=9,892 cf, Atten= 2%, Lag= 0.9 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 1.77 fps, Min. Travel Time= 5.4 min Avg. Velocity = 0.86 fps, Avg. Travel Time= 11.1 min

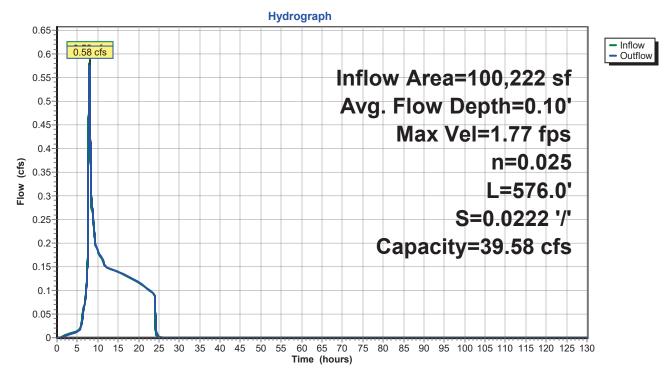
Peak Storage= 187 cf @ 8.01 hrs Average Depth at Peak Storage= 0.10' Bank-Full Depth= 1.00' Flow Area= 6.0 sf, Capacity= 39.58 cfs

Custom cross-section, Length= 576.0' Slope= 0.0222 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 703.59', Outlet Invert= 690.82'

‡

 Offset (feet)	Elevation (feet)	Chan.Depth (feet)	
 -4.50	1.00	0.00	
-1.50	0.00	1.00	
1.50	0.00	1.00	
4.50	1.00	0.00	
epth End		erim. Sto	ora

Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	3.0	0	0.00
1.00	6.0	9.3	3,456	39.58



Reach R2: Ditch 2

Summary for Reach R3: Ditch 3

 Inflow Area =
 155,802 sf, 20.71% Impervious, Inflow Depth =
 1.41" for 2-Year event

 Inflow =
 1.13 cfs @
 7.96 hrs, Volume=
 18,253 cf

 Outflow =
 1.12 cfs @
 8.00 hrs, Volume=
 18,253 cf, Atten=

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 1.70 fps, Min. Travel Time= 5.2 min Avg. Velocity = 0.84 fps, Avg. Travel Time= 10.5 min

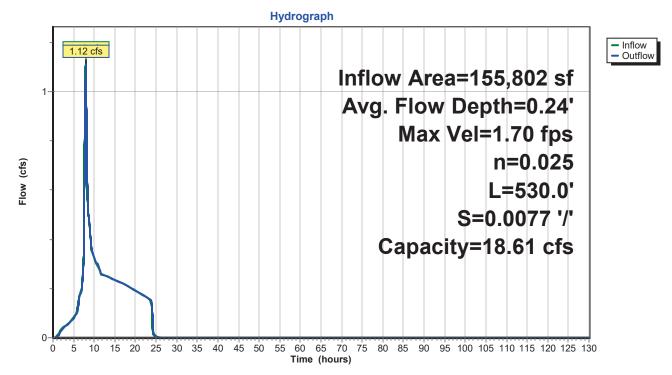
Peak Storage= 347 cf @ 8.00 hrs Average Depth at Peak Storage= 0.24' Bank-Full Depth= 1.00' Flow Area= 5.0 sf, Capacity= 18.61 cfs

Custom cross-section, Length= 530.0' Slope= 0.0077 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 707.64', Outlet Invert= 703.54'

‡

	Offset Ele (feet)	evation (feet)	Chan.Depth (feet)	
-	-4.00	1.00	0.00	
	-1.00	0.00	1.00	
	1.00	0.00	1.00	
	4.00	1.00	0.00	
	Denth End Are		rim St	~ *

Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	2.0	0	0.00
	5.0	8.3	2,650	18.61



Reach R3: Ditch 3

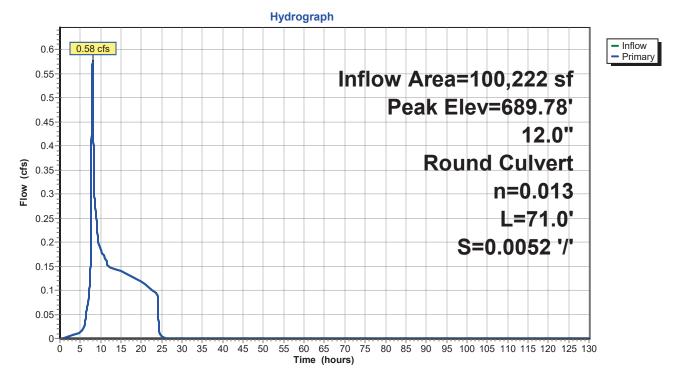
Summary for Pond DI: Ditch Inlet

Inflow Area	a =	100,222 sf,	5.61% Impervious,	Inflow Depth = 1.18" for 2-Year event
Inflow	=	0.58 cfs @	8.01 hrs, Volume=	9,892 cf
Outflow	=	0.58 cfs @	8.01 hrs, Volume=	9,892 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.58 cfs @	8.01 hrs, Volume=	9,892 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 689.78' @ 8.01 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	689.33'	12.0" Round From Ditch Inlet L= 71.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 689.33' / 688.96' S= 0.0052 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.58 cfs @ 8.01 hrs HW=689.78' TW=689.34' (Dynamic Tailwater) **1=From Ditch Inlet** (Outlet Controls 0.58 cfs @ 2.45 fps)



Pond DI: Ditch Inlet

Summary for Pond FSMH: Flow Splitter Manhole

Inflow Area =	492,008 sf,	28.65% Impervious,	Inflow Depth = 1.52" for 2-Year event
Inflow =	1.37 cfs @	8.03 hrs, Volume=	62,427 cf
Outflow =	1.37 cfs @	8.03 hrs, Volume=	62,427 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.47 cfs @	8.03 hrs, Volume=	22,531 cf
Secondary =	0.90 cfs @	8.03 hrs, Volume=	39,897 cf

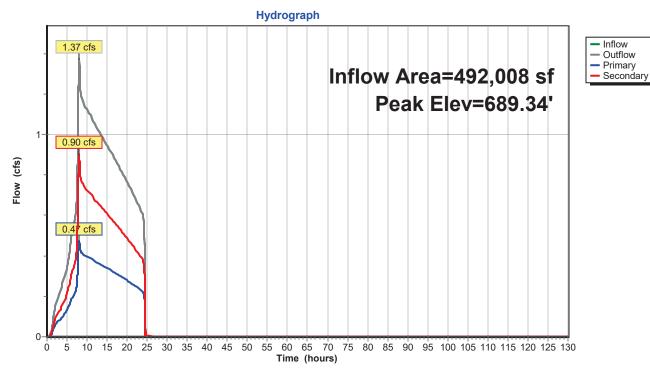
Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 689.34' @ 8.03 hrs

Device	Routing	Invert	Outlet Devices		
#1	Primary	688.86'	8.0" Round To Existing Culvert		
			L= 38.0' RCP, square edge headwall, Ke= 0.500		
			Inlet / Outlet Invert= 688.86' / 688.65' S= 0.0055 '/' Cc= 0.900		
			n= 0.013, Flow Area= 0.35 sf		
#2	Device 1	687.36'	5.0" Horiz. Orifice C= 0.620 Limited to weir flow at low heads		
#3	Secondary	688.86'	18.0" Round Bypassed Flow		
L= 148.0' RCP, square edge headwall, Ke= 0.50			L= 148.0' RCP, square edge headwall, Ke= 0.500		
			Inlet / Outlet Invert= 688.86' / 688.11' S= 0.0051 '/' Cc= 0.900		
			n= 0.013, Flow Area= 1.77 sf		

Primary OutFlow Max=0.47 cfs @ 8.03 hrs HW=689.34' (Free Discharge)

1=To Existing Culvert (Passes 0.47 cfs of 0.48 cfs potential flow) **2=Orifice** (Orifice Controls 0.47 cfs @ 3.44 fps)

Secondary OutFlow Max=0.90 cfs @ 8.03 hrs HW=689.34' TW=0.00' (Dynamic Tailwater) -3=Bypassed Flow (Barrel Controls 0.90 cfs @ 2.77 fps)



Pond FSMH: Flow Splitter Manhole

Summary for Pond Pond A: Pond A

Inflow Area =		349,468 sf, 50.40% Impervious, Inflow Depth = 1.84" for 2-Year event
Inflow	=	3.52 cfs @ 7.92 hrs, Volume= 53,630 cf
Outflow	=	D.61 cfs @ 14.88 hrs, Volume= 53,638 cf, Atten= 83%, Lag= 417.7 min
Primary	=	0.61 cfs @ 14.88 hrs, Volume= 53,638 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 709.31' @ 14.88 hrs Surf.Area= 8,083 sf Storage= 12,925 cf

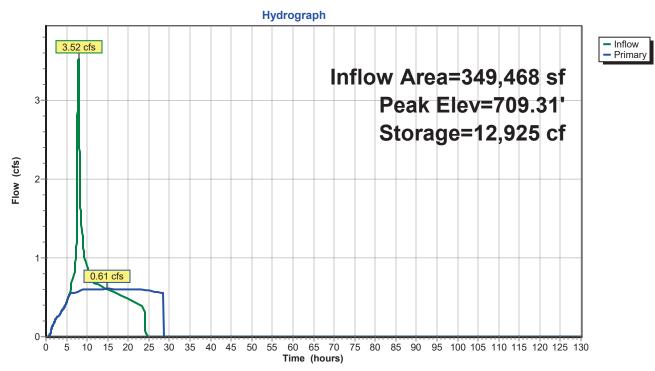
Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 236.5 min (955.2 - 718.7)

Volume	Invei	t Avail.Sto	age Storage Description		
#1	707.50)' 70,55	55 cf Custom	Stage Data (Prisr	natic) Listed below (Recalc)
Elevatio		Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
707.5	50	6,107	0	0	
708.0	00	6,714	3,205	3,205	
709.0	00	7,742	7,228	10,433	
710.0	00	8,825	8,284	18,717	
711.0	00	9,967	9,396	28,113	
712.0	00	11,161	10,564	38,677	
713.0	00	12,412	11,787	50,463	
714.0	00	13,717	13,065	63,528	
714.5	50	14,392	7,027	70,555	
		,	,		
Device	Routing	Invert	Outlet Devices	i	
#1	Primary	698.43'	18.0" Round	Culvert	
L= 138.0' CMP, square edge headwall, Ke= 0.500			eadwall, Ke= 0.500		
					6.24' S= 0.0159 '/' Cc= 0.900
			n= 0.013. Flov	v Area= 1.77 sf	
#2	Device 1	696.43'	,		.620 Limited to weir flow at low heads
#3	Device 1	709.40'		H Vert. Orifice/G	
Primary OutFlow Max=0.61 cfs @ 14.88 hrs HW=709.31' TW=0.00' (Dynamic Tailwater)					

-1=Culvert (Passes 0.61 cfs of 24.05 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.61 cfs @ 16.42 fps)

-3=Orifice/Grate (Controls 0.00 cfs)



Pond Pond A: Pond A

Summary for Pond Pond D: Pond D

Inflow Area	a =	391,786 sf,	34.55% Impervious,	Inflow Depth = 1.61" for 2-Year event	
Inflow	=	3.33 cfs @	7.96 hrs, Volume=	52,530 cf	
Outflow	=	0.93 cfs @	9.82 hrs, Volume=	52,535 cf, Atten= 72%, Lag= 111.3 mi	n
Primary	=	0.93 cfs @	9.82 hrs, Volume=	52,535 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 694.55' @ 9.82 hrs Surf.Area= 5,938 sf Storage= 8,059 cf

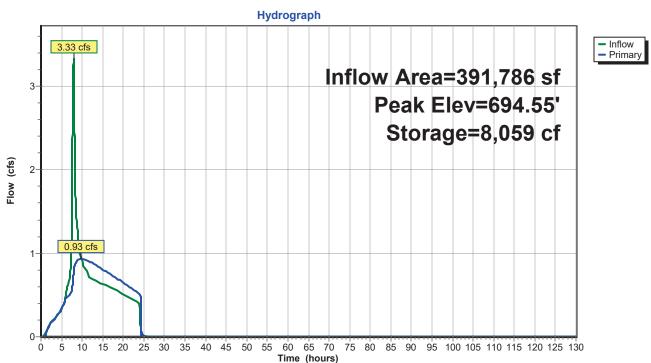
Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 84.5 min (832.1 - 747.6)

Volume	Inve	ert Avail.Sto	rage Storage	Description	
#1	693.0	0' 45,10	06 cf Custom	Stage Data (Pr	rismatic) Listed below (Recalc)
		o ()			
Elevatio		Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
693.0	00	4,453	0	0	
694.0	00	5,388	4,921	4,921	
695.0	00	6,380	5,884	10,805	
696.0	00	7,428	6,904	17,709	
697.0	00	8,533	7,981	25,689	
698.0	00	9,694	9,114	34,803	
699.0	00	10,912	10,303	45,106	
		,	,	,	
Device	Routing	Invert	Outlet Devices	S	
#1	Primary	692.50'	12.0" Round	Culvert	
	5		L= 121.0' CN	IP, square edge	e headwall, Ke= 0.500
				, I O	'688.97' S= 0.0292 '/' Cc= 0.900
			n= 0.013. Flo	w Area= 0.79 st	f
#2	Device 1	690.50'	,		= 0.620 Limited to weir flow at low heads
#3	Device 1	694.60'	15.0" W x 3.0'	H Vert. Orifice	e/Grate C= 0.620
Primary	OutFlow	Max=0.03 cfs (@ 0.82 hrs HW	=604 55' TW=6	689.29' (Dynamic Tailwater)

Primary OutFlow Max=0.93 cfs @ 9.82 hrs HW=694.55' TW=689.29' (Dynamic Tailwater)

-1=Culvert (Passes 0.93 cfs of 4.71 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.93 cfs @ 7.13 fps)



Pond Pond D: Pond D

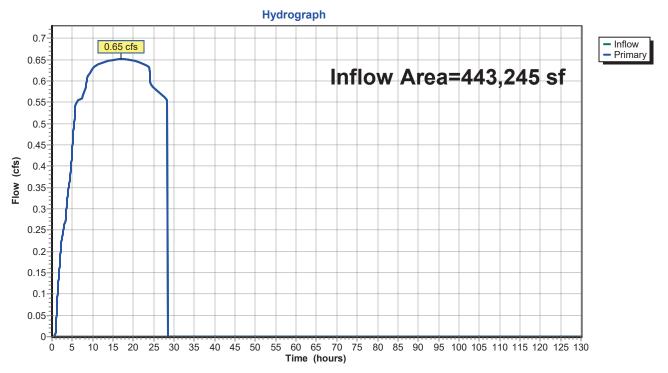
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Type IA 24-hr 2-Year Rainfall=2.80"

Summary for Link L1: Pipe 2

Inflow Are	a =	443,245 sf, 39	9.74% Impervious,	Inflow Depth = 1.51"	for 2-Year event
Inflow	=	0.65 cfs @ 16	6.95 hrs, Volume=	55,923 cf	
Primary	=	0.65 cfs @ 16	6.95 hrs, Volume=	55,923 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs

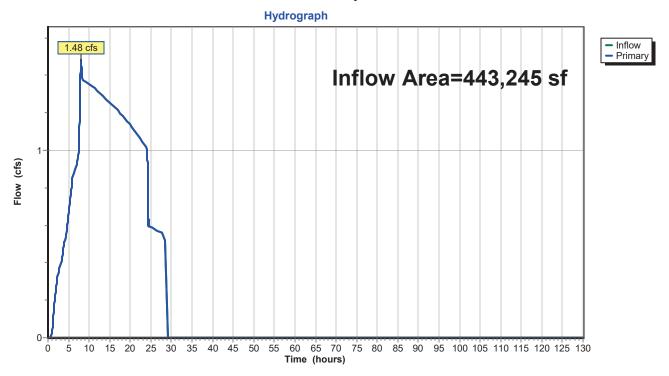


Link L1: Pipe 2

Summary for Link L2: Pipe 19

Inflow Area	a =	443,245 sf,	39.74% Impervious,	Inflow Depth = 2.59"	for 2-Year event
Inflow	=	1.48 cfs @	8.03 hrs, Volume=	95,820 cf	
Primary	=	1.48 cfs @	8.03 hrs, Volume=	95,820 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs

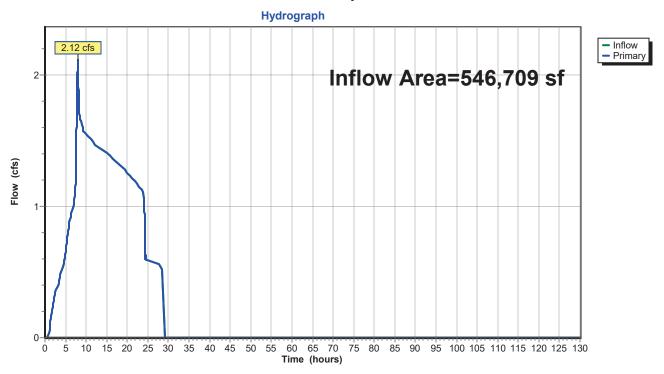


Link L2: Pipe 19

Summary for Link L3: Pipe 24

Inflow Are	a =	546,709 sf,	32.22% Impervious,	Inflow Depth = 2.33"	for 2-Year event
Inflow	=	2.12 cfs @	8.00 hrs, Volume=	106,369 cf	
Primary	=	2.12 cfs @	8.00 hrs, Volume=	106,369 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs



Link L3: Pipe 24

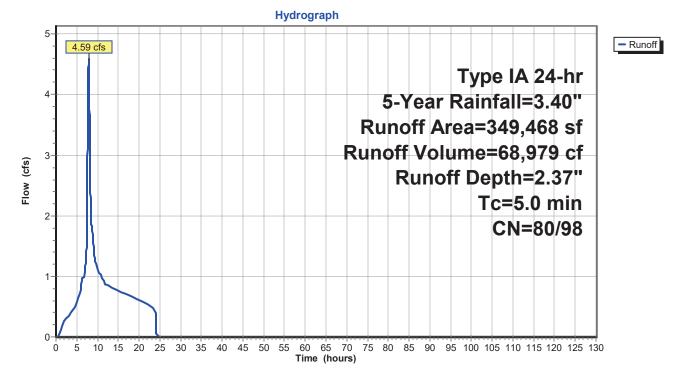
Summary for Subcatchment 1-6, 23: Basins 1-6

Runoff = 4.59 cfs @ 7.91 hrs, Volume= 68,979 cf, Depth= 2.37"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

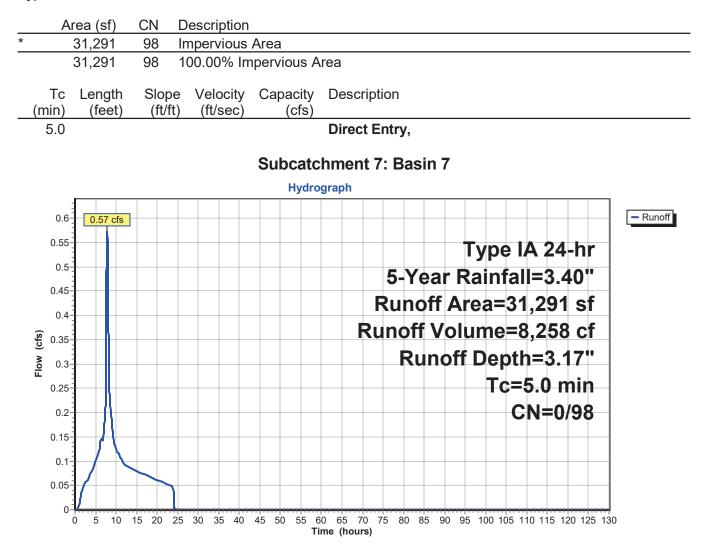
	A	rea (sf)	CN	Description						
*	1	76,142	98	Impervious	Area					
	1	73,326	80	>75% Grass	s cover, Go	bod, HSG D				
	349,468 89 Weighted Average									
173,326 80 49.60% Pervious Area					l					
	176,142 98 50.40% Impervious Are			50.40% Imp	ervious Ar	ea				
	Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description				
	5.0					Direct Entry,				
	Subcatchment 1-6, 23: Basins 1-6									

Subcatchment 1-6, 23: Basins 1-6



Summary for Subcatchment 7: Basin 7

Runoff = 0.57 cfs @ 7.88 hrs, Volume= 8,258 cf, Depth= 3.17"



0.1

0

Summary for Subcatchment 12: Basin 12

Runoff = 0.73 cfs @ 7.93 hrs, Volume= 11,165 cf, Depth= 2.06"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 5-Year Rainfall=3.40"

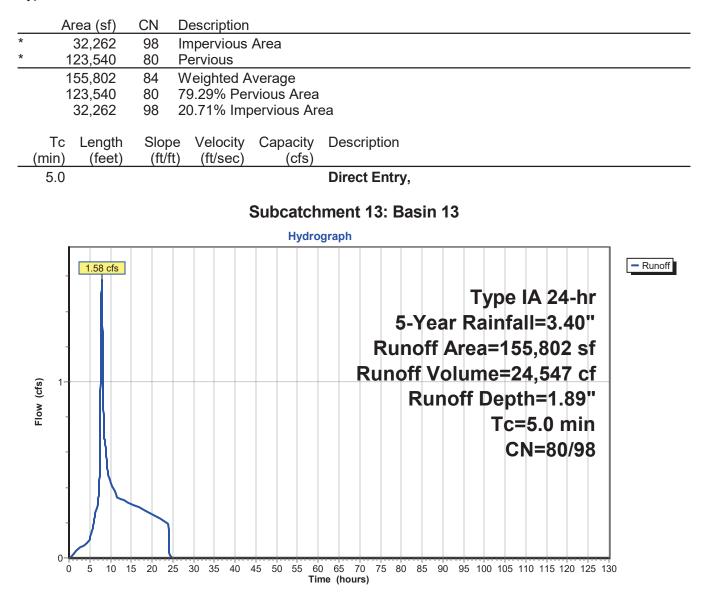
	Area (sf)	CN E	escription						
*	20,370		npervious						
*	44,610		Pervious						
	64,980		Veighted A						
	44,610			rvious Area					
	20,370	98 3	1.35% Imp	pervious Ar	ea				
	Tc Length (min) (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	5.0				Direct Entry,				
				O h a a ta h					
					ment 12: Ba	sin 12			
	0.8	, , , ,		Hydro	graph				
	0.8 0.75 0.73 cfs								- Runoff
	0.7								
	0.65						ype IA 2		
	0.6				5	-Year Ra	infall=3	.40"	
	0.55					inoff Are	a=64.98	30 sf	
	0.5					ff Volum	-		
	(g) 0.45								
	(\$5) 0.45 0.45 0.45 0.45 0.45 0.45 0.45					Runoff [
	e 0.35						Tc=5.0	min	
	0.3						CN=8	0/98	
	0.25								
	0.2								
	0.15								

5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130

Time (hours)

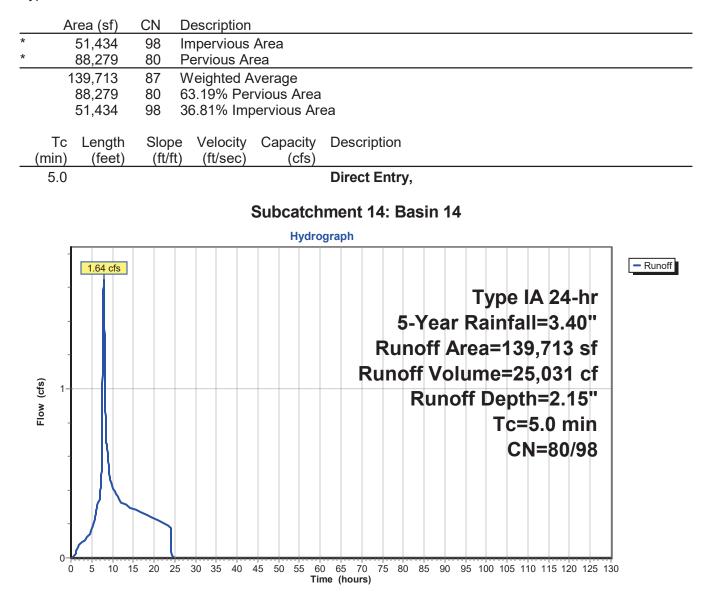
Summary for Subcatchment 13: Basin 13

Runoff = 1.58 cfs @ 7.95 hrs, Volume= 24,547 cf, Depth= 1.89"



Summary for Subcatchment 14: Basin 14

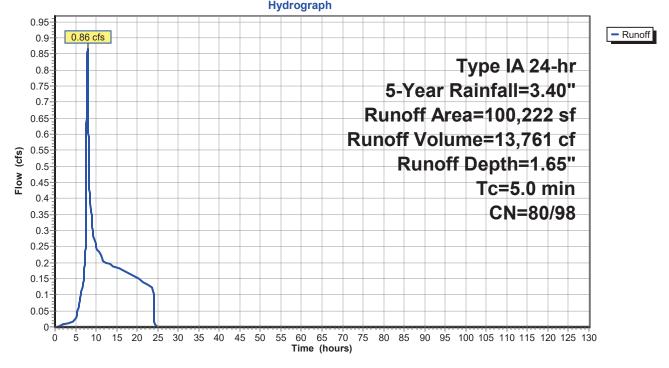
Runoff = 1.64 cfs @ 7.93 hrs, Volume= 25,031 cf, Depth= 2.15"



Summary for Subcatchment 15: Basin 15

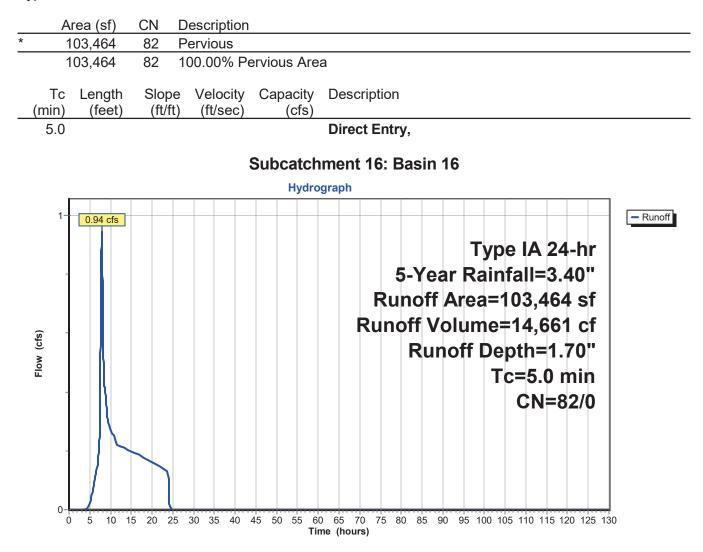
Runoff = 0.86 cfs @ 7.97 hrs, Volume= 13,761 cf, Depth= 1.65"

_	Area (sf)	CN	Description						
*	5,624	98	Impervious	Area					
*	94,598	80	Pervious						
	100,222	81	Weighted A	verage					
	94,598	80	94.39% Per	vious Area					
	5,624	98	5.61% Impe	ervious Area	3				
	Tc Length (min) (feet)	Slop (ft/f	•	Capacity (cfs)	Description				
	5.0				Direct Entry,				
	Subcatchment 15: Basin 15								



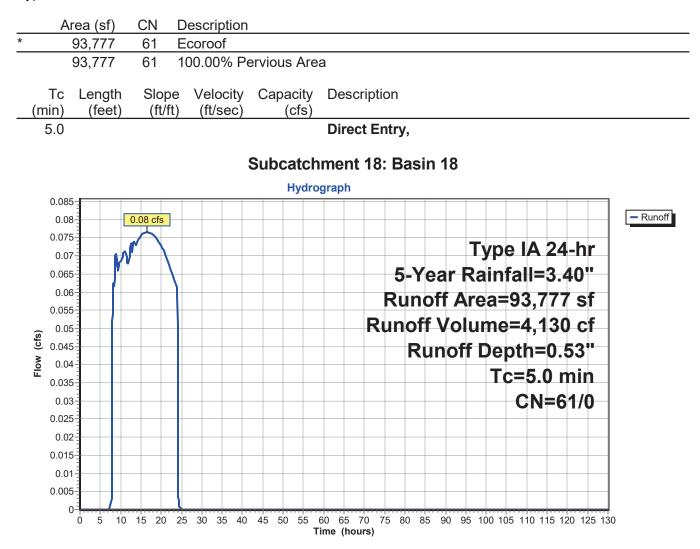
Summary for Subcatchment 16: Basin 16

Runoff = 0.94 cfs @ 7.97 hrs, Volume= 14,661 cf, Depth= 1.70"



Summary for Subcatchment 18: Basin 18

Runoff = 0.08 cfs @ 16.53 hrs, Volume= 4,130 cf, Depth= 0.53"



Summary for Reach R1: Ditch 1

15.74

561

96,271 sf, 53.66% Impervious, Inflow Depth = 2.42" for 5-Year event Inflow Area = Inflow 1.30 cfs @ 7.91 hrs, Volume= 19.423 cf = 7.92 hrs, Volume= Outflow 1.29 cfs @ 19,423 cf, Atten= 0%, Lag= 0.8 min =

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 2.42 fps, Min. Travel Time= 1.2 min Avg. Velocity = 1.25 fps, Avg. Travel Time= 2.3 min

Peak Storage= 94 cf @ 7.92 hrs Average Depth at Peak Storage= 0.20' Bank-Full Depth= 0.75' Flow Area= 3.2 sf, Capacity= 15.74 cfs

Custom cross-section, Length= 176.0' Slope= 0.0187 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 702.30', Outlet Invert= 699.00'

‡

	Offs	et Eleva	ition	Cha	n.Deptl	h		
_	(fee	et) (f	eet)		(feet	<u>;)</u>		
	-3.2	25 ().75		0.0	0		
	-1.0	00	00.0		0.7	5		
	1.0	00	00.0		0.7	5		
	3.2	25).75		0.0	0		
	Depth I	End Area	Pe	erim.		Storage	Disch	arge
_	(feet)	(sq-ft)	(f	eet)	(cı	ubic-feet)		(cfs)
	0.00	0.0		2.0		0		0.00

6.7

3.2

0.75

Hydrograph Inflow 1.29 cfs - Outflow Inflow Area=96,271 sf Avg. Flow Depth=0.20' 1 Max Vel=2.42 fps n=0.025 Flow (cfs) L=176.0' S=0.0187 '/' Capacity=15.74 cfs 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 Ó Time (hours)

Reach R1: Ditch 1

Summary for Reach R2: Ditch 2

 Inflow Area =
 100,222 sf, 5.61% Impervious, Inflow Depth = 1.65" for 5-Year event

 Inflow =
 0.86 cfs @
 7.97 hrs, Volume=
 13,761 cf

 Outflow =
 0.86 cfs @
 8.00 hrs, Volume=
 13,761 cf, Atten= 1%, Lag= 1.8 min

na Diasharra

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 2.04 fps, Min. Travel Time= 4.7 min Avg. Velocity = 0.96 fps, Avg. Travel Time= 10.0 min

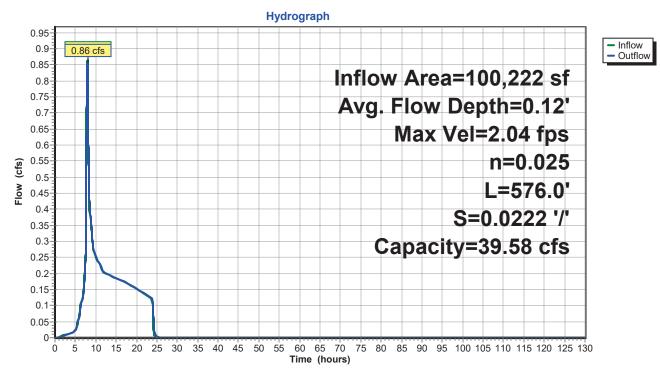
Peak Storage= 242 cf @ 8.00 hrs Average Depth at Peak Storage= 0.12' Bank-Full Depth= 1.00' Flow Area= 6.0 sf, Capacity= 39.58 cfs

Custom cross-section, Length= 576.0' Slope= 0.0222 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 703.59', Outlet Invert= 690.82'

‡

Offset (feet)		Chan.Depth (feet)
-4.50	1.00	0.00
-1.50	0.00	1.00
1.50	0.00	1.00
4.50	1.00	0.00
Depth En		erim. Stora

ige Discharge
et) (cfs)
0 0.00
56 39.58



Reach R2: Ditch 2

Summary for Reach R3: Ditch 3

Discharge

 Inflow Area =
 155,802 sf, 20.71% Impervious, Inflow Depth =
 1.89" for 5-Year event

 Inflow =
 1.58 cfs @
 7.95 hrs, Volume=
 24,547 cf

 Outflow =
 1.56 cfs @
 8.00 hrs, Volume=
 24,547 cf, Atten=

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 1.89 fps, Min. Travel Time= 4.7 min Avg. Velocity = 0.92 fps, Avg. Travel Time= 9.6 min

Peak Storage= 439 cf @ 8.00 hrs Average Depth at Peak Storage= 0.29' Bank-Full Depth= 1.00' Flow Area= 5.0 sf, Capacity= 18.61 cfs

Custom cross-section, Length= 530.0' Slope= 0.0077 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 707.64', Outlet Invert= 703.54'

‡

Offset (feet)		on (et)	Chan.Depth (feet)	
-4.00	1.	00	0.00	
-1.00	0.	00	1.00	
1.00	0.	00	1.00	
4.00	1.	00	0.00	
Depth End	d Area (sɑ-ft)	Peri (fe		Storage

(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	2.0	0	0.00
1.00	5.0	8.3	2,650	18.61

Hydrograph Inflow 1.56 cfs - Outflow Inflow Area=155,802 sf Avg. Flow Depth=0.29' Max Vel=1.89 fps n=0.025 1 Flow (cfs) L=530.0' S=0.0077 '/' Capacity=18.61 cfs 0 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 5 Ó Time (hours)

Reach R3: Ditch 3

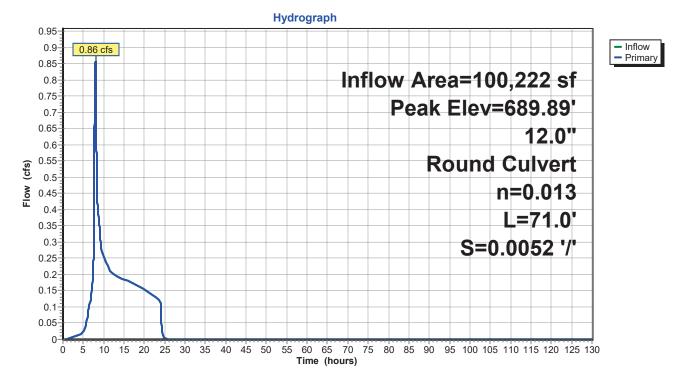
Summary for Pond DI: Ditch Inlet

Inflow Area	a =	100,222 sf,	5.61% Impervious,	Inflow Depth = 1.65" for 5-Year event
Inflow	=	0.86 cfs @	8.00 hrs, Volume=	13,761 cf
Outflow	=	0.86 cfs @	8.00 hrs, Volume=	13,761 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.86 cfs @	8.00 hrs, Volume=	13,761 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 689.89' @ 8.01 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	689.33'	12.0" Round From Ditch Inlet L= 71.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 689.33' / 688.96' S= 0.0052 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.86 cfs @ 8.00 hrs HW=689.89' TW=689.43' (Dynamic Tailwater) T=From Ditch Inlet (Outlet Controls 0.86 cfs @ 2.70 fps)



Pond DI: Ditch Inlet

Summary for Pond FSMH: Flow Splitter Manhole

Inflow Area =	492,008 sf,	28.65% Impervious,	Inflow Depth = 2.02" for 5-Year event
Inflow =	2.11 cfs @	8.73 hrs, Volume=	82,763 cf
Outflow =	2.11 cfs @	8.73 hrs, Volume=	82,763 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.54 cfs @	8.73 hrs, Volume=	28,233 cf
Secondary =	1.57 cfs @	8.73 hrs, Volume=	54,530 cf

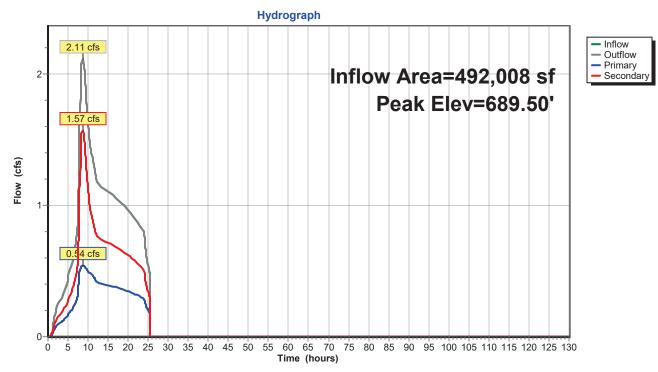
Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 689.50' @ 8.73 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	688.86'	8.0" Round To Existing Culvert
			L= 38.0' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 688.86' / 688.65' S= 0.0055 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.35 sf
#2	Device 1	687.36'	5.0" Horiz. Orifice C= 0.620 Limited to weir flow at low heads
#3	Secondary	688.86'	18.0" Round Bypassed Flow
			L= 148.0' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 688.86' / 688.11' S= 0.0051 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=0.54 cfs @ 8.73 hrs HW=689.50' (Free Discharge)

1=To Existing Culvert (Passes 0.54 cfs of 0.75 cfs potential flow) **2=Orifice** (Orifice Controls 0.54 cfs @ 3.98 fps)

Secondary OutFlow Max=1.57 cfs @ 8.73 hrs HW=689.50' TW=0.00' (Dynamic Tailwater) -3=Bypassed Flow (Barrel Controls 1.57 cfs @ 3.21 fps)



Pond FSMH: Flow Splitter Manhole

Summary for Pond Pond A: Pond A

Inflow Area =	349,468 sf,	50.40% Impervious,	Inflow Depth =	2.37"	for 5-Year event
Inflow =	4.59 cfs @	7.91 hrs, Volume=	68,979 c	f	
Outflow =	1.15 cfs @	9.95 hrs, Volume=	68,985 c	f, Atter	n= 75%, Lag= 122.5 min
Primary =	1.15 cfs @	9.95 hrs, Volume=	68,985 c	f	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 709.64' @ 9.95 hrs Surf.Area= 8,441 sf Storage= 15,652 cf

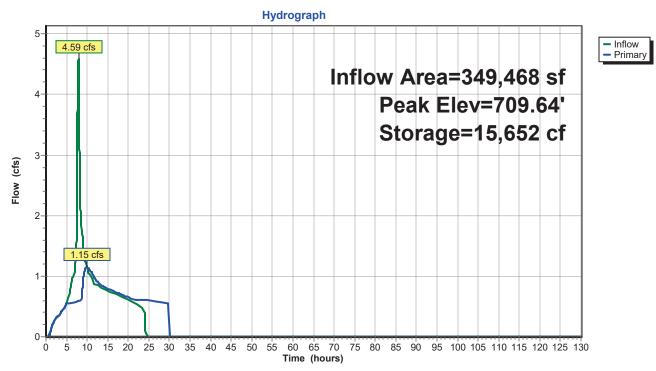
Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 238.9 min (951.6 - 712.7)

Volume	Inver	t Avail.Sto	rage Storage D	escription	
#1	707.50)' 70,55	55 cf Custom S	tage Data (Prisr	matic) Listed below (Recalc)
F lavistic			la e Otene	Ourse Otherse	
Elevatio		Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
707.5	50	6,107	0	0	
708.0	00	6,714	3,205	3,205	
709.0	00	7,742	7,228	10,433	
710.0	00	8,825	8,284	18,717	
711.0	00	9,967	9,396	28,113	
712.0	00	11,161	10,564	38,677	
713.0	00	12,412	11,787	50,463	
714.0	00	13,717	13,065	63,528	
714.5	50	14,392	7,027	70,555	
Device	Routing	Invert	Outlet Devices		
#1	Primary	698.43'	18.0" Round C	ulvert	
	-		L= 138.0' CMF	^o , square edge h	eadwall, Ke= 0.500
			Inlet / Outlet Inv	vert= 698.43' / 69	6.24' S= 0.0159 '/' Cc= 0.900
			n= 0.013, Flow	Area= 1.77 sf	
#2	Device 1	696.43'	,		0.620 Limited to weir flow at low heads
#3	Device 1	709.40'		H Vert. Orifice/G	
			@ 9.95 hrs HW=		0' (Dynamic Tailwater)

-1=Culvert (Passes 1.15 cfs of 24.39 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.61 cfs @ 16.66 fps)

-3=Orifice/Grate (Orifice Controls 0.54 cfs @ 1.64 fps)



Pond Pond A: Pond A

Summary for Pond Pond D: Pond D

Inflow Area	=	391,786 sf,	34.55% Impervious,	Inflow Depth = 2.11" for 5-Year event
Inflow	=	4.48 cfs @	7.96 hrs, Volume=	69,001 cf
Outflow	=	1.74 cfs @	8.84 hrs, Volume=	69,001 cf, Atten= 61%, Lag= 52.9 min
Primary	=	1.74 cfs @	8.84 hrs, Volume=	69,001 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 694.94' @ 8.84 hrs Surf.Area= 6,325 sf Storage= 10,452 cf

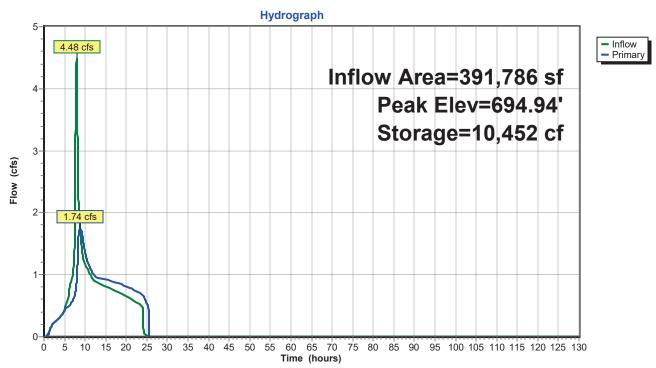
Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 102.2 min (841.7 - 739.5)

Volume	Inve	rt Avail.Sto	rage Storage	Description	
#1	693.0	0' 45,10	06 cf Custom	Stage Data (Pris	matic) Listed below (Recalc)
Elevatio		Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
693.0	00	4,453	0	0	
694.0	00	5,388	4,921	4,921	
695.0	00	6,380	5,884	10,805	
696.0	00	7,428	6,904	17,709	
697.0	00	8,533	7,981	25,689	
698.0	00	9,694	9,114	34,803	
699.0	00	10,912	10,303	45,106	
Device	Routing	Invert	Outlet Devices	S	
#1	Primary	692.50'	12.0" Round	Culvert	
	,		L= 121.0' CN	/IP, square edge	headwall, Ke= 0.500
			Inlet / Outlet Ir	nvert= 692.50' / 6	88.97' S= 0.0292 '/' Cc= 0.900
			n= 0.013, Flo	w Area= 0.79 sf	
#2	Device 1	690.50'	,	rifice/Grate C=	0.620 Limited to weir flow at low heads
#3	Device 1	694.60'	15.0" W x 3.0'	" H Vert. Orifice/	Grate C= 0.620
Primary	OutFlow	Max=1.74 cfs (@ 8.84 hrs HW	'=694.94' TW=68	39.50' (Dynamic Tailwater)

-1=Culvert (Passes 1.74 cfs of 5.27 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.02 cfs @ 7.78 fps)

-3=Orifice/Grate (Orifice Controls 0.72 cfs @ 2.30 fps)

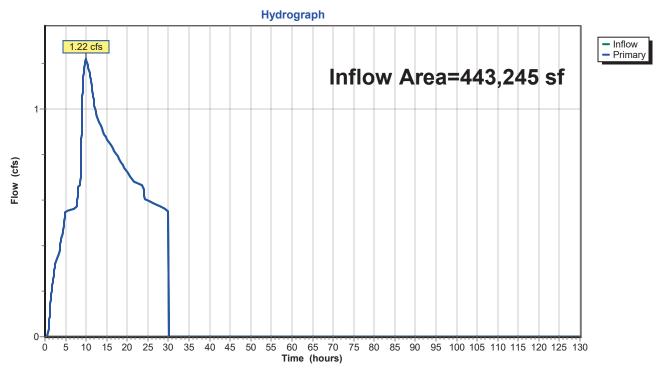


Pond Pond D: Pond D

Summary for Link L1: Pipe 2

Inflow Are	a =	443,245 sf,	39.74% Impervious,	Inflow Depth = 1.98"	for 5-Year event
Inflow	=	1.22 cfs @	9.96 hrs, Volume=	73,115 cf	
Primary	=	1.22 cfs @	9.96 hrs, Volume=	73,115 cf, Atte	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs

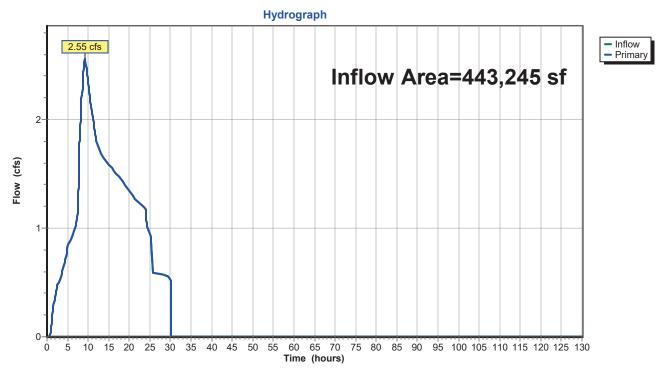


Link L1: Pipe 2

Summary for Link L2: Pipe 19

Inflow Area	a =	443,245 sf,	39.74% Impervious,	Inflow Depth = 3.46"	for 5-Year event
Inflow	=	2.55 cfs @	9.21 hrs, Volume=	127,645 cf	
Primary	=	2.55 cfs @	9.21 hrs, Volume=	127,645 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs

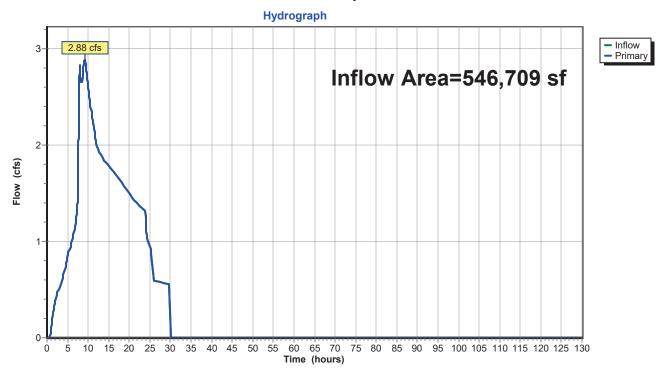


Link L2: Pipe 19

Summary for Link L3: Pipe 24

Inflow Are	a =	546,709 sf,	32.22% Impervious,	Inflow Depth = 3.12"	for 5-Year event
Inflow	=	2.88 cfs @	9.11 hrs, Volume=	142,306 cf	
Primary	=	2.88 cfs @	9.11 hrs, Volume=	142,306 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs



Link L3: Pipe 24

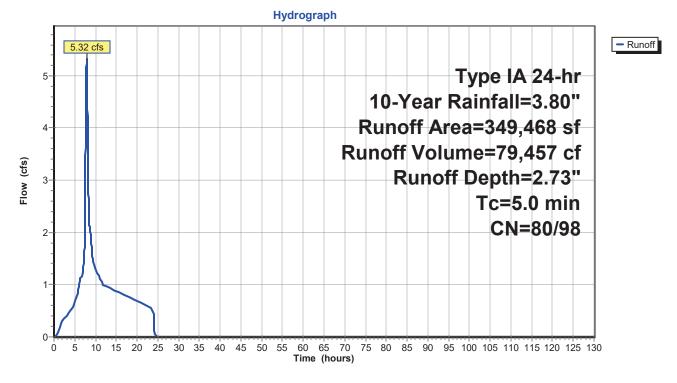
Summary for Subcatchment 1-6, 23: Basins 1-6

Runoff = 5.32 cfs @ 7.91 hrs, Volume= 79,457 cf, Depth= 2.73"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

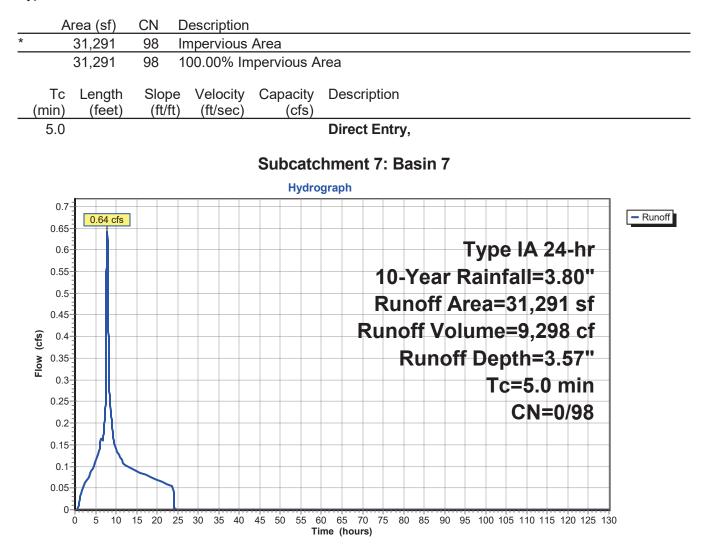
_	Ar	ea (sf)	CN	Description					
*	1	76,142	98	98 Impervious Area					
_	1	73,326	80	0 >75% Grass cover, Good, HSG D					
349,468 89 Weighted Average					verage				
173,326 80 49.60% Pervious Area					vious Area				
	176,142		98	50.40% Impervious Area					
_	Tc (min)	Length (feet)	Slop (ft/ft	,	Capacity (cfs)	Description			
	5.0					Direct Entry,			

Subcatchment 1-6, 23: Basins 1-6



Summary for Subcatchment 7: Basin 7

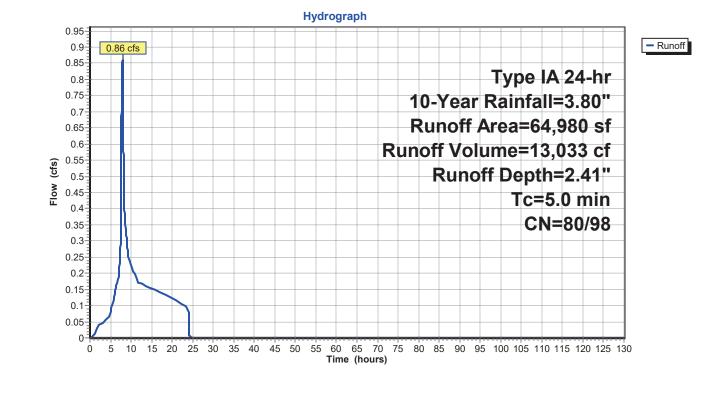
Runoff = 0.64 cfs @ 7.88 hrs, Volume= 9,298 cf, Depth= 3.57"



Summary for Subcatchment 12: Basin 12

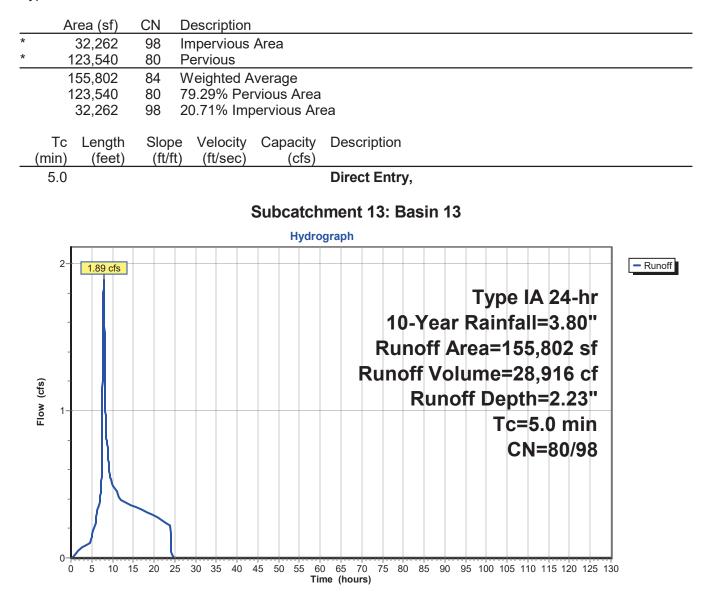
Runoff = 0.86 cfs @ 7.93 hrs, Volume= 13,033 cf, Depth= 2.41"

	A	Area (sf) CN Description								
*		20,370	98	Impervious Area						
*		44,610	80	Pervious						
		64,980 86 Weighted Average								
	44,610 80 68.65% Pervious Ar				rvious Area	l l				
	20,370 98 31.35% Impervious Area					ea				
	Та	Longth	Clan	a Valasitu	Conocity	Description				
	Tc (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description				
	5.0		Direct Entry,							
Subcatchment 12: Basin 12										



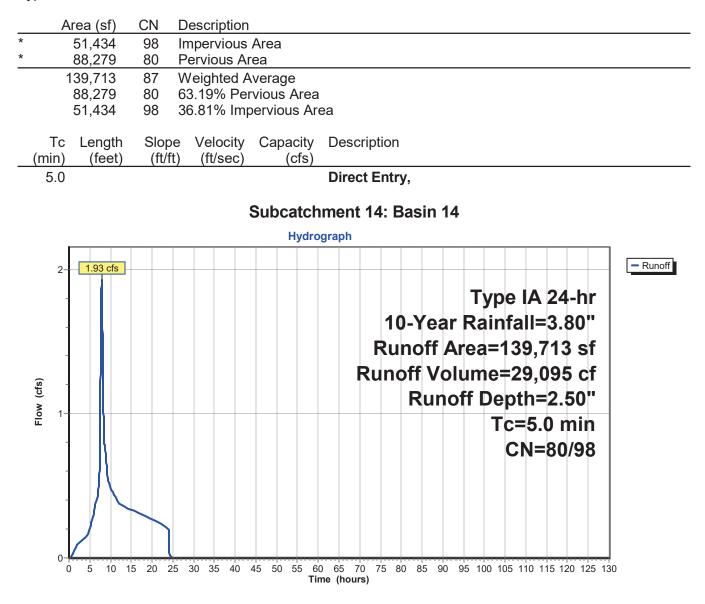
Summary for Subcatchment 13: Basin 13

Runoff = 1.89 cfs @ 7.94 hrs, Volume= 28,916 cf, Depth= 2.23"



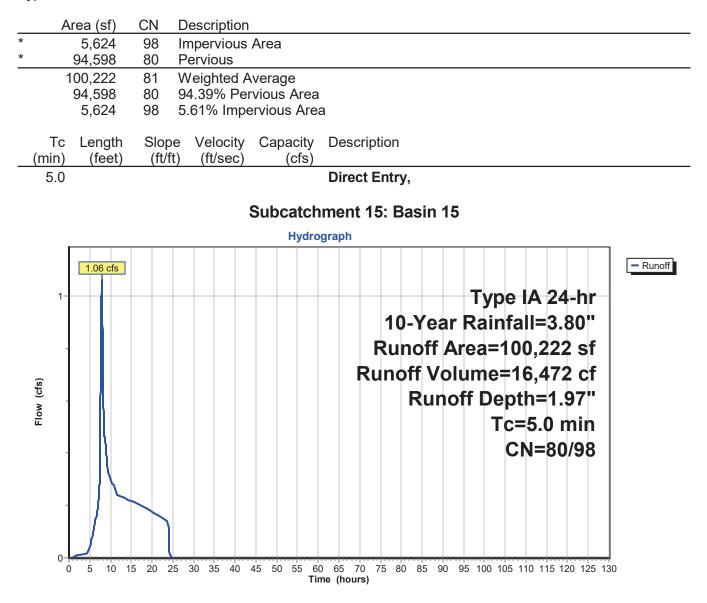
Summary for Subcatchment 14: Basin 14

Runoff = 1.93 cfs @ 7.92 hrs, Volume= 29,095 cf, Depth= 2.50"



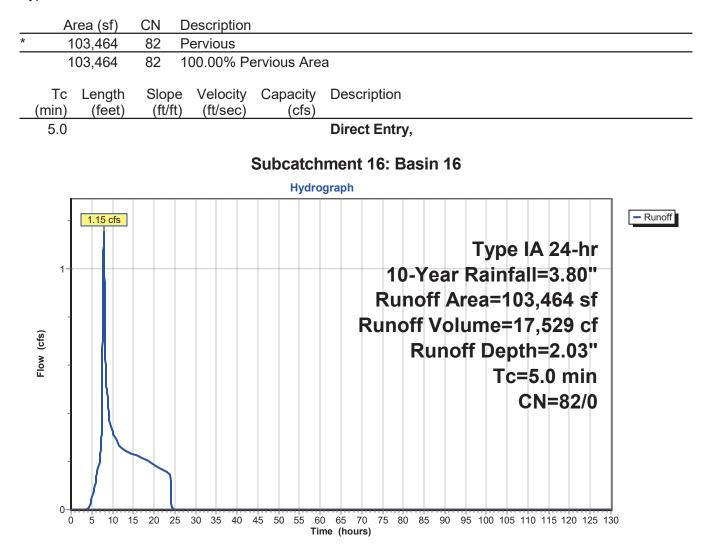
Summary for Subcatchment 15: Basin 15

Runoff = 1.06 cfs @ 7.96 hrs, Volume= 16,472 cf, Depth= 1.97"



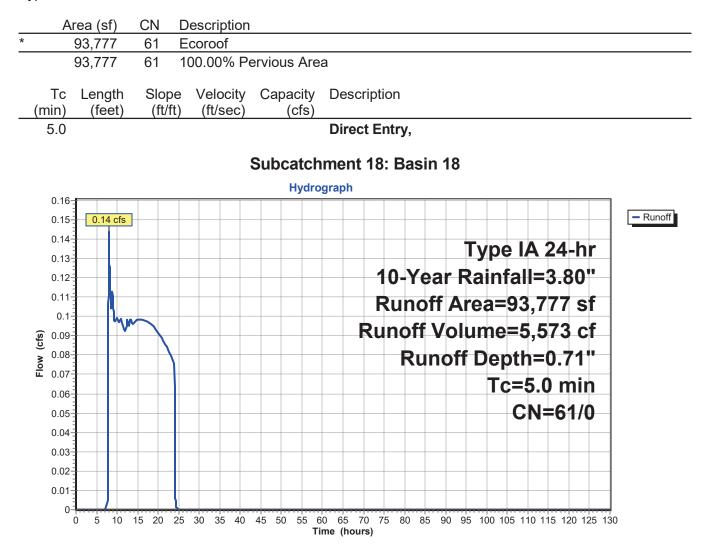
Summary for Subcatchment 16: Basin 16

Runoff = 1.15 cfs @ 7.96 hrs, Volume= 17,529 cf, Depth= 2.03"



Summary for Subcatchment 18: Basin 18

Runoff = 0.14 cfs @ 8.01 hrs, Volume= 5,573 cf, Depth= 0.71"



Summary for Reach R1: Ditch 1

15.74

561

96,271 sf, 53.66% Impervious, Inflow Depth = 2.78" for 10-Year event Inflow Area = Inflow 1.50 cfs @ 7.91 hrs, Volume= 22.330 cf = 7.92 hrs, Volume= Outflow 1.50 cfs @ 22,330 cf, Atten= 0%, Lag= 0.8 min =

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 2.53 fps, Min. Travel Time= 1.2 min Avg. Velocity = 1.31 fps, Avg. Travel Time= 2.2 min

Peak Storage= 104 cf @ 7.92 hrs Average Depth at Peak Storage= 0.22' Bank-Full Depth= 0.75' Flow Area= 3.2 sf, Capacity= 15.74 cfs

Custom cross-section, Length= 176.0' Slope= 0.0187 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 702.30', Outlet Invert= 699.00'

‡

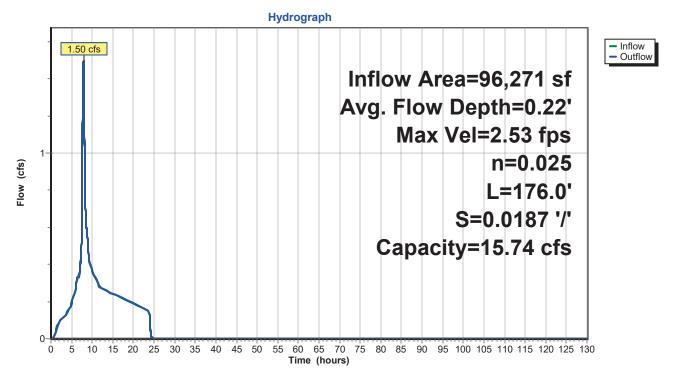
0.75

	Offse	t Eleva	tion	Cha	n.Depth		
_	(feet) (f	eet)		(feet)		
	-3.25	5 ().75		0.00		
	-1.00) (00.0		0.75		
	1.00) (00.0		0.75		
	3.25	5 ().75		0.00		
	Depth E	nd Area	Pe	rim.	S	Storage	Discharge
_	(feet)	(sq-ft)	(fe	eet)	(cub	ic-feet)	(cfs)
	0.00	0.0		2.0		0	0.00

6.7

3.2

Reach R1: Ditch 1



Summary for Reach R2: Ditch 2

Inflow Area =100,222 sf, 5.61% Impervious, Inflow Depth = 1.97" for 10-Year eventInflow =1.06 cfs @7.96 hrs, Volume=16,472 cfOutflow =1.05 cfs @8.00 hrs, Volume=16,472 cf, Atten= 1%, Lag= 2.1 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 2.19 fps, Min. Travel Time= 4.4 min Avg. Velocity = 1.02 fps, Avg. Travel Time= 9.4 min

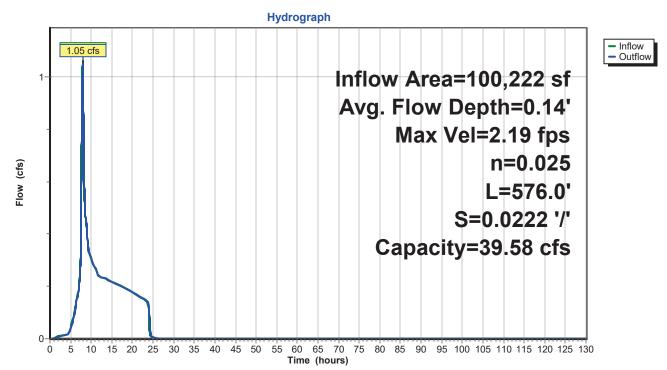
Peak Storage= 277 cf @ 8.00 hrs Average Depth at Peak Storage= 0.14' Bank-Full Depth= 1.00' Flow Area= 6.0 sf, Capacity= 39.58 cfs

Custom cross-section, Length= 576.0' Slope= 0.0222 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 703.59', Outlet Invert= 690.82'

‡

	Offset (feet)	Elevation (feet)	Chan.Depth (feet)	
•	-4.50	1.00	0.00	
	-1.50	0.00	1.00	
	1.50	0.00	1.00	
	4.50	1.00	0.00	
	Depth End	Area Pe	erim. S	Stora

	End Area	Perim.		Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	3.0	0	0.00
1.00	6.0	9.3	3,456	39.58



Reach R2: Ditch 2

Summary for Reach R3: Ditch 3

 Inflow Area =
 155,802 sf, 20.71% Impervious, Inflow Depth = 2.23" for 10-Year event

 Inflow =
 1.89 cfs @
 7.94 hrs, Volume=
 28,916 cf

 Outflow =
 1.87 cfs @
 8.00 hrs, Volume=
 28,916 cf, Atten= 1%, Lag= 3.1 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 1.99 fps, Min. Travel Time= 4.4 min Avg. Velocity = 0.97 fps, Avg. Travel Time= 9.1 min

Peak Storage= 500 cf @ 8.00 hrs Average Depth at Peak Storage= 0.32' Bank-Full Depth= 1.00' Flow Area= 5.0 sf, Capacity= 18.61 cfs

Custom cross-section, Length= 530.0' Slope= 0.0077 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 707.64', Outlet Invert= 703.54'

‡

Offset (feet)	Elevation (feet)	Chan.Depth (feet)	
-4.00	1.00	0.00	
-1.00	0.00	1.00	
1.00	0.00	1.00	
4.00	1.00	0.00	
Depth End	Area Pe	erim.	Stora

Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	2.0 8.3	0 2,650	0.00 18.61

Hydrograph 2 Inflow 1.87 cfs - Outflow Inflow Area=155,802 sf Avg. Flow Depth=0.32' Max Vel=1.99 fps n=0.025 Flow (cfs) L=530.0' S=0.0077 '/' Capacity=18.61 cfs 0 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 5 Ó Time (hours)

Reach R3: Ditch 3

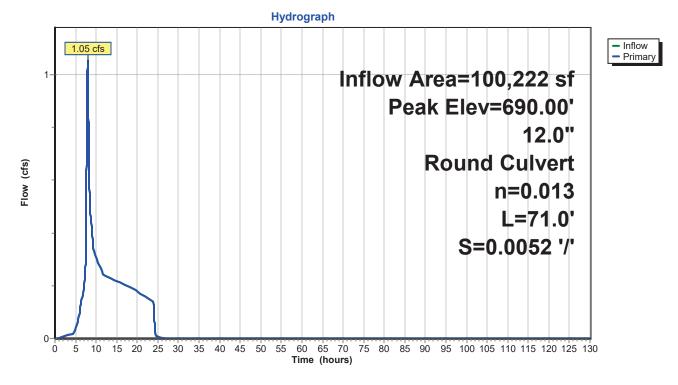
Summary for Pond DI: Ditch Inlet

Inflow Area	a =	100,222 sf,	5.61% Impervious,	Inflow Depth = 1.97" for 10-Year event
Inflow	=	1.05 cfs @	8.00 hrs, Volume=	16,472 cf
Outflow	=	1.05 cfs @	8.00 hrs, Volume=	16,472 cf, Atten= 0%, Lag= 0.0 min
Primary	=	1.05 cfs @	8.00 hrs, Volume=	16,472 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 690.00' @ 8.03 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	689.33'	12.0" Round From Ditch Inlet L= 71.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 689.33' / 688.96' S= 0.0052 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=1.04 cfs @ 8.00 hrs HW=689.99' TW=689.58' (Dynamic Tailwater) **1=From Ditch Inlet** (Outlet Controls 1.04 cfs @ 2.66 fps)



Pond DI: Ditch Inlet

Summary for Pond FSMH: Flow Splitter Manhole

Inflow Area =	492,008 sf,	28.65% Impervious,	Inflow Depth = 2.36" for 10-Year event
Inflow =	2.77 cfs @	8.13 hrs, Volume=	96,821 cf
Outflow =	2.77 cfs @	8.13 hrs, Volume=	96,821 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.59 cfs @	8.13 hrs, Volume=	31,451 cf
Secondary =	2.18 cfs @	8.13 hrs, Volume=	65,371 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 689.63' @ 8.13 hrs

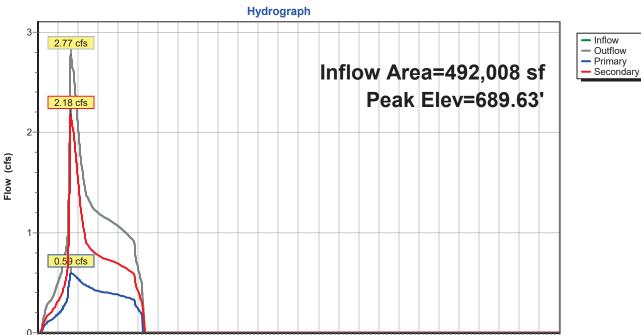
Routing	Invert	Outlet Devices
Primary	688.86'	8.0" Round To Existing Culvert
		L= 38.0' RCP, square edge headwall, Ke= 0.500
		Inlet / Outlet Invert= 688.86' / 688.65' S= 0.0055 '/' Cc= 0.900
		n= 0.013, Flow Area= 0.35 sf
Device 1	687.36'	5.0" Horiz. Orifice C= 0.620 Limited to weir flow at low heads
Secondary	688.86'	18.0" Round Bypassed Flow
		L= 148.0' RCP, square edge headwall, Ke= 0.500
		Inlet / Outlet Invert= 688.86' / 688.11' S= 0.0051 '/' Cc= 0.900
		n= 0.013, Flow Area= 1.77 sf
	Primary Device 1	Primary 688.86' Device 1 687.36'

Primary OutFlow Max=0.59 cfs @ 8.13 hrs HW=689.63' (Free Discharge)

1=To Existing Culvert (Passes 0.59 cfs of 0.93 cfs potential flow) **2=Orifice** (Orifice Controls 0.59 cfs @ 4.36 fps)

Secondary OutFlow Max=2.18 cfs @ 8.13 hrs HW=689.63' TW=0.00' (Dynamic Tailwater) -3=Bypassed Flow (Barrel Controls 2.18 cfs @ 3.49 fps)

5 Ó



Pond FSMH: Flow Splitter Manhole

10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 Time (hours)

Summary for Pond Pond A: Pond A

Inflow Area =	349,468 sf, 50.40% Impervious,	Inflow Depth = 2.73" for 10-Year event
Inflow =	5.32 cfs @ 7.91 hrs, Volume=	79,457 cf
Outflow =	1.53 cfs @ 9.26 hrs, Volume=	79,464 cf, Atten= 71%, Lag= 80.8 min
Primary =	1.53 cfs @ 9.26 hrs, Volume=	79,464 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 709.83' @ 9.26 hrs Surf.Area= 8,641 sf Storage= 17,232 cf

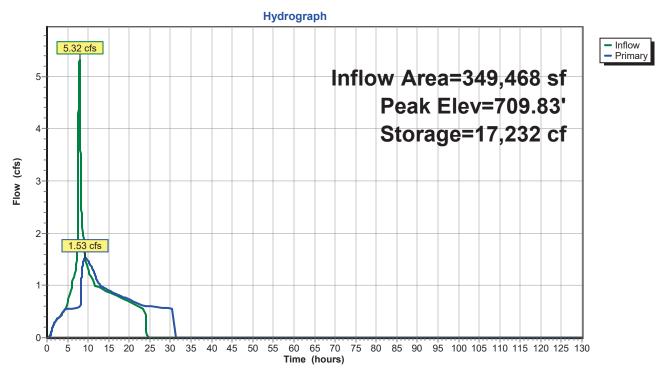
Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 221.5 min (930.6 - 709.1)

Volume	Invei	rt Avail.Sto	rage Storage [Description		
#1	707.50)' 70,55	55 cf Custom	of Custom Stage Data (Prismatic) Listed below (Recalc)		
F laviatia			la chana	Ourse Otherse		
Elevatio		Surf.Area	Inc.Store	Cum.Store		
(fee	/	(sq-ft)	(cubic-feet)	(cubic-feet)		
707.5	50	6,107	0	0		
708.0	00	6,714	3,205	3,205		
709.0	00	7,742	7,228	10,433		
710.0	00	8,825	8,284	18,717		
711.0	00	9,967	9,396	28,113		
712.0	00	11,161	10,564	38,677		
713.0	00	12,412	11,787	50,463		
714.0		13,717	13,065	63,528		
714.5		14,392	7,027	70,555		
		·				
Device	Routing	Invert	Outlet Devices	;		
#1	Primary	698.43'	18.0" Round	Culvert		
			L= 138.0' CM	P. square edge h	eadwall, Ke= 0.500	
					6.24' S= 0.0159 '/' Cc= 0.900	
				v Area= 1.77 sf		
#2	Device 1	696.43'	,		.620 Limited to weir flow at low heads	
#3	Device 1	709.40'				
110	2001001	100.40				
· · · · ·	Primary OutFlow Max=1.53 cfs @ 9.26 hrs HW=709.83' TW=0.00' (Dynamic Tailwater)					

1=Culvert (Passes 1.53 cfs of 24.58 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.62 cfs @ 16.80 fps)

-3=Orifice/Grate (Orifice Controls 0.91 cfs @ 2.73 fps)



Pond Pond A: Pond A

Summary for Pond Pond D: Pond D

Inflow Area	ı =	391,786 sf,	34.55% Impervious,	Inflow Depth = 2.46" for 10-Year event
Inflow	=	5.28 cfs @	7.95 hrs, Volume=	80,341 cf
Outflow	=	2.16 cfs @	8.69 hrs, Volume=	80,349 cf, Atten= 59%, Lag= 44.4 min
Primary	=	2.16 cfs @	8.69 hrs, Volume=	80,349 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 695.22' @ 8.69 hrs Surf.Area= 6,610 sf Storage= 12,230 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 106.0 min (840.7 - 734.7)

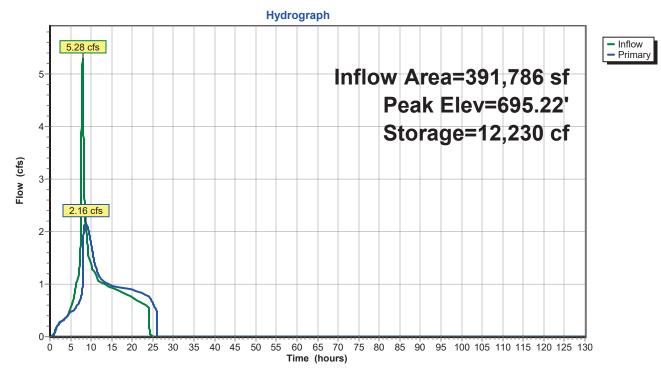
Volume	Inve	rt Avail.Sto	age Storage Description			
#1	693.0	0' 45,10	06 cf Custom	Stage Data (Pr	rismatic) Listed below (Recalc)	
_						
Elevatio		Surf.Area	Inc.Store	Cum.Store		
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)		
693.0	00	4,453	0	0		
694.0)0	5,388	4,921	4,921		
695.0	00	6,380	5,884	10,805		
696.0	00	7,428	6,904	17,709		
697.0	00	8,533	7,981	25,689		
698.0	00	9,694	9,114	34,803		
699.0)0	10,912	10,303	45,106		
Device	Routing	Invert	Outlet Devices	S		
#1	Primary	692.50'	12.0" Round	Culvert		
	-		L= 121.0' CN	IP, square edge	e headwall, Ke= 0.500	
			Inlet / Outlet Ir	nvert= 692.50' /	'688.97' S= 0.0292 '/' Cc= 0.900	
			n= 0.013, Flo	w Area= 0.79 s	f	
#2	Device 1	690.50'	4.9" Horiz. Or	ifice/Grate C	= 0.620 Limited to weir flow at low heads	
#3	Device 1	694.60'	15.0" W x 3.0'	H Vert. Orifice	e/Grate C= 0.620	
Primary	OutFlow	Max=2 16 cfs (බ 8 69 hrs HW	=695 22' TW=	689.60' (Dynamic Tailwater)	

Primary OutFlow Max=2.16 cfs @ 8.69 hrs HW=695.22' TW=689.60' (Dynamic Tailwater)

-1=Culvert (Passes 2.16 cfs of 5.63 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.07 cfs @ 8.20 fps)

-3=Orifice/Grate (Orifice Controls 1.09 cfs @ 3.49 fps)

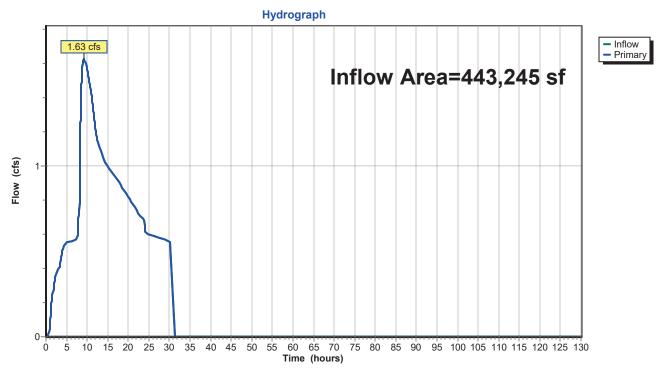


Pond Pond D: Pond D

Summary for Link L1: Pipe 2

Inflow Area	a =	443,245 sf,	39.74% Impervious,	Inflow Depth = 2.30" for 10-Year event	
Inflow	=	1.63 cfs @	9.21 hrs, Volume=	85,036 cf	
Primary	=	1.63 cfs @	9.21 hrs, Volume=	85,036 cf, Atten= 0%, Lag= 0.0 min	

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs

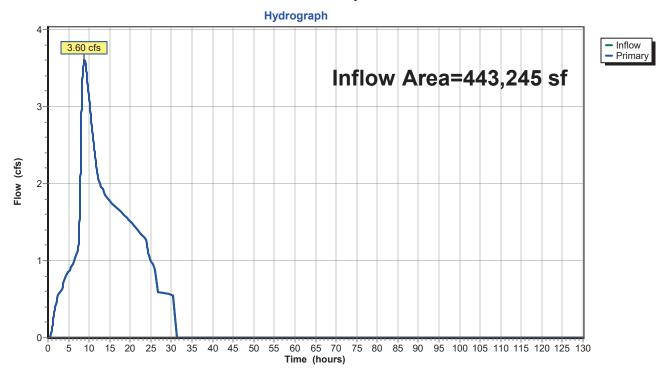


Link L1: Pipe 2

Summary for Link L2: Pipe 19

Inflow Are	a =	443,245 sf,	39.74% Impervious,	Inflow Depth = 4.07"	for 10-Year event
Inflow	=	3.60 cfs @	8.86 hrs, Volume=	150,407 cf	
Primary	=	3.60 cfs @	8.86 hrs, Volume=	150,407 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs

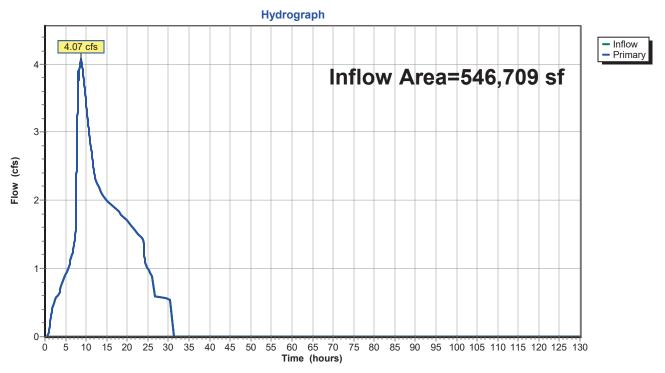


Link L2: Pipe 19

Summary for Link L3: Pipe 24

Inflow Area	a =	546,709 sf,	32.22% Impervious,	Inflow Depth = 3.69"	for 10-Year event
Inflow	=	4.07 cfs @	8.76 hrs, Volume=	167,937 cf	
Primary	=	4.07 cfs @	8.76 hrs, Volume=	167,937 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs



Link L3: Pipe 24

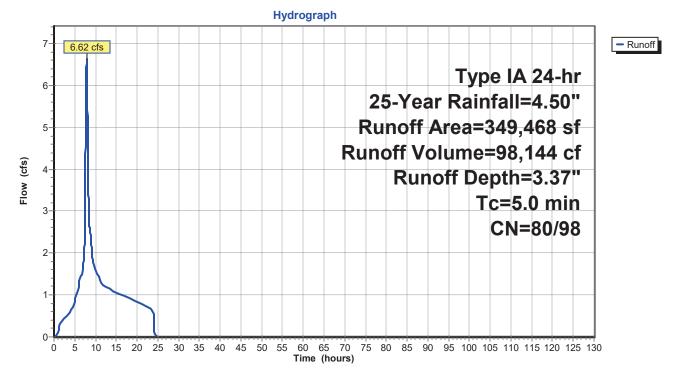
Summary for Subcatchment 1-6, 23: Basins 1-6

Runoff = 6.62 cfs @ 7.91 hrs, Volume= 98,144 cf, Depth= 3.37"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 25-Year Rainfall=4.50"

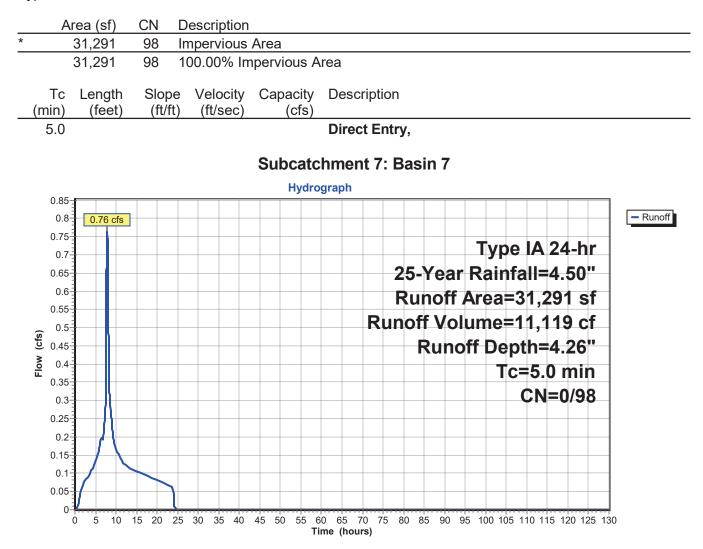
_	A	rea (sf)	CN	Description				
*	1	76,142	98	Impervious	Area			
_	1	73,326	80	>75% Gras	s cover, Go	bod, HSG D		
	3	49,468	89	Weighted Average				
	1	73,326	80	30 49.60% Pervious Area				
	1	76,142	98	50.40% Imp	ervious Ar	ea		
_	Tc (min)	Length (feet)	Slop (ft/fl	,	Capacity (cfs)	Description		
	5.0					Direct Entry,		

Subcatchment 1-6, 23: Basins 1-6



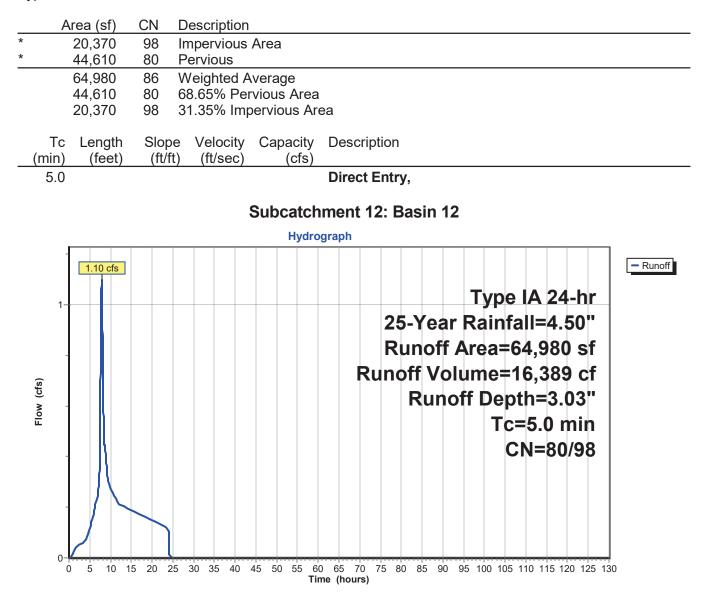
Summary for Subcatchment 7: Basin 7

Runoff = 0.76 cfs @ 7.88 hrs, Volume= 11,119 cf, Depth= 4.26"



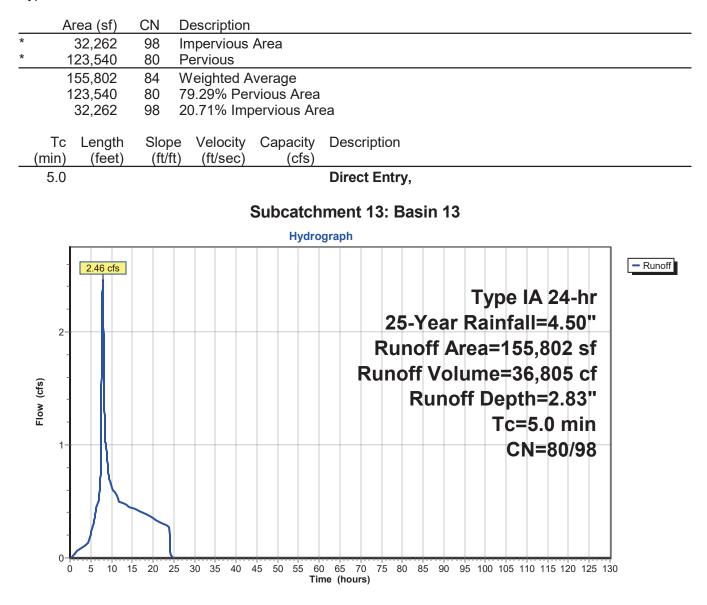
Summary for Subcatchment 12: Basin 12

Runoff = 1.10 cfs @ 7.92 hrs, Volume= 16,389 cf, Depth= 3.03"



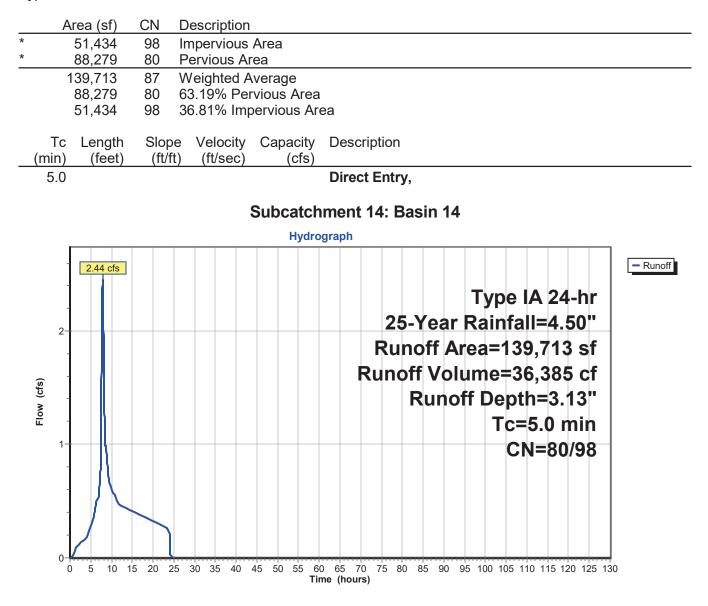
Summary for Subcatchment 13: Basin 13

Runoff = 2.46 cfs @ 7.93 hrs, Volume= 36,805 cf, Depth= 2.83"



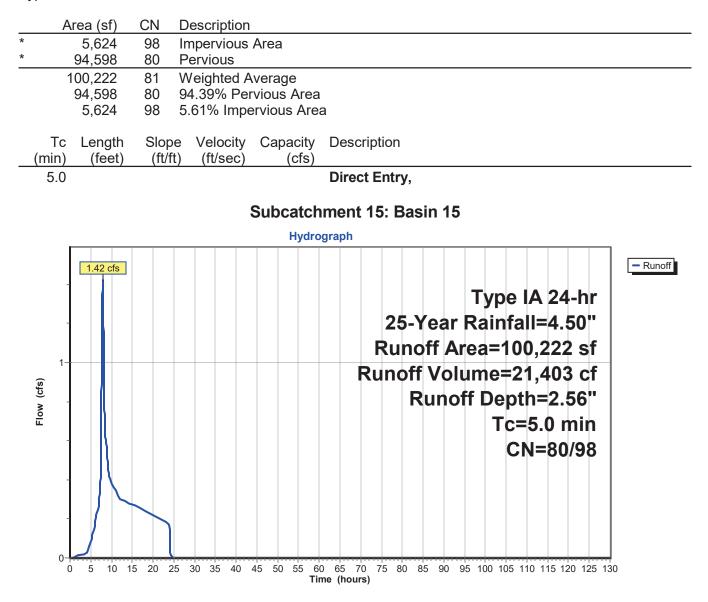
Summary for Subcatchment 14: Basin 14

Runoff = 2.44 cfs @ 7.92 hrs, Volume= 36,385 cf, Depth= 3.13"



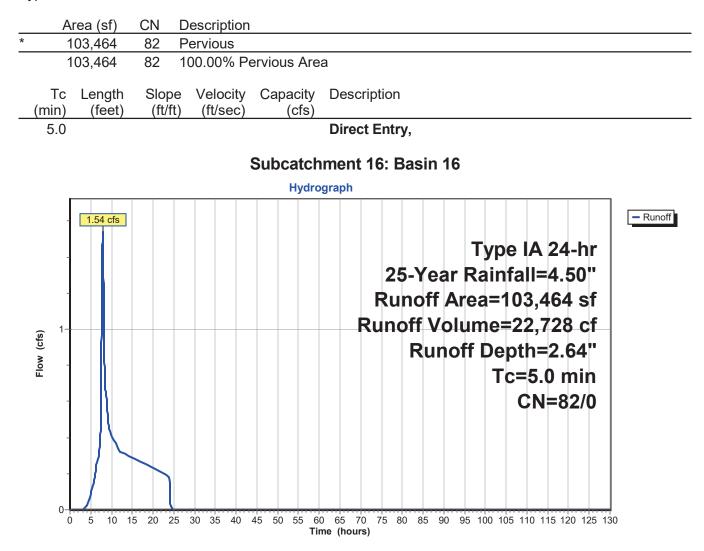
Summary for Subcatchment 15: Basin 15

Runoff = 1.42 cfs @ 7.95 hrs, Volume= 21,403 cf, Depth= 2.56"



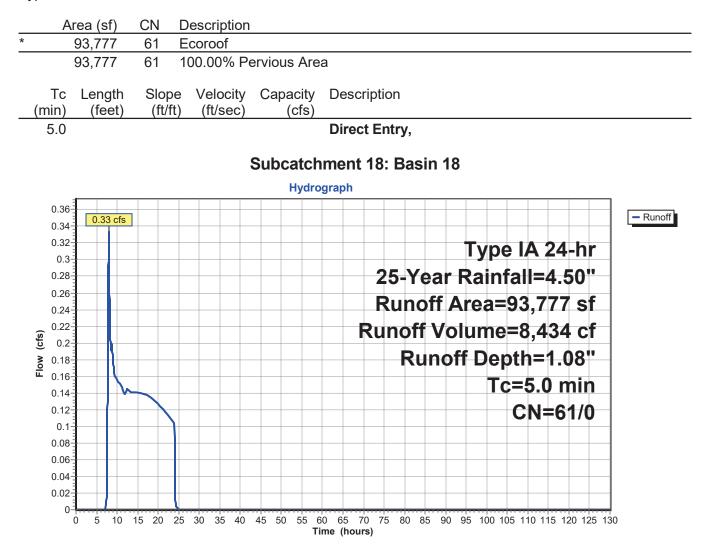
Summary for Subcatchment 16: Basin 16

Runoff = 1.54 cfs @ 7.95 hrs, Volume= 22,728 cf, Depth= 2.64"



Summary for Subcatchment 18: Basin 18

Runoff = 0.33 cfs @ 8.00 hrs, Volume= 8,434 cf, Depth= 1.08"



Summary for Reach R1: Ditch 1

15.74

561

96,271 sf, 53.66% Impervious, Inflow Depth = 3.43" for 25-Year event Inflow Area = Inflow = 1.86 cfs @ 7.90 hrs, Volume= 27.508 cf 7.92 hrs, Volume= Outflow 1.86 cfs @ 27,508 cf, Atten= 0%, Lag= 0.8 min =

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 2.71 fps, Min. Travel Time= 1.1 min Avg. Velocity = 1.40 fps, Avg. Travel Time= 2.1 min

Peak Storage= 121 cf @ 7.92 hrs Average Depth at Peak Storage= 0.25' Bank-Full Depth= 0.75' Flow Area= 3.2 sf, Capacity= 15.74 cfs

Custom cross-section, Length= 176.0' Slope= 0.0187 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 702.30', Outlet Invert= 699.00'

‡

	Offs	set Elev	ation	Cha	n.Depth	า		
_	(fe	et)	(feet)		(feet)		
	-3.	25	0.75		0.00	C		
	-1.	00	0.00		0.75	5		
	1.	00	0.00		0.75	5		
	3.	25	0.75		0.00	C		
	Depth	End Area	Pe	erim.		Storage	Disch	narge
	(feet)	(sq-ft)	(1	feet)	(CL	ubic-feet)		(cfs)
	0.00	0.0		2.0		0		0.00

6.7

3.2

0.75

2

Flow (cfs)

1

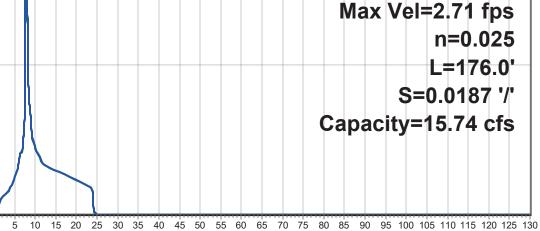
0

Ó

Inflow
Outflow

Hydrograph

Reach R1: Ditch 1



Time (hours)

Summary for Reach R2: Ditch 2

(cfs) 0.00

39.58

0

3,456

100,222 sf, 5.61% Impervious, Inflow Depth = 2.56" Inflow Area = for 25-Year event Inflow 7.95 hrs. Volume= 1.42 cfs @ 21.403 cf = 7.99 hrs, Volume= Outflow 1.41 cfs @ 21,403 cf, Atten= 1%, Lag= 2.6 min =

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 2.43 fps, Min. Travel Time= 4.0 min Avg. Velocity = 1.12 fps, Avg. Travel Time= 8.6 min

Peak Storage= 335 cf @ 7.99 hrs Average Depth at Peak Storage= 0.17' Bank-Full Depth= 1.00' Flow Area= 6.0 sf, Capacity= 39.58 cfs

Custom cross-section, Length= 576.0' Slope= 0.0222 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 703.59', Outlet Invert= 690.82'

‡

0.00

1.00

	Offset		Chan.Depth		
_	(feet)	(feet)	(feet)		
	-4.50	1.00	0.00		
	-1.50	0.00	1.00		
	1.50	0.00	1.00		
	4.50	1.00	0.00		
	Depth End	l Area 🛛 Pe	erim.	Storage	Discharge
_	(feet)	(sq-ft) (t	feet) (cub	oic-feet)	(cfs)

3.0

9.3

0.0

6.0

Hydrograph Inflow 1.41 cfs - Outflow Inflow Area=100,222 sf Avg. Flow Depth=0.17' Max Vel=2.43 fps 1 n=0.025 Flow (cfs) L=576.0' S=0.0222 '/' Capacity=39.58 cfs 0 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 5 Ó Time (hours)

Reach R2: Ditch 2

Summary for Reach R3: Ditch 3

 Inflow Area =
 155,802 sf, 20.71% Impervious, Inflow Depth = 2.83" for 25-Year event

 Inflow =
 2.46 cfs @
 7.93 hrs, Volume=
 36,805 cf

 Outflow =
 2.44 cfs @
 7.98 hrs, Volume=
 36,805 cf, Atten= 1%, Lag= 3.0 min

Diasharra

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 2.15 fps, Min. Travel Time= 4.1 min Avg. Velocity = 1.04 fps, Avg. Travel Time= 8.5 min

Peak Storage= 603 cf @ 7.98 hrs Average Depth at Peak Storage= 0.37' Bank-Full Depth= 1.00' Flow Area= 5.0 sf, Capacity= 18.61 cfs

Custom cross-section, Length= 530.0' Slope= 0.0077 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 707.64', Outlet Invert= 703.54'

‡

Offset (feet)	Elevation (feet)	Chan.Depth (feet)	
-4.00	1.00	0.00	
-1.00	0.00	1.00	
1.00	0.00	1.00	
4.00	1.00	0.00	
Depth End	Area Pe	rim.	Storag

End Area	Perim.	Storage	Discharge
(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.0	2.0	0	0.00
5.0	8.3	2,650	18.61
	(sq-ft) 0.0	(sq-ft) (feet) 0.0 2.0	(sq-ft) (feet) (cubic-feet) 0.0 2.0 0

Hydrograph Inflow 2.44 cfs - Outflow Inflow Area=155,802 sf Avg. Flow Depth=0.37' 2 Max Vel=2.15 fps n=0.025 Flow (cfs) L=530.0' S=0.0077 '/' 1 Capacity=18.61 cfs 0 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 5 Ó Time (hours)

Reach R3: Ditch 3

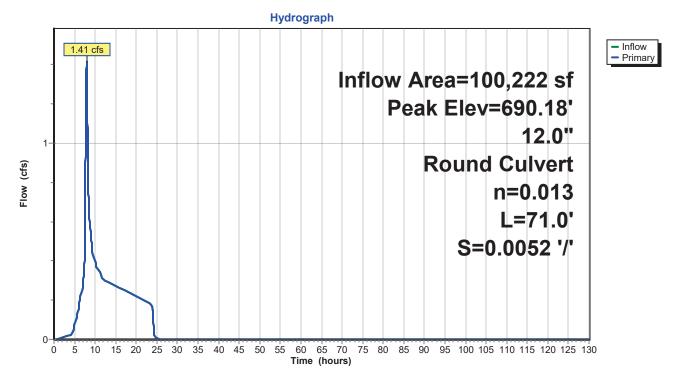
Summary for Pond DI: Ditch Inlet

Inflow Area	a =	100,222 sf,	5.61% Impervious,	Inflow Depth = 2.56" for 25-Year event
Inflow	=	1.41 cfs @	7.99 hrs, Volume=	21,403 cf
Outflow	=	1.41 cfs @	7.99 hrs, Volume=	21,403 cf, Atten= 0%, Lag= 0.0 min
Primary	=	1.41 cfs @	7.99 hrs, Volume=	21,403 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 690.18' @ 8.01 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	689.33'	12.0" Round From Ditch Inlet L= 71.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 689.33' / 688.96' S= 0.0052 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=1.41 cfs @ 7.99 hrs HW=690.18' TW=689.80' (Dynamic Tailwater) **1=From Ditch Inlet** (Outlet Controls 1.41 cfs @ 2.68 fps)



Pond DI: Ditch Inlet

Summary for Pond FSMH: Flow Splitter Manhole

Inflow Area =	492,008 sf,	28.65% Impervious,	Inflow Depth = 2.98" for 25-Year event
Inflow =	3.84 cfs @	8.03 hrs, Volume=	122,101 cf
Outflow =	3.84 cfs @	8.03 hrs, Volume=	122,101 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.66 cfs @	8.03 hrs, Volume=	36,262 cf
Secondary =	3.18 cfs @	8.03 hrs, Volume=	85,840 cf

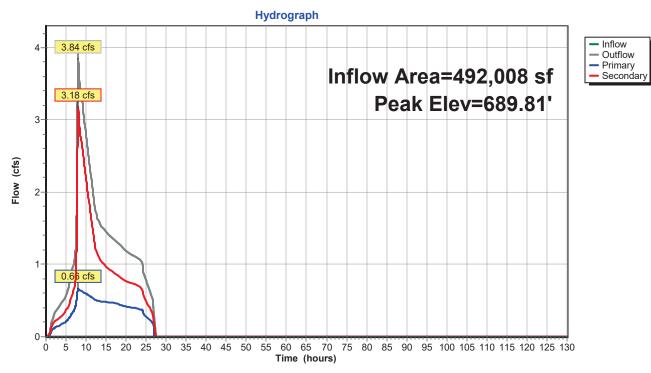
Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 689.81' @ 8.03 hrs

Device	Routing	Invert	Outlet Devices			
#1	Primary	688.86'	8.0" Round To Existing Culvert			
			L= 38.0' RCP, square edge headwall, Ke= 0.500			
			Inlet / Outlet Invert= 688.86' / 688.65' S= 0.0055 '/' Cc= 0.900			
			n= 0.013, Flow Area= 0.35 sf			
#2	Device 1	687.36'	5.0" Horiz. Orifice C= 0.620 Limited to weir flow at low heads			
#3	Secondary	688.86'	18.0" Round Bypassed Flow			
	L= 148.0' RCP, square edge headwall, Ke= 0.500					
			Inlet / Outlet Invert= 688.86' / 688.11' S= 0.0051 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf			

Primary OutFlow Max=0.66 cfs @ 8.03 hrs HW=689.81' (Free Discharge)

1=To Existing Culvert (Passes 0.66 cfs of 1.05 cfs potential flow) **2=Orifice** (Orifice Controls 0.66 cfs @ 4.85 fps)

Secondary OutFlow Max=3.18 cfs @ 8.03 hrs HW=689.81' TW=0.00' (Dynamic Tailwater) -3=Bypassed Flow (Barrel Controls 3.18 cfs @ 3.83 fps)



Pond FSMH: Flow Splitter Manhole

Summary for Pond Pond A: Pond A

Inflow Area =	349,468 sf, 50.40% Impervious,	Inflow Depth = 3.37" for 25-Year event
Inflow =	6.62 cfs @ 7.91 hrs, Volume=	98,144 cf
Outflow =	2.09 cfs @ 9.07 hrs, Volume=	98,147 cf, Atten= 68%, Lag= 69.5 min
Primary =	2.09 cfs @ 9.07 hrs, Volume=	98,147 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 710.30' @ 9.07 hrs Surf.Area= 9,166 sf Storage= 21,406 cf

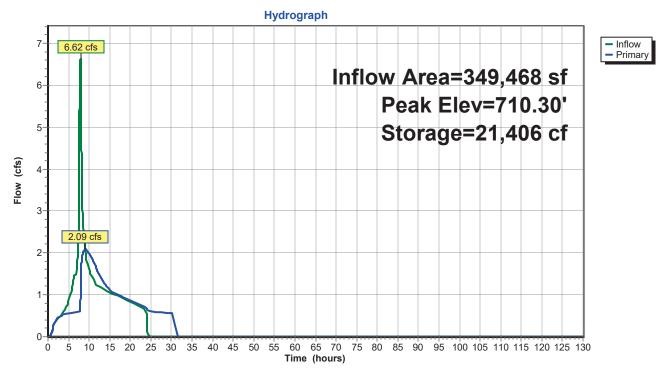
Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 199.1 min (902.8 - 703.7)

Volume	Inve	rt Avail.Sto	brage Storage Description				
#1	707.50	0' 70,55	55 cf Custom S	Stage Data (Prisn	natic) Listed below (Recalc)		
				_			
Elevatio		Surf.Area	Inc.Store	Cum.Store			
(fee	t)	(sq-ft)	(cubic-feet)	(cubic-feet)			
707.5	50	6,107	0	0			
708.0	0	6,714	3,205	3,205			
709.0	0	7,742	7,228	10,433			
710.0	0	8,825	8,284	18,717			
711.0	0	9,967	9,396	28,113			
712.0	0	11,161	10,564	38,677			
713.0	0	12,412	11,787	50,463			
714.0	0	13,717	13,065	63,528			
714.5	50	14,392	7,027	70,555			
Device	Routing	Invert	Outlet Devices				
#1	Primary	698.43'	18.0" Round Culvert				
			L= 138.0' CMI	P, square edge h	eadwall, Ke= 0.500		
			Inlet / Outlet Inv	vert= 698.43' / 69	6.24' S= 0.0159 '/' Cc= 0.900		
			n= 0.013, Flow	/ Area= 1.77 sf			
#2	Device 1	696.43'	2.6" Horiz. Orifice/Grate C= 0.620 Limited to weir flow at low heads				
#3	Device 1	709.40'	16.0" W x 3.0" H Vert. Orifice/Grate C= 0.620				
Primary OutFlow Max=2.09 cfs @ 9.07 hrs HW=710.30' TW=0.00' (Dynamic Tailwater)							

-1=Culvert (Passes 2.09 cfs of 25.05 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.63 cfs @ 17.14 fps)

-3=Orifice/Grate (Orifice Controls 1.46 cfs @ 4.37 fps)



Pond Pond A: Pond A

Summary for Pond Pond D: Pond D

Inflow Area =	391,786 sf,	34.55% Impervious,	Inflow Depth = 3.08"	for 25-Year event
Inflow =	6.72 cfs @	7.94 hrs, Volume=	100,698 cf	
Outflow =	2.79 cfs @	8.51 hrs, Volume=	100,698 cf, Atter	n= 59%, Lag= 34.2 min
Primary =	2.79 cfs @	8.51 hrs, Volume=	100,698 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 695.79' @ 8.51 hrs Surf.Area= 7,209 sf Storage= 16,179 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 105.6 min (832.9 - 727.3)

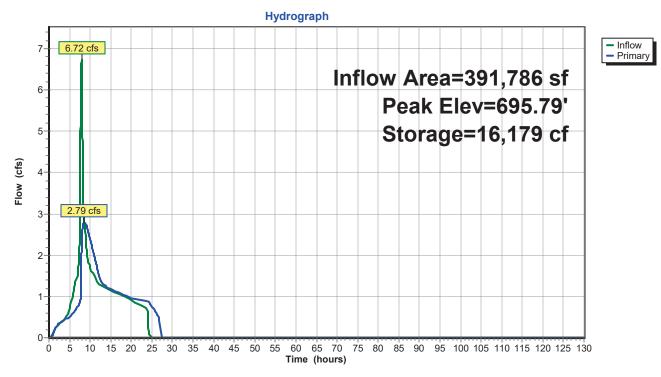
Volume	Inve	rt Avail.Sto	rage Storage	Description	
#1	693.0	0' 45,10	06 cf Custom	Stage Data (Pr	ismatic) Listed below (Recalc)
Elevatio		Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
693.0	00	4,453	0	0	
694.0	00	5,388	4,921	4,921	
695.0	00	6,380	5,884	10,805	
696.0	00	7,428	6,904	17,709	
697.0	00	8,533	7,981	25,689	
698.0	00	9,694	9,114	34,803	
699.0)0	10,912	10,303	45,106	
Device	Routing	Invert	Outlet Device	S	
#1	Primary	692.50'	12.0" Round	Culvert	
	2		L= 121.0' CN	/IP, square edge	e headwall, Ke= 0.500
			Inlet / Outlet I	nvert= 692.50'/	688.97' S= 0.0292 '/' Cc= 0.900
			n= 0.013, Flo	w Area= 0.79 st	F
#2	Device 1	690.50'	4.9" Horiz. O	rifice/Grate Ca	= 0.620 Limited to weir flow at low heads
#3	Device 1	694.60'	15.0" W x 3.0	" H Vert. Orifice	e/Grate C= 0.620
Primary	OutFlow	Max=2 79 cfs (@ 8.51 hrs HW	=695 79' TW=	689 74' (Dynamic Tailwater)

Primary OutFlow Max=2.79 cfs @ 8.51 hrs HW=695.79' TW=689.74' (Dynamic Tailwater)

-1=Culvert (Passes 2.79 cfs of 6.32 cfs potential flow)

-2=Orifice/Grate (Orifice Controls 1.18 cfs @ 9.03 fps)

-3=Orifice/Grate (Orifice Controls 1.60 cfs @ 5.13 fps)

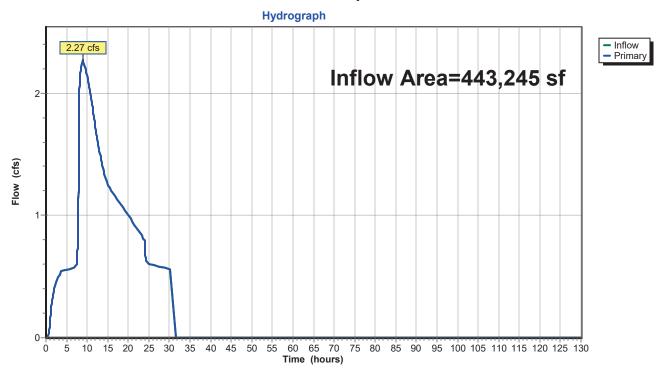


Pond Pond D: Pond D

Summary for Link L1: Pipe 2

Inflow Area	a =	443,245 sf,	39.74% Impervious,	Inflow Depth = 2.89 "	for 25-Year event
Inflow	=	2.27 cfs @	8.94 hrs, Volume=	106,582 cf	
Primary	=	2.27 cfs @	8.94 hrs, Volume=	106,582 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs

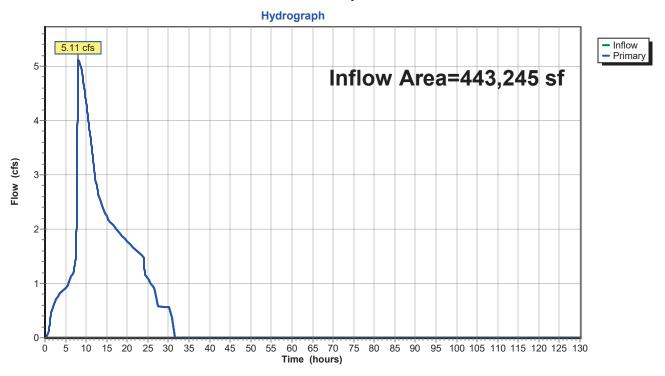


Link L1: Pipe 2

Summary for Link L2: Pipe 19

Inflow Area	a =	443,245 sf,	39.74% Impervious,	Inflow Depth = 5.21"	for 25-Year event
Inflow	=	5.11 cfs @	8.12 hrs, Volume=	192,421 cf	
Primary	=	5.11 cfs @	8.12 hrs, Volume=	192,421 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs

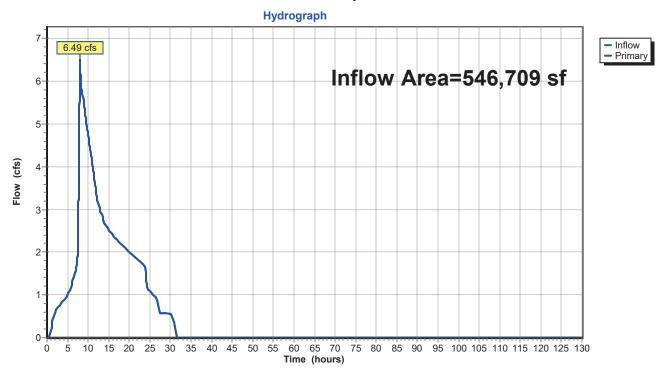


Link L2: Pipe 19

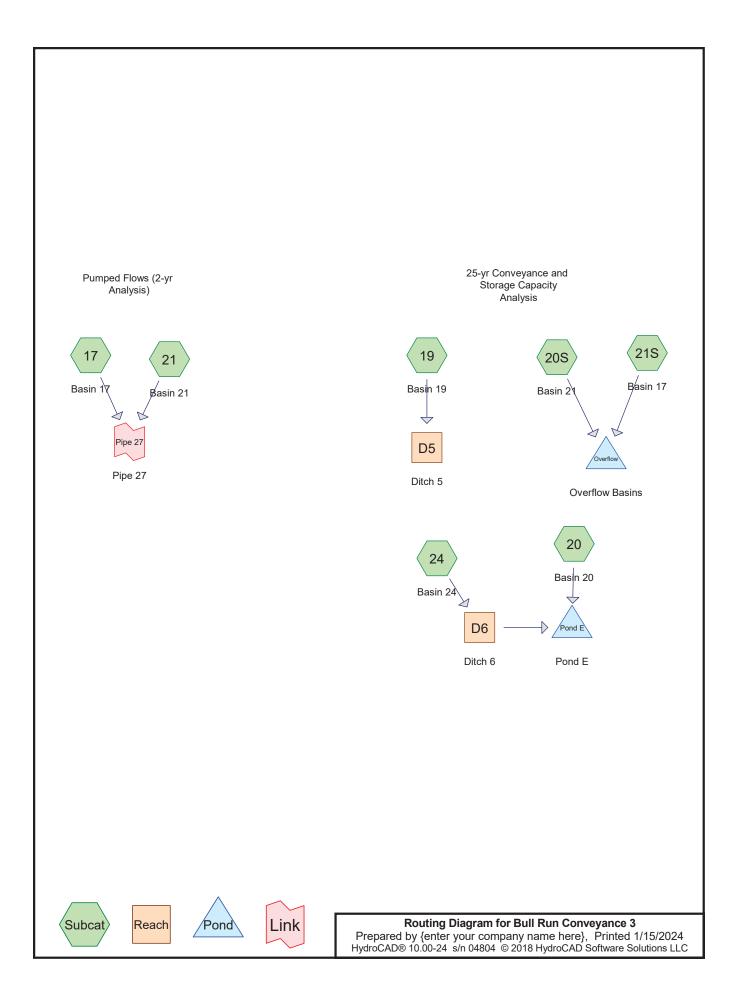
Summary for Link L3: Pipe 24

Inflow Are	a =	546,709 sf,	32.22% Impervious,	Inflow Depth = 4.72"	for 25-Year event
Inflow	=	6.49 cfs @	8.01 hrs, Volume=	215,150 cf	
Primary	=	6.49 cfs @	8.01 hrs, Volume=	215,150 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs



Link L3: Pipe 24

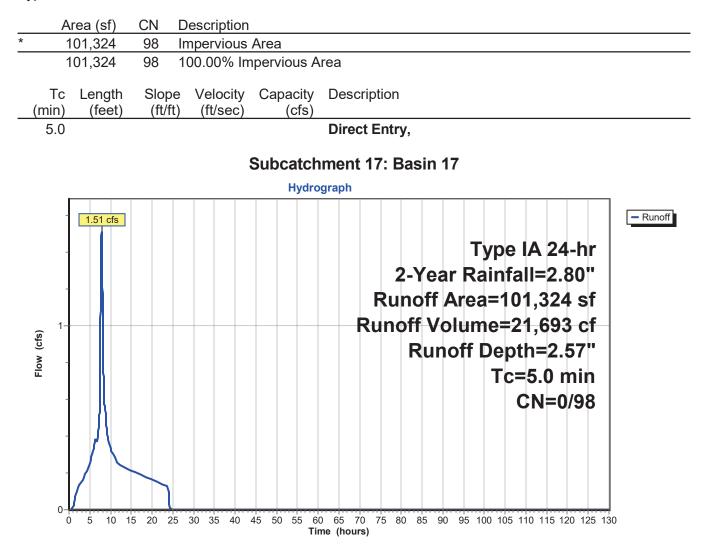


Area Listing (selected nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
17,049	98	(20)
544,328	80	>75% Grass cover, Good, HSG D (20, 24)
701,982	98	Impervious Area (17, 19, 20, 20S, 21, 21S)
284,597	80	Pervious (19, 20S, 21)
1,547,956	88	TOTAL AREA

Summary for Subcatchment 17: Basin 17

Runoff = 1.51 cfs @ 7.88 hrs, Volume= 21,693 cf, Depth= 2.57"



0.09

0.07-0.06-0.05-0.04-0.03-0.02-0.01-0-

Ó

CN=80/98

Summary for Subcatchment 19: Basin 19

Runoff = 0.22 cfs @ 7.98 hrs, Volume= 3,625 cf, Depth= 1.29"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Year Rainfall=2.80"

	А	rea (sf)	CN E	escription							
*		4,230	98 li	npervious	Area						
*		29,613		ervious							
		33,843	82 V	Veighted A	verage						
		29,613			vious Area						
		4,230	98 1	2.50% Imp	pervious Are	ea					
	Тс	Length	Slope	Velocity	Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	5.0					Direct Entry,					
	Subcatchment 19: Basin 19										
				,	Subcatch	ment 19: Ba	sin 19				
					Hydro	graph					
	0.24- 0.23-										- Runoff
	0.23										
	0.21- 0.2-						<u> </u>	уре	IA 2	24-hr	•
	0.19- 0.18-					2-)	ear R	ainfa	all=2	2.80"	
	0.17- 0.16-					Run	off Ar	ea=3	38	43 cf	F
	0.16-										
	(s) 0.14- 0.13-					Runo	ff Volu	me=	:3,6	25 ct	
	≥ 0.12-					R	unoff	Dept	th='	1.29"	1
	₩ 0.11- 0.1-							-) min	
	0 09-										

5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130

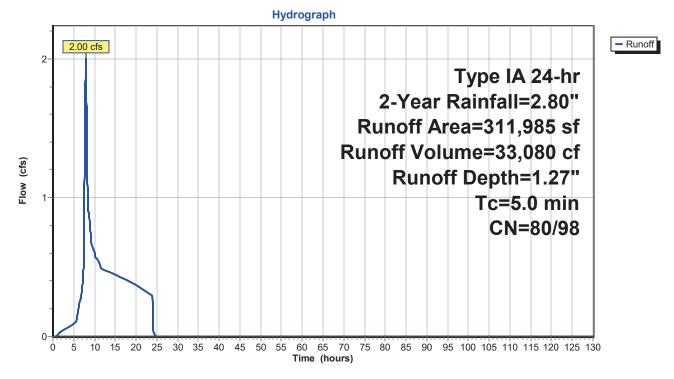
Time (hours)

Summary for Subcatchment 20: Basin 20

Runoff = 2.00 cfs @ 7.98 hrs, Volume= 33,080 cf, Depth= 1.27"

	Area (sf)	CN	Description
*	19,160	98	Impervious Area
	275,776	80	>75% Grass cover, Good, HSG D
*	17,049	98	
	311,985	82	Weighted Average
	275,776	80	88.39% Pervious Area
	36,209	98	11.61% Impervious Area
	Tc Length (min) (feet)	Slop (ft/	
	5.0		Direct Entry,

Subcatchment 20: Basin 20



Summary for Subcatchment 20S: Basin 21

Runoff = 4.20 cfs @ 7.90 hrs, Volume= 62,658 cf, Depth= 2.06"

	Area (sf)	CN Description							
*	237,972	98 Impervious							
*	127,492	80 Pervious							
	365,464	92 Weighted A							
	127,492		rvious Area						
	237,972 98 65.12% Impervious Area								
	Tc Length in) (feet)	Slope Velocity (ft/ft) (ft/sec)	Capacity (cfs)	Description					
<u> </u>	5.0		(010)	Direct Entry,					
			Subcatch	ment 20S: Basin 21					
		·	Hydro						
	-			a					
	4.20 cfs			- Runoff					
	4			Type IA 24-hr					
	-								
	-			2-Year Rainfall=2.80"					
	-			Runoff Area=365,464 sf					
	3			Runoff Volume=62,658 cf					
cfs)	-								
Flow (cfs)	-			Runoff Depth=2.06"					
Flo	2			Tc=5.0 min					
	-			CN=80/98					
	1 1			CIN-00/30					
	-								
	0								
	0 5 10 1	5 20 25 30 35 40		0 65 70 75 80 85 90 95 100 105 110 115 120 125 130 ne (hours)					

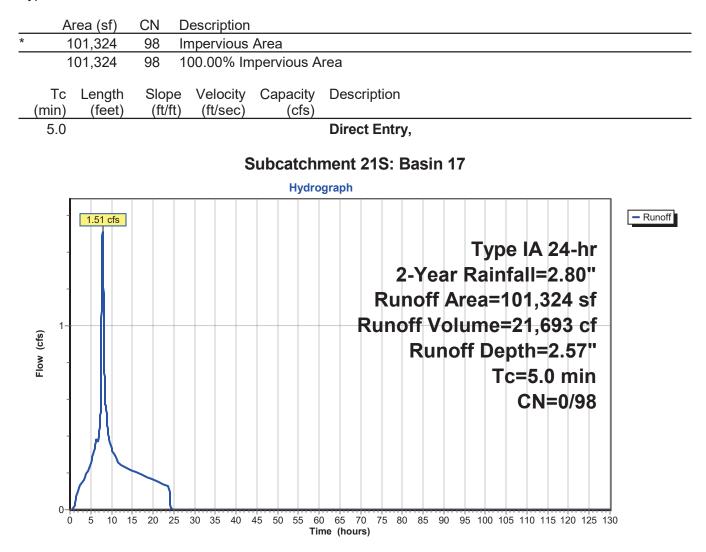
Summary for Subcatchment 21: Basin 21

Runoff = 4.20 cfs @ 7.90 hrs, Volume= 62,658 cf, Depth= 2.06"

	Area (sf)		escription		
	237,972 127,492		npervious ervious	Area	
	365,464 127,492 237,972	92 V 80 3	/eighted A 4.88% Per	verage vious Area pervious Are	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	
5.0					Direct Entry,
			:	Subcatch	hment 21: Basin 21
	6			Hydro	ograph
4- 3-	4.20 cfs				Type IA 24-hr 2-Year Rainfall=2.80" Runoff Area=365,464 sf
-2 Elow (cts)	-				Runoff Volume=62,658 cf Runoff Depth=2.06" Tc=5.0 min
1-					CN=80/98
Ũ	0 5 10 15	5 20 25	30 35 40		60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 Time (hours)

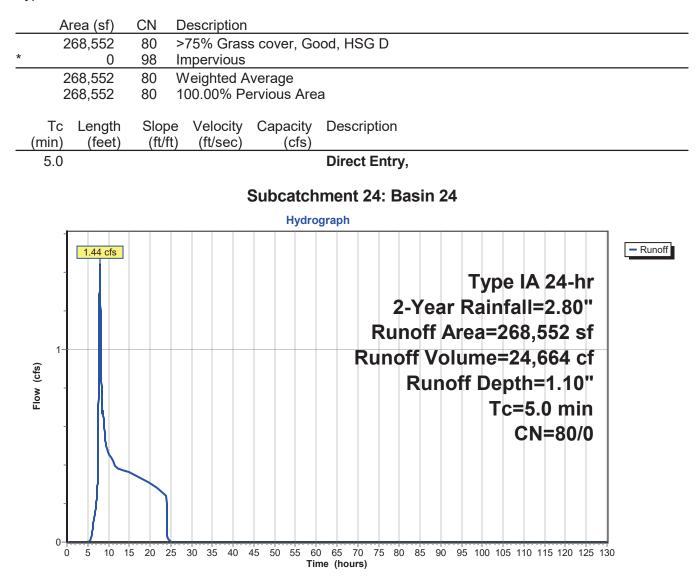
Summary for Subcatchment 21S: Basin 17

Runoff = 1.51 cfs @ 7.88 hrs, Volume= 21,693 cf, Depth= 2.57"



Summary for Subcatchment 24: Basin 24

Runoff = 1.44 cfs @ 8.00 hrs, Volume= 24,664 cf, Depth= 1.10"



Summary for Reach D5: Ditch 5

 Inflow Area =
 33,843 sf, 12.50% Impervious, Inflow Depth >665.11" for 2-Year event

 Inflow =
 4.22 cfs @
 7.98 hrs, Volume=
 1,875,769 cf, Incl. 4.00 cfs Base Flow

 Outflow =
 4.22 cfs @
 8.00 hrs, Volume=
 1,874,939 cf, Atten= 0%, Lag= 1.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 2.01 fps, Min. Travel Time= 3.4 min Avg. Velocity = 1.98 fps, Avg. Travel Time= 3.5 min

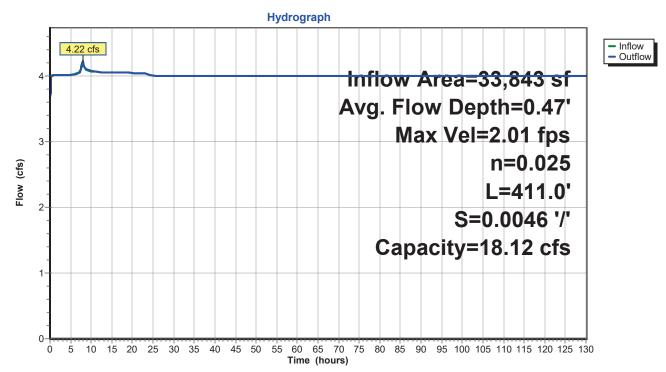
Peak Storage= 862 cf @ 8.00 hrs Average Depth at Peak Storage= 0.47' Bank-Full Depth= 1.00' Flow Area= 6.0 sf, Capacity= 18.12 cfs

Custom cross-section, Length= 411.0' Slope= 0.0046 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 693.07', Outlet Invert= 691.16'

‡

	Offset (feet)	Elevation (feet)	Chan.Depth (feet)		
-					
	-4.50	1.00	0.00		
	-1.50	0.00	1.00		
	1.50	0.00	1.00		
	4.50	1.00	0.00		
	Depth End	Area Pe	erim.	Storage	Discharge
_	(feet)	(sq-ft) (t	feet) (cub	pic-feet)	(cfs)

(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	3.0	0	0.00
1.00	6.0	9.3	2,466	18.12



Reach D5: Ditch 5

Summary for Reach D6: Ditch 6

 Inflow Area =
 268,552 sf, 0.00% Impervious, Inflow Depth = 1.10" for 2-Year event

 Inflow =
 1.44 cfs @
 8.00 hrs, Volume=
 24,664 cf

 Outflow =
 1.36 cfs @
 8.02 hrs, Volume=
 24,664 cf, Atten= 5%, Lag= 1.6 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 2.29 fps, Min. Travel Time= 8.0 min Avg. Velocity = 1.18 fps, Avg. Travel Time= 15.4 min

Peak Storage= 652 cf @ 8.02 hrs Average Depth at Peak Storage= 0.22' Bank-Full Depth= 1.00' Flow Area= 5.0 sf, Capacity= 26.07 cfs

Custom cross-section, Length= 1,095.0' Slope= 0.0152 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 733.54', Outlet Invert= 716.92'

‡

	Offset (feet)	Elevation (feet)	Chan.Depth (feet)	
-	-4.00	1.00	0.00	
	-1.00	0.00	1.00	
	1.00	0.00	1.00	
	4.00	1.00	0.00	
	Depth End			Storage

Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	2.0	0	0.00
1.00	5.0	8.3	5,475	26.07

Hydrograph 1.44 cfs Inflow Outflow 1.36 cfs Inflow Area=268,552 sf Avg. Flow Depth=0.22' Max Vel=2.29 fps 1 n=0.025 Flow (cfs) L=1,095.0' S=0.0152 '/' Capacity=26.07 cfs 0 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 5 Ó Time (hours)

Reach D6: Ditch 6

Summary for Pond Overflow: Overflow Basins

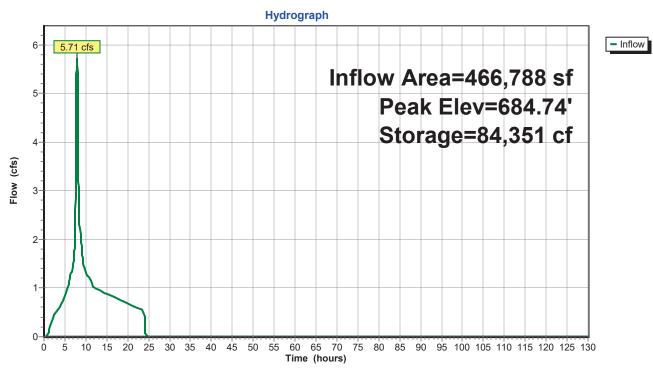
Inflow Are	a =	466,788 sf,	72.69% Impervious,	Inflow Depth = 2.17"	for 2-Year event
Inflow	=	5.71 cfs @	7.90 hrs, Volume=	84,351 cf	
Outflow	=	0.00 cfs @	0.00 hrs, Volume=	0 cf, Atter	n= 100%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 684.74' @ 26.13 hrs Surf.Area= 130,029 sf Storage= 84,351 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	684.00'	760,666 cf	South Basin (Prismatic) Listed below (Recalc)
#2	684.00'	806,919 cf	North Basin (Prismatic) Listed below (Recalc)
		1,567,585 cf	Total Available Storage

Elevation (feet)			Cum.Store (cubic-feet)
684.00	29,903	0	0
685.00	69,069	49,486	49,486
686.00	73,956	71,513	120,999
687.00	78,710	76,333	197,332
688.00	83,799	81,255	278,586
689.00	88,812	86,306	364,892
690.00	93,849	91,331	456,222
691.00	99,018	96,434	552,656
692.00	104,068	101,543	654,199
693.00	108,867	106,468	760,666
El su sti su	Our of America	luce Oterre	Ourse Otherse
Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
(feet) 684.00	<u>(sq-ft)</u> 66,768	(cubic-feet) 0	(cubic-feet) 0
(feet) 684.00 685.00	(sq-ft) 66,768 72,428	(cubic-feet) 0 69,598	(cubic-feet) 0 69,598
(feet) 684.00 685.00 686.00	(sq-ft) 66,768 72,428 77,062	(cubic-feet) 0 69,598 74,745	(cubic-feet) 0 69,598 144,343
(feet) 684.00 685.00 686.00 687.00	(sq-ft) 66,768 72,428 77,062 81,957	(cubic-feet) 0 69,598 74,745 79,510	(cubic-feet) 0 69,598 144,343 223,853
(feet) 684.00 685.00 686.00 687.00 688.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936	(cubic-feet) 0 69,598 74,745 79,510 84,447	(cubic-feet) 0 69,598 144,343 223,853 308,299
(feet) 684.00 685.00 686.00 687.00 688.00 689.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936 91,978	(cubic-feet) 0 69,598 74,745 79,510 84,447 89,457	(cubic-feet) 0 69,598 144,343 223,853 308,299 397,756
(feet) 684.00 685.00 686.00 687.00 688.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936	(cubic-feet) 0 69,598 74,745 79,510 84,447	(cubic-feet) 0 69,598 144,343 223,853 308,299 397,756 492,301
(feet) 684.00 685.00 686.00 687.00 688.00 689.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936 91,978	(cubic-feet) 0 69,598 74,745 79,510 84,447 89,457	(cubic-feet) 0 69,598 144,343 223,853 308,299 397,756
(feet) 684.00 685.00 686.00 687.00 688.00 689.00 690.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936 91,978 97,111	(cubic-feet) 0 69,598 74,745 79,510 84,447 89,457 94,545	(cubic-feet) 0 69,598 144,343 223,853 308,299 397,756 492,301



Pond Overflow: Overflow Basins

Summary for Pond Pond E: Pond E

Inflow Area =		580,537 sf,	6.24% Impervious,	Inflow Depth = 1.19" for 2-Year event
Inflow	=	3.35 cfs @	8.00 hrs, Volume=	57,743 cf
Outflow	=	1.29 cfs @	9.15 hrs, Volume=	57,677 cf, Atten= 61%, Lag= 69.1 min
Primary	=	1.29 cfs @	9.15 hrs, Volume=	57,677 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 713.95' @ 9.15 hrs Surf.Area= 9,456 sf Storage= 8,235 cf

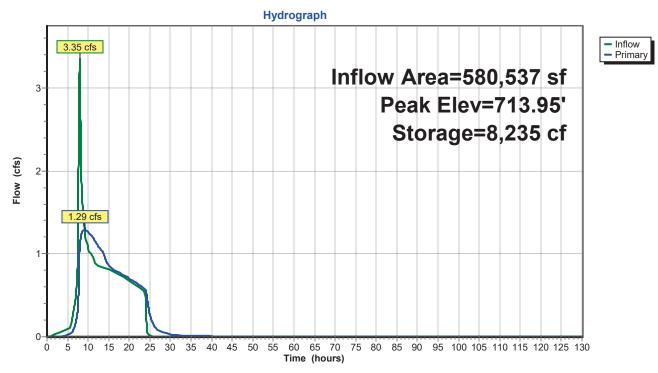
Plug-Flow detention time= 109.6 min calculated for 57,677 cf (100% of inflow) Center-of-Mass det. time= 108.8 min (927.7 - 819.0)

Volume	Inve	rt Avail.Sto	prage Storage Description		
#1	713.0	0' 31,30	08 cf Custon	n Stage Data (Pri	smatic) Listed below (Recalc)
Elevatio	on s	Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
713.0	00	7,916	0	0	
714.0	00	9,540	8,728	8,728	
715.0	00	11,265	10,403	19,131	
716.0)0	13,090	12,178	31,308	
Device	Routing	Invert	Outlet Device	es	
#1	Primary	713.00'	15.0" Round	d Culvert	
	-		L= 44.0' CN	IP, square edge l	neadwall, Ke= 0.500
			Inlet / Outlet	Invert= 713.00' /	712.80' S= 0.0045 '/' Cc= 0.900
			n= 0.013, Flo	ow Area= 1.23 sf	
#2	Device 1	711.00'	7.0" Horiz. O	rifice/Grate C=	0.620 Limited to weir flow at low heads
#3	Device 1	713.95'	16.0" W x 3.0)" H Vert. Orifice	/Grate C= 0.620
Primary	OutFlow	Max=1.29 cfs (2) 9.15 hrs HV	V=713.95' (Free	Discharge)

-1=Culvert (Passes 1.29 cfs of 2.45 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.29 cfs @ 4.84 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

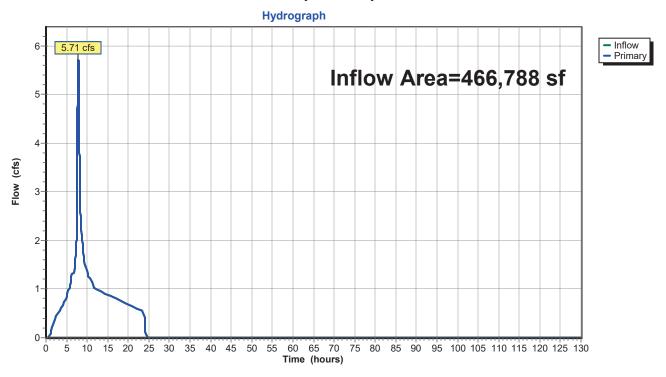


Pond Pond E: Pond E

Summary for Link Pipe 27: Pipe 27

Inflow Area =		466,788 sf,	72.69% Impervious,	Inflow Depth = 2.17"	for 2-Year event
Inflow	=	5.71 cfs @	7.90 hrs, Volume=	84,351 cf	
Primary	=	5.71 cfs @	7.90 hrs, Volume=	84,351 cf, Atter	n= 0%, Lag= 0.0 min

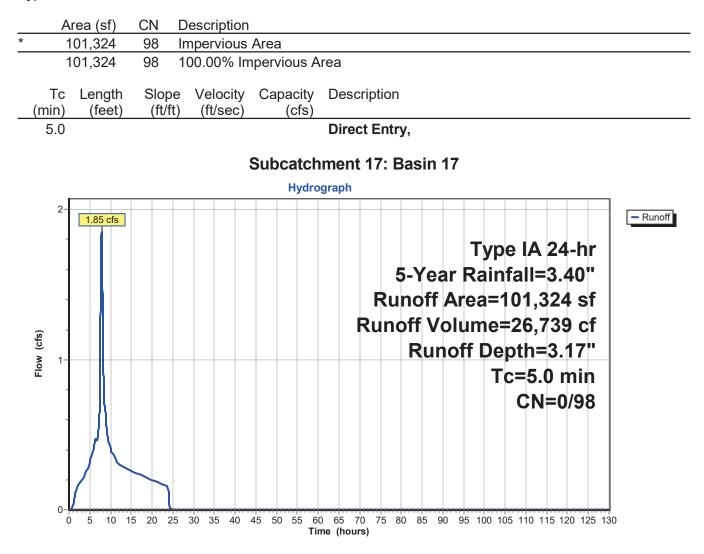
Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs



Link Pipe 27: Pipe 27

Summary for Subcatchment 17: Basin 17

Runoff = 1.85 cfs @ 7.88 hrs, Volume= 26,739 cf, Depth= 3.17"



Summary for Subcatchment 19: Basin 19

Runoff = 0.31 cfs @ 7.96 hrs, Volume= 4,960 cf, Depth= 1.76"

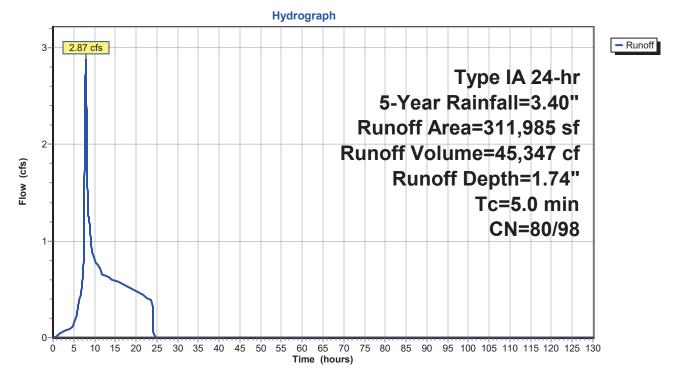
	Area (sf)	CN	Description		
*	4,230		Impervious	Area	
*	29,613		Pervious		
	33,843		Weighted A		
	29,613		87.50% Per		
	4,230	98	12.50% Imp	pervious Are	rea
T (mir	0	Slope (ft/ft)		Capacity (cfs)	Description
5.	/ (/	(1011)	(18300)	(003)	Direct Entry,
				Subcatch	hment 19: Basin 19
					ograph
0.	.34				
0.	.32 0.31 cfs				
	0.3				Type IA 24-hr
	.28				5-Year Rainfall=3.40"
	.24				Runoff Area=33,843 sf
0.	.22				
cfs)	0.2				Runoff Volume=4,960 cf
2	.18				Runoff Depth=1.76"
_	.14				Tc=5.0 min
0.	.12				CN=80/98
(0.1				CIN-00/90
	.08				
	.06	\mathbf{i}			
	.04				
0.	0				
		15 20 2	5 30 35 40		60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 Fime (hours)

Summary for Subcatchment 20: Basin 20

Runoff = 2.87 cfs @ 7.96 hrs, Volume= 45,347 cf, Depth= 1.74"

_	Area (sf)	CN	Description		
*	19,160	98	Impervious	Area	
	275,776	80	>75% Gras	s cover, Go	ood, HSG D
*	17,049	98			
	311,985	82	Weighted A	verage	
	275,776	80	88.39% Per	vious Area	3
	36,209	36,209 98 11.61% Impervious Ar			rea
	To Longth	Slov	na Valacity	Conocity	Description
	Tc Length (min) (feet)	Slop (ft/		Capacity (cfs)	Description
_		(11/	it) (it/sec)	(CIS)	
	5.0				Direct Entry,

Subcatchment 20: Basin 20



Summary for Subcatchment 20S: Basin 21

Runoff = 5.35 cfs @ 7.90 hrs, Volume= 79,347 cf, Depth= 2.61"

* 4	Area (sf) 237,972 127,492 365,464 127,492 237,972 Length (feet)	98 li 80 F 92 V 80 3	5.12% Imp	Area	rea
			5	Subcatch	ment 20S: Basin 21
					ograph
	5.35 cfs	5 20 25	30 35 40		Type IA 24-hr 5-Year Rainfall=3.40" Runoff Area=365,464 sf Runoff Volume=79,347 cf Runoff Depth=2.61" Tc=5.0 min CN=80/98

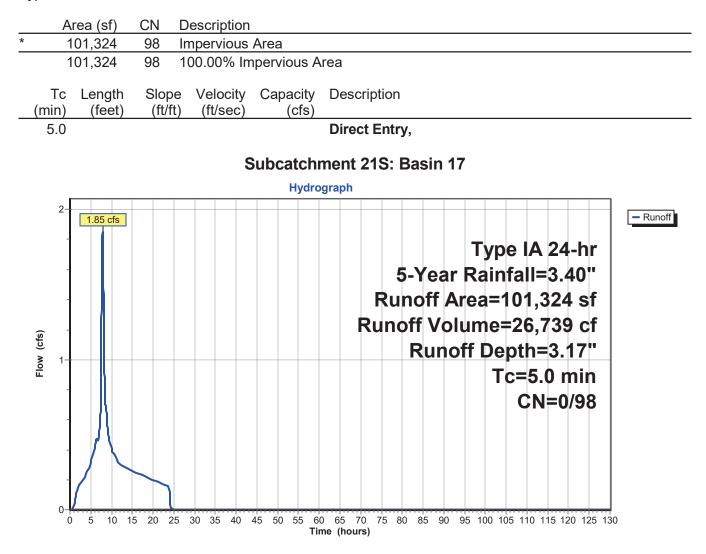
Summary for Subcatchment 21: Basin 21

Runoff = 5.35 cfs @ 7.90 hrs, Volume= 79,347 cf, Depth= 2.61"

Area (sf) CN Description	
* 237,972 98 Impervious Area	
<u>* 127,492 80 Pervious</u> 365,464 92 Weighted Average	
127,492 80 34.88% Pervious Area	
237,972 98 65.12% Impervious Area	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
5.0 Direct Entry,	
Subcatchment 21: Basin 21	
Hydrograph	
5.35 cfs	- Runoff
5 Type IA 24-h	r
5-Year Rainfall=3.40	
4 Runoff Area=365,464 s	
Runoff Volume=79,347 c	
Runoff Depth=2.61	
2-] CN=80/9	8
0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 1 Time (hours)	25 130

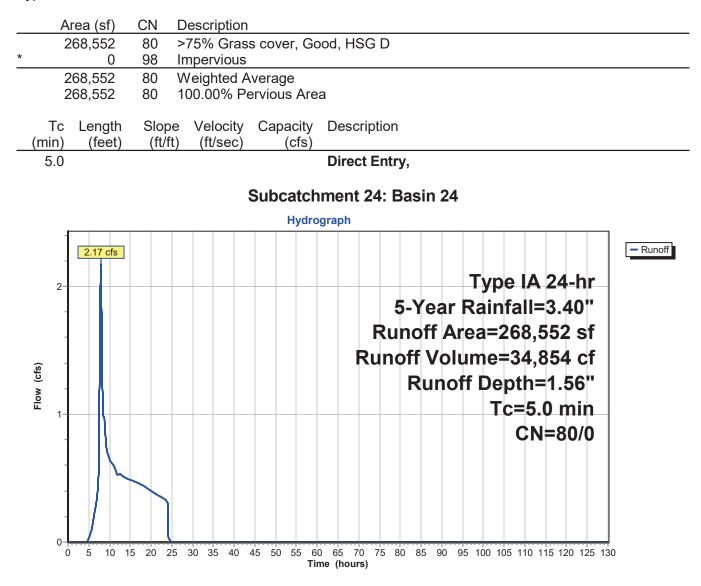
Summary for Subcatchment 21S: Basin 17

Runoff = 1.85 cfs @ 7.88 hrs, Volume= 26,739 cf, Depth= 3.17"



Summary for Subcatchment 24: Basin 24

Runoff = 2.17 cfs @ 7.98 hrs, Volume= 34,854 cf, Depth= 1.56"



Summary for Reach D5: Ditch 5

 Inflow Area =
 33,843 sf,
 12.50% Impervious,
 Inflow Depth >665.58"
 for
 5-Year event

 Inflow =
 4.31 cfs @
 7.96 hrs,
 Volume=
 1,877,104 cf,
 Incl. 4.00 cfs
 Base Flow

 Outflow =
 4.31 cfs @
 8.00 hrs,
 Volume=
 1,876,273 cf,
 Atten= 0%,
 Lag= 2.1 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 2.02 fps, Min. Travel Time= 3.4 min Avg. Velocity = 1.98 fps, Avg. Travel Time= 3.5 min

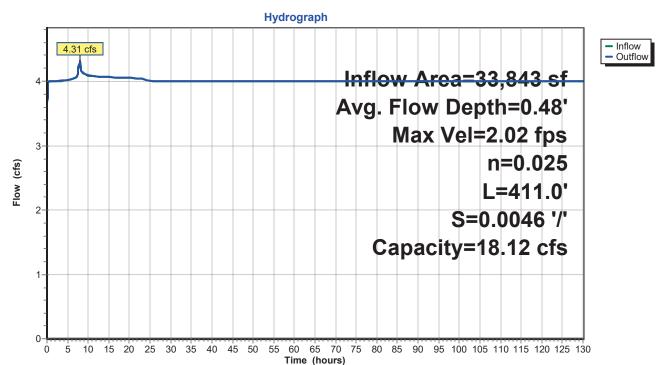
Peak Storage= 876 cf @ 8.00 hrs Average Depth at Peak Storage= 0.48' Bank-Full Depth= 1.00' Flow Area= 6.0 sf, Capacity= 18.12 cfs

Custom cross-section, Length= 411.0' Slope= 0.0046 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 693.07', Outlet Invert= 691.16'

‡

	Offset (feet)	Elevation (feet)	Chan.Depth (feet)		
-	-4.50	1.00	0.00		
	-1.50	0.00	1.00		
	1.50	0.00	1.00		
	4.50	1.00	0.00		
_	Depth End (feet)			Storage bic-feet)	Discharge (cfs)

(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	3.0	0	0.00
1.00	6.0	9.3	2,466	18.12



Reach D5: Ditch 5

Summary for Reach D6: Ditch 6

 Inflow Area =
 268,552 sf,
 0.00% Impervious, Inflow Depth =
 1.56" for 5-Year event

 Inflow =
 2.17 cfs @
 7.98 hrs, Volume=
 34,854 cf

 Outflow =
 2.10 cfs @
 8.01 hrs, Volume=
 34,854 cf,

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 2.61 fps, Min. Travel Time= 7.0 min Avg. Velocity = 1.31 fps, Avg. Travel Time= 14.0 min

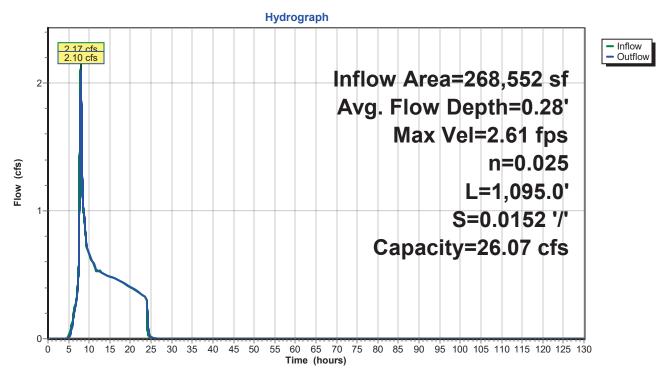
Peak Storage= 883 cf @ 8.01 hrs Average Depth at Peak Storage= 0.28' Bank-Full Depth= 1.00' Flow Area= 5.0 sf, Capacity= 26.07 cfs

Custom cross-section, Length= 1,095.0' Slope= 0.0152 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 733.54', Outlet Invert= 716.92'

‡

Offset (feet)	Elevation (feet)	Chan.Depth (feet)	
-4.00	1.00	0.00	
-1.00	0.00	1.00	
1.00	0.00	1.00	
4.00	1.00	0.00	
Depth End	Area Pe	erim. S	Stora

	Depth	End Area	Perim.	Storage	Discharge	
_	(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)	
	0.00	0.0	2.0	0	0.00	
	1.00	5.0	8.3	5,475	26.07	
	0.00	0.0		0 5,475	0.00	



Reach D6: Ditch 6

Summary for Pond Overflow: Overflow Basins

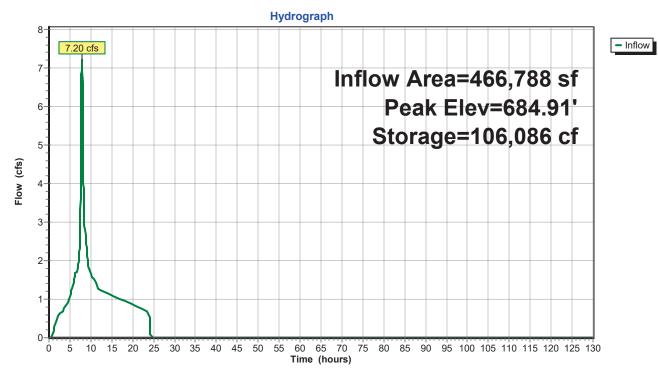
Inflow Are	a =	466,788 sf,	72.69% Impervious,	Inflow Depth = 2.73"	for 5-Year event
Inflow	=	7.20 cfs @	7.89 hrs, Volume=	106,086 cf	
Outflow	=	0.00 cfs @	0.00 hrs, Volume=	0 cf, Atter	n= 100%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 684.91' @ 26.07 hrs Surf.Area= 137,318 sf Storage= 106,086 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	684.00'	760,666 cf	South Basin (Prismatic) Listed below (Recalc)
#2	684.00'	806,919 cf	North Basin (Prismatic) Listed below (Recalc)
		1,567,585 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
/			
684.00	29,903	0	0
685.00	69,069	49,486	49,486
686.00	73,956	71,513	120,999
687.00	78,710	76,333	197,332
688.00	83,799	81,255	278,586
689.00	88,812	86,306	364,892
690.00	93,849	91,331	456,222
691.00	99,018	96,434	552,656
692.00	104,068	101,543	654,199
693.00	108,867	106,468	760,666
	,	,	,
Elevation	Surf.Area	Inc.Store	Cum.Store
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
(feet)	(sq-ft)	(cubic-feet)	
(feet) 684.00	(sq-ft) 66,768 72,428	(cubic-feet) 0	(cubic-feet) 0
(feet) 684.00 685.00	<u>(sq-ft)</u> 66,768 72,428 77,062	(cubic-feet) 0 69,598 74,745	(cubic-feet) 0 69,598 144,343
(feet) 684.00 685.00 686.00 687.00	(sq-ft) 66,768 72,428 77,062 81,957	(cubic-feet) 0 69,598 74,745 79,510	(cubic-feet) 0 69,598 144,343 223,853
(feet) 684.00 685.00 686.00 687.00 688.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936	(cubic-feet) 0 69,598 74,745 79,510 84,447	(cubic-feet) 0 69,598 144,343 223,853 308,299
(feet) 684.00 685.00 686.00 687.00 688.00 688.00 689.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936 91,978	(cubic-feet) 0 69,598 74,745 79,510 84,447 89,457	(cubic-feet) 0 69,598 144,343 223,853 308,299 397,756
(feet) 684.00 685.00 686.00 687.00 688.00 689.00 690.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936 91,978 97,111	(cubic-feet) 0 69,598 74,745 79,510 84,447 89,457 94,545	(cubic-feet) 0 69,598 144,343 223,853 308,299 397,756 492,301
(feet) 684.00 685.00 686.00 687.00 688.00 689.00 690.00 691.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936 91,978 97,111 102,276	(cubic-feet) 0 69,598 74,745 79,510 84,447 89,457 94,545 99,694	(cubic-feet) 0 69,598 144,343 223,853 308,299 397,756 492,301 591,994
(feet) 684.00 685.00 686.00 687.00 688.00 689.00 690.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936 91,978 97,111	(cubic-feet) 0 69,598 74,745 79,510 84,447 89,457 94,545	(cubic-feet) 0 69,598 144,343 223,853 308,299 397,756 492,301



Pond Overflow: Overflow Basins

Summary for Pond Pond E: Pond E

Inflow Area =		580,537 sf,	6.24% Impervious,	Inflow Depth = 1.66" for 5-Year event
Inflow	=	4.96 cfs @	8.00 hrs, Volume=	80,201 cf
Outflow	=	2.21 cfs @	8.73 hrs, Volume=	80,134 cf, Atten= 55%, Lag= 44.2 min
Primary	=	2.21 cfs @	8.73 hrs, Volume=	80,134 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 714.27' @ 8.73 hrs Surf.Area= 10,002 sf Storage= 11,343 cf

Plug-Flow detention time= 104.4 min calculated for 80,127 cf (100% of inflow) Center-of-Mass det. time= 104.2 min (905.1 - 800.9)

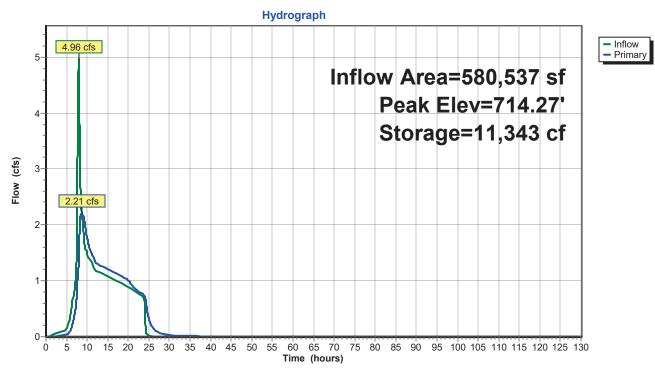
Volume	Inve	ert Avail.Sto	rage Storage	Description				
#1	713.0	0' 31,30	08 cf Custom	Stage Data (Pri	smatic) Listed below (Recalc)			
Elevatio (fee 713.0	et)	Surf.Area (sq-ft) 7,916	Inc.Store (cubic-feet) 0	Cum.Store (cubic-feet) 0				
714.0		9,540	8,728	8,728				
715.0	00	11,265	10,403	19,131				
716.0	00	13,090	12,178	31,308				
Device	Routing	Invert	Outlet Device	S				
#1	Primary	713.00'	15.0" Round	Culvert				
			L= 44.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 713.00' / 712.80' S= 0.0045 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf					
#2	Device 1	711.00'			= 0.620 Limited to weir flow at low heads			
#3	Device 1	713.95'	16.0" W x 3.0	" H Vert. Orifice	/ Grate C= 0.620			
Primary	Primary OutFlow Max=2.21 cfs @ 8.73 hrs $HW=714.27'$ (Free Discharge)							

Primary OutFlow Max=2.21 cfs @ 8.73 hrs HW=714.27' (Free Discharge)

-1=Culvert (Passes 2.21 cfs of 3.81 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.50 cfs @ 5.60 fps)

-3=Orifice/Grate (Orifice Controls 0.71 cfs @ 2.14 fps)

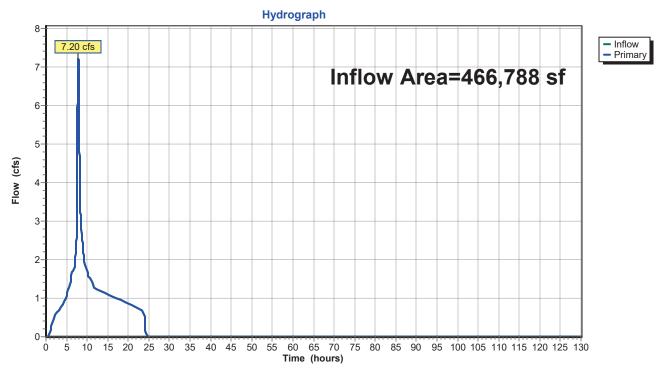


Pond Pond E: Pond E

Summary for Link Pipe 27: Pipe 27

Inflow Area =		466,788 sf,	72.69% Impervious,	Inflow Depth = 2.73"	for 5-Year event
Inflow	=	7.20 cfs @	7.89 hrs, Volume=	106,086 cf	
Primary	=	7.20 cfs @	7.89 hrs, Volume=	106,086 cf, Atte	n= 0%, Lag= 0.0 min

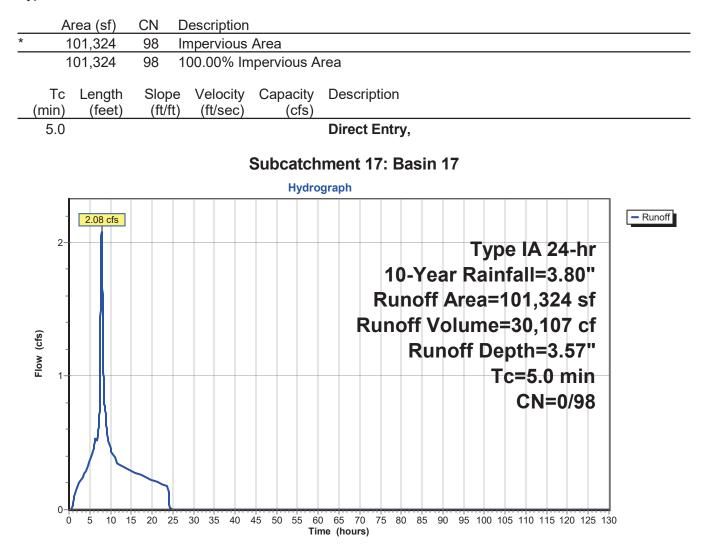
Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs



Link Pipe 27: Pipe 27

Summary for Subcatchment 17: Basin 17

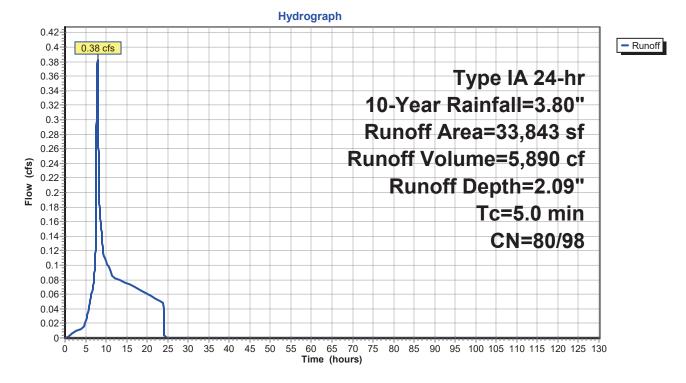
Runoff = 2.08 cfs @ 7.88 hrs, Volume= 30,107 cf, Depth= 3.57"



Summary for Subcatchment 19: Basin 19

Runoff = 0.38 cfs @ 7.95 hrs, Volume= 5,890 cf, Depth= 2.09"

	Area (sf)	CN	Description						
*	4,230	98	Impervious	Area					
*	29,613	80	Pervious						
	33,843	33,843 82 Weighted Average							
	29,613	9,613 80 87.50% Pervious Area							
	4,230	98	3 12.50% Impervious Area						
_				• •					
I	c Length			Capacity	Description				
(mir	ı) (feet)	(ft/1	ft) (ft/sec)	(cfs)					
5.	0				Direct Entry,				
	Subcatchment 19: Basin 19								

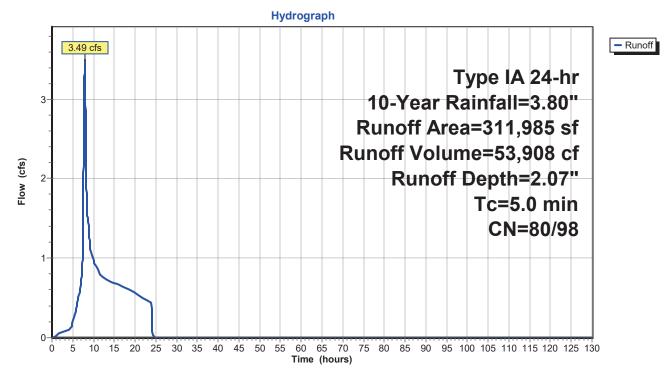


Summary for Subcatchment 20: Basin 20

Runoff = 3.49 cfs @ 7.96 hrs, Volume= 53,908 cf, Depth= 2.07"

_	Area (sf)	CN	Description			
*	19,160	98	Impervious	Area		
	275,776	80	>75% Gras	s cover, Go	ood, HSG D	
*	17,049	98				
	311,985	85 82 Weighted Average				
	275,776	80	88.39% Per	vious Area	3	
	36,209	98	11.61% Impervious Are		rea	
				• •		
	Tc Length	Slo	,	Capacity	Description	
_	(min) (feet)	(ft/	ft) (ft/sec)	(cfs)		
	5.0				Direct Entry,	

Subcatchment 20: Basin 20



Summary for Subcatchment 20S: Basin 21

Runoff = 6.13 cfs @ 7.90 hrs, Volume= 90,658 cf, Depth= 2.98"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Type IA 24-hr 10-Year Rainfall=3.80"

	А	rea (sf)	CN I	Description						
*		37,972		mpervious	Area					
*		27,492		Pervious						
		65,464		Neighted A						
		27,492		34.88% Per						
	2	37,972	98 (65.12% Imp	pervious Ar	ea				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	5.0					Direct Entry,				
				3		ment 20S: Ba	asin 21			
	r				Hydro	graph				1
	1	6.13 cfs								- Runoff
	6-							Type IA	24-hr	
	5-					10-		Rainfall		
	5-					Run	off Are	ea=365,	464 sf	
	-	<u>,</u>								
	(sj: 4-							me=90,		
	Flow (cfs)						Runof	f Depth:	=2.98"	
	₽ 3 ⁻							Tc=5	.0 min	-
									=80/98	
	2-									
	-	1								
	1-									

10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130

Time (hours)

5

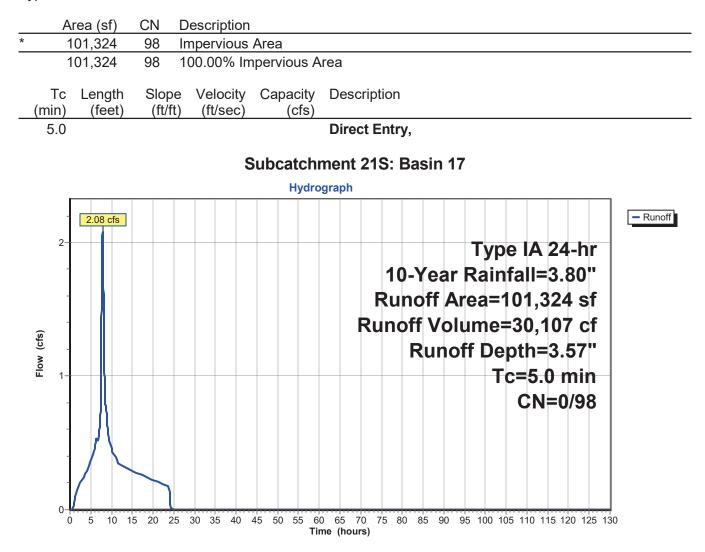
Summary for Subcatchment 21: Basin 21

Runoff = 6.13 cfs @ 7.90 hrs, Volume= 90,658 cf, Depth= 2.98"

*	Area (sf) 237,972 127,492 365,464 127,492 237,972	98 li 80 F 92 V 80 3	5.12% Imp	Area verage rvious Area pervious Are	ea
(mir	/ (/	(ft/ft)	(ft/sec)	Capacity (cfs)	Description
5.	0				Direct Entry,
				Subcatch	nment 21: Basin 21
	-			Hydro	ograph
Flow (cfs)	6.13 cfs 6 5 4 4 3 2 1 0 0 5 10 1	5 20 25	30 35 40		Type IA 24-hr 10-Year Rainfall=3.80" Runoff Area=365,464 sf Runoff Volume=90,658 cf Runoff Depth=2.98" Tc=5.0 min CN=80/98

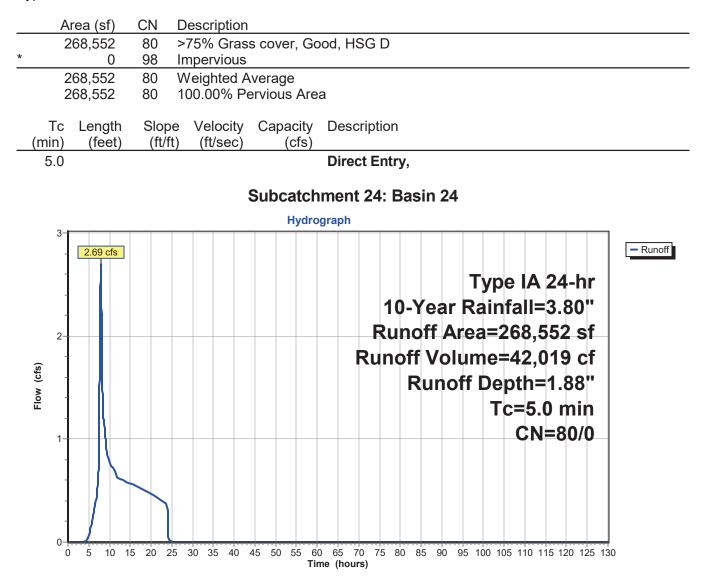
Summary for Subcatchment 21S: Basin 17

Runoff = 2.08 cfs @ 7.88 hrs, Volume= 30,107 cf, Depth= 3.57"



Summary for Subcatchment 24: Basin 24

Runoff = 2.69 cfs @ 7.97 hrs, Volume= 42,019 cf, Depth= 1.88"



Summary for Reach D5: Ditch 5

Discharge

 Inflow Area =
 33,843 sf,
 12.50% Impervious,
 Inflow Depth >665.91"
 for
 10-Year event

 Inflow =
 4.38 cfs @
 7.95 hrs,
 Volume=
 1,878,034 cf,
 Incl. 4.00 cfs
 Base Flow

 Outflow =
 4.38 cfs @
 7.99 hrs,
 Volume=
 1,877,204 cf,
 Atten= 0%,
 Lag= 2.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 2.03 fps, Min. Travel Time= 3.4 min Avg. Velocity = 1.98 fps, Avg. Travel Time= 3.5 min

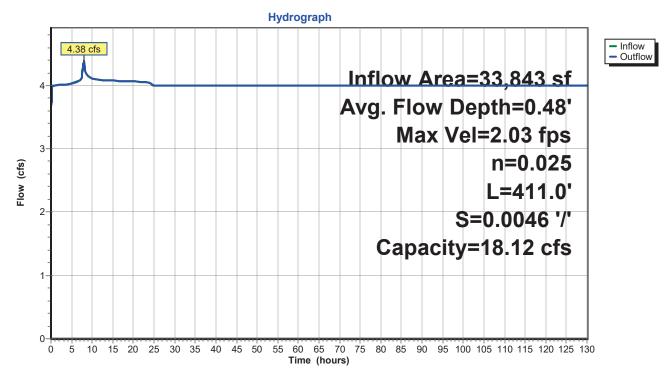
Peak Storage= 885 cf @ 7.99 hrs Average Depth at Peak Storage= 0.48' Bank-Full Depth= 1.00' Flow Area= 6.0 sf, Capacity= 18.12 cfs

Custom cross-section, Length= 411.0' Slope= 0.0046 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 693.07', Outlet Invert= 691.16'

‡

	Offset (feet)	Elevation (feet)	Chan.Depth (feet)	
_	· · · · · ·		· · · · · · · · · · · · · · · · · · ·	
	-4.50	1.00	0.00	
	-1.50	0.00	1.00	
	1.50	0.00	1.00	
	4.50	1.00	0.00	
	Depth End (feet) (Storage bic-feet)

(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	3.0	0	0.00
1.00	6.0	9.3	2,466	18.12



Reach D5: Ditch 5

Summary for Reach D6: Ditch 6

 Inflow Area =
 268,552 sf,
 0.00% Impervious, Inflow Depth =
 1.88" for 10-Year event

 Inflow =
 2.69 cfs @
 7.97 hrs, Volume=
 42,019 cf

 Outflow =
 2.63 cfs @
 8.01 hrs, Volume=
 42,019 cf,

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 2.79 fps, Min. Travel Time= 6.5 min Avg. Velocity = 1.38 fps, Avg. Travel Time= 13.3 min

Peak Storage= 1,033 cf @ 8.01 hrs Average Depth at Peak Storage= 0.32' Bank-Full Depth= 1.00' Flow Area= 5.0 sf, Capacity= 26.07 cfs

Custom cross-section, Length= 1,095.0' Slope= 0.0152 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 733.54', Outlet Invert= 716.92'

‡

Offset (feet)	Elevation (feet)	Chan.Depth (feet)	
-4.00	1.00	0.00	
-1.00	0.00	1.00	
1.00	0.00	1.00	
4.00	1.00	0.00	
Depth End	Area Pe	rim.	Storage

Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	2.0	0	0.00
1.00	5.0	8.3	5,475	26.07

Hydrograph 3 Inflow 2.60 cfs 2.63 cfs - Outflow Inflow Area=268,552 sf Avg. Flow Depth=0.32' Max Vel=2.79 fps 2 n=0.025 Flow (cfs) L=1,095.0' S=0.0152 '/' 1 Capacity=26.07 cfs 0 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 5 Ó Time (hours)

Reach D6: Ditch 6

Summary for Pond Overflow: Overflow Basins

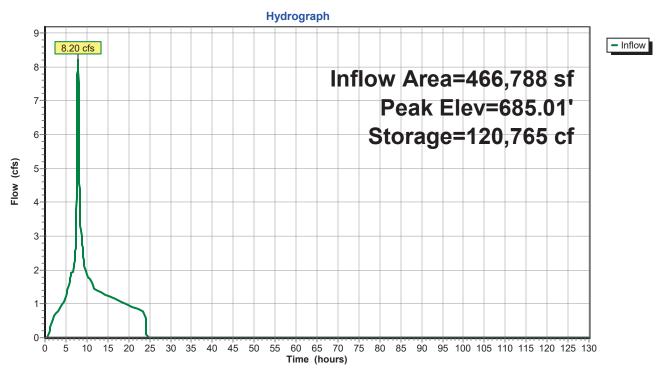
Inflow Are	a =	466,788 sf,	72.69% Impervious,	Inflow Depth = 3.10"	for 10-Year event
Inflow	=	8.20 cfs @	7.89 hrs, Volume=	120,765 cf	
Outflow	=	0.00 cfs @	0.00 hrs, Volume=	0 cf, Atter	n= 100%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 685.01' @ 25.99 hrs Surf.Area= 141,610 sf Storage= 120,765 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	684.00'	760,666 cf	South Basin (Prismatic) Listed below (Recalc)
#2	684.00'	806,919 cf	North Basin (Prismatic) Listed below (Recalc)
		1,567,585 cf	Total Available Storage

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
684.00	29,903	0	0
685.00	69,069	49,486	49,486
686.00	73,956	71,513	120,999
687.00	78,710	76,333	197,332
688.00	83,799	81,255	278,586
689.00	88,812	86,306	364,892
690.00	93,849	91,331	456,222
691.00	99,018	96,434	552,656
692.00	104,068	101,543	654,199
693.00	108,867	106,468	760,666
Elevation	Surf.Area	Inc.Store	Cum.Store
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
(feet)	(sq-ft)		
(feet) 684.00	<u>(sq-ft)</u> 66,768	(cubic-feet) 0	(cubic-feet) 0
(feet) 684.00 685.00	(sq-ft) 66,768 72,428	(cubic-feet) 0 69,598	(cubic-feet) 0 69,598
(feet) 684.00 685.00 686.00 687.00 688.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936	(cubic-feet) 0 69,598 74,745 79,510 84,447	(cubic-feet) 0 69,598 144,343 223,853 308,299
(feet) 684.00 685.00 686.00 687.00 688.00 689.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936 91,978	(cubic-feet) 0 69,598 74,745 79,510	(cubic-feet) 0 69,598 144,343 223,853 308,299 397,756
(feet) 684.00 685.00 686.00 687.00 688.00 689.00 690.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936 91,978 97,111	(cubic-feet) 0 69,598 74,745 79,510 84,447 89,457 94,545	(cubic-feet) 0 69,598 144,343 223,853 308,299 397,756 492,301
(feet) 684.00 685.00 686.00 687.00 688.00 689.00 690.00 691.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936 91,978 97,111 102,276	(cubic-feet) 0 69,598 74,745 79,510 84,447 89,457 94,545 99,694	(cubic-feet) 0 69,598 144,343 223,853 308,299 397,756 492,301 591,994
(feet) 684.00 685.00 686.00 687.00 688.00 689.00 690.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936 91,978 97,111	(cubic-feet) 0 69,598 74,745 79,510 84,447 89,457 94,545	(cubic-feet) 0 69,598 144,343 223,853 308,299 397,756 492,301



Pond Overflow: Overflow Basins

Summary for Pond Pond E: Pond E

Inflow Area =	=	580,537 sf,	6.24% Impervious,	Inflow Depth = 1.98" for 10-Year	event
Inflow =	:	6.10 cfs @	8.00 hrs, Volume=	95,927 cf	
Outflow =	:	2.70 cfs @	8.69 hrs, Volume=	95,860 cf, Atten= 56%, Lag-	= 41.2 min
Primary =	:	2.70 cfs @	8.69 hrs, Volume=	95,860 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 714.50' @ 8.69 hrs Surf.Area= 10,400 sf Storage= 13,700 cf

Plug-Flow detention time= 103.9 min calculated for 95,860 cf (100% of inflow) Center-of-Mass det. time= 103.4 min (894.4 - 791.0)

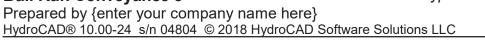
Volume	Inve	ert Avail.Sto	rage Storage	Description	
#1	713.0	0' 31,30	08 cf Custom	Stage Data (Pris	smatic) Listed below (Recalc)
Elevatio	et)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
713.0		7,916	0	0	
714.(715.(9,540 11,265	8,728 10,403	8,728 19,131	
716.0	00	13,090	12,178	31,308	
Device	Routing	Invert	Outlet Device	S	
#1	Primary	713.00'	15.0" Round	Culvert	
				eadwall, Ke= 0.500 '12.80' S= 0.0045 '/' Cc= 0.900	
#2	Device 1	711.00'	7.0" Horiz. Or	rifice/Grate C=	0.620 Limited to weir flow at low heads
#3	Device 1	713.95'	16.0" W x 3.0	" H Vert. Orifice/	Grate C= 0.620
Primary		Max=2.70 cfs (@ 8 69 hrs H\W	/=71/ 50' (Free	Discharge)

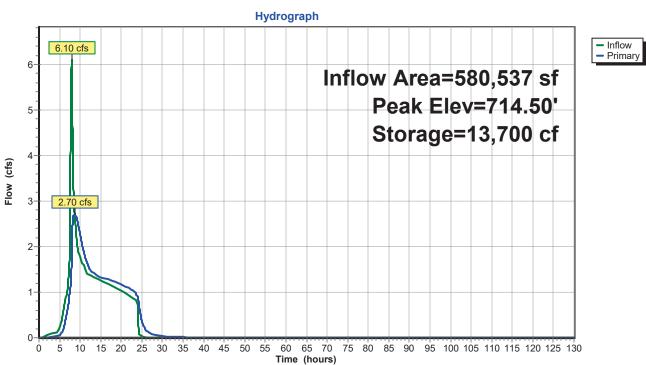
Primary OutFlow Max=2.70 cfs @ 8.69 hrs HW=714.50' (Free Discharge)

—1=Culvert (Passes 2.70 cfs of 4.65 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.63 cfs @ 6.09 fps)

-3=Orifice/Grate (Orifice Controls 1.08 cfs @ 3.23 fps)



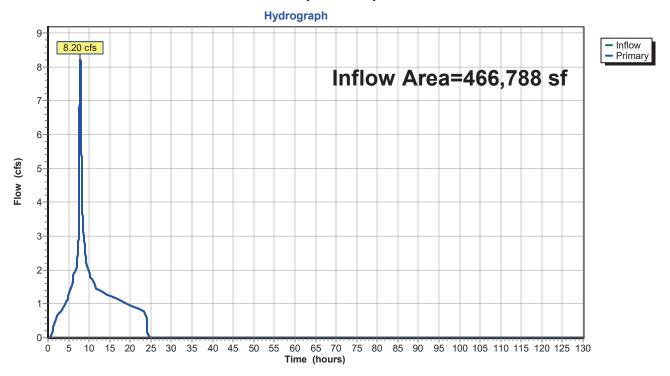


Pond Pond E: Pond E

Summary for Link Pipe 27: Pipe 27

Inflow Area =		466,788 sf,	72.69% Impervious,	Inflow Depth = 3.10"	for 10-Year event
Inflow	=	8.20 cfs @	7.89 hrs, Volume=	120,765 cf	
Primary	=	8.20 cfs @	7.89 hrs, Volume=	120,765 cf, Atter	n= 0%, Lag= 0.0 min

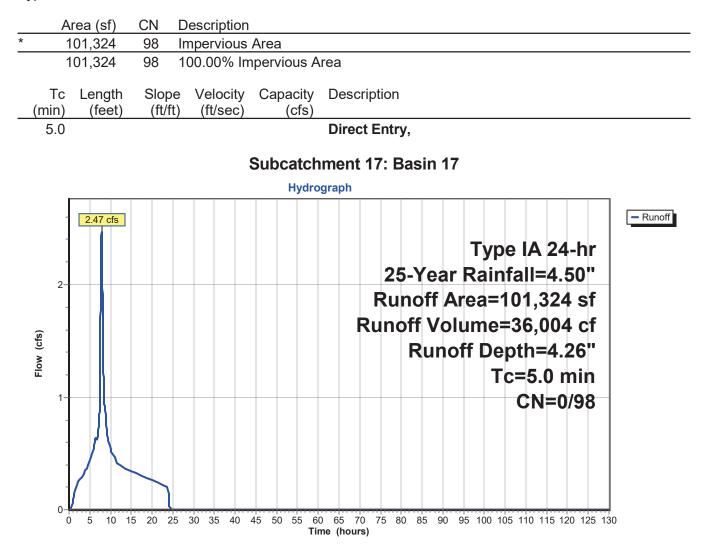
Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs



Link Pipe 27: Pipe 27

Summary for Subcatchment 17: Basin 17

Runoff = 2.47 cfs @ 7.88 hrs, Volume= 36,004 cf, Depth= 4.26"



Summary for Subcatchment 19: Basin 19

Runoff = 0.50 cfs @ 7.94 hrs, Volume= 7,578 cf, Depth= 2.69"

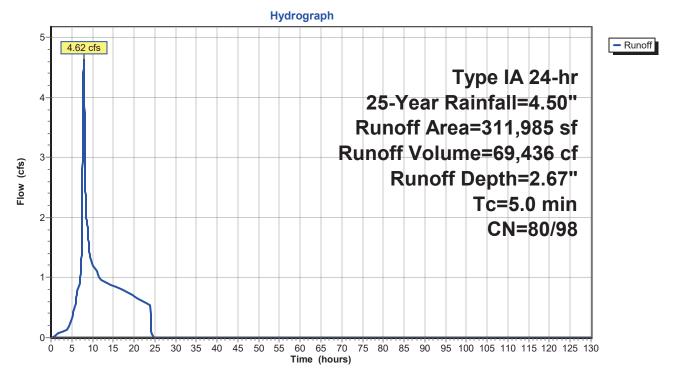
	A	rea (sf)		Description										
		4,230		Impervious	Area									
		29,613		Pervious										
		33,843 29,613		Weighted A 87.50% Pei										
		4,230		12.50% Imp										
	_													
1	Tc min)	Length	Slope			Desc	ription							
(<u>min)</u> 5.0	(feet)	(ft/ft)	(II/Sec)	(cfs)	Direc	t Entr							
	0.0					Direc		y ,						
					Subcatch	ment	19: E	Basir	า 19					
					Hydro	graph								-
	0.55	0.50 cfs												- Runof
	0.5												_ • •	
	0.45-									l y	pe I	A 24	1-hr	
							25	-Ye	ar I	Rai	nfal	l l=4 .	50"	
	0.4-						Ru	not	fΔ	rea	n=3 :	3 84	3 sf	
	0.35													
(ofc)	0.3						Rune					•		
	0.3 0.3	-						Rur	າof	f D	eptl	h=2.	69"	
L	L 0.25										Tc=	5.0	min	
	0.2-											1=8(1/98	
	0.15	<u> </u>										1-01	,,30	
	0.1-							-						ang
	0.05													
	0-	0 5 10	15 20 2	5 30 35 40	45 50 55 6	60 65 7	 • • • • • • • • • • •	<u> </u>						-

Summary for Subcatchment 20: Basin 20

Runoff = 4.62 cfs @ 7.94 hrs, Volume= 69,436 cf, Depth= 2.67"

_	Area (sf)	CN	Description					
*	19,160	98	Impervious /	Impervious Area				
	275,776	80	>75% Grass	cover, Go	ood, HSG D			
*	17,049	98						
	311,985	82	Weighted Av	verage				
	275,776	80	88.39% Per	vious Area	а			
	36,209	98	11.61% Imp	ervious Ar	rea			
	Tc Length (min) (feet)	Slop (ft/	,	Capacity (cfs)				
_	5.0	(14		(0.0)	Direct Entry,			

Subcatchment 20: Basin 20



Summary for Subcatchment 20S: Basin 21

Runoff = 7.51 cfs @ 7.90 hrs, Volume= 110,712 cf, Depth= 3.64"

	Area (sf)	CN [Description						
*	237,972		mpervious						
*	127,492		Pervious	/ 100					
	365,464		Veighted A						
	127,492			rvious Area					
	237,972	98 6	5.12% Imp	pervious Ar	ea				
To (min)	0	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
5.0		(1011)	(10300)	(013)	Direct Entry,				
				Subcatch	ment 20S: B	asin 21			
				Hydro	graph				
8	- - 7.51 cfs								- Runoff
7	-						Гуре IA	24-hr	
i	-				21	5-Year R			
6									
	-					noff Are	-		
5 (s					Runo	ff Volum			
Flow (cfs)	-					Runoff	Depth=	=3.64"	
4 4							Tc=5.	0 min	
3							CN=	80/98	
-	1 N								
2	1 11								
1									
0	0 5 10 1	5 20 25	30 35 40		0 65 70 75 80	85 90 95 1	00 105 110 1	15 120 125 13	60
				Tir	me (hours)				

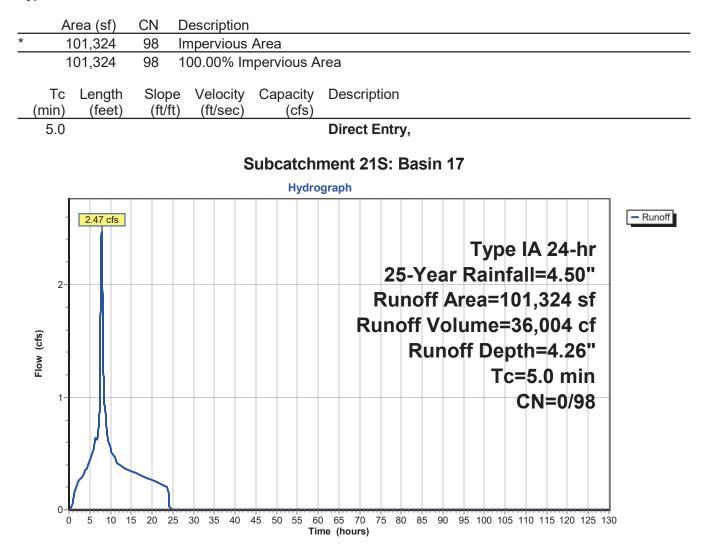
Summary for Subcatchment 21: Basin 21

Runoff = 7.51 cfs @ 7.90 hrs, Volume= 110,712 cf, Depth= 3.64"

1 3 1 2	27,492 37,972	80 92 80 98	34.88% Pe 65.12% Im	Average rvious Area pervious Ar	ea	ription								
nin)				(cfs)										
ວ.ບ					Direc	ct Entr	у,							
						21: E	Basin	21						
r				Hydro	graph					1				
8 - 7 -	7.51 cf	S							-	-				- Runoff
6 - - 5						R	uno	ff Ar	ea=	=365	5,46	64 s	f	
-											•			
4- - 3-									•					
2- 1- 0-														
	1 2 Tc iin) 5.0 8 8 7 6 6 6 6 6 7 6 6 7 6 7 6 7 7 7 7 7	127,492 237,972 Tc Lengt in) (fee 5.0	127,492 80 237,972 98 Tc Length Slop in) (feet) (ft/ 5.0 8 7 6 6 5 4 4 3 2 4 4 4 3 2 4 4 4 4 4 4 4 4 4 4 4	127,492 80 34.88% Pe 237,972 98 65.12% Im Tc Length Slope Velocity in) (feet) (ft/ft) (ft/sec) 5.0	127,492 80 34.88% Pervious Area 237,972 98 65.12% Impervious Ar Tc Length Slope Velocity Capacity (ft/ft) (ft/sec) (cfs) 5.0 Subcatch Hydro	127,492 80 34.88% Pervious Area 237,972 98 65.12% Impervious Area Tc Length Slope Velocity Capacity Desc (ft/ft) (ft/sec) (cfs) 5.0 Direc Subcatchment Hydrograph	127,492 80 34.88% Pervious Area 237,972 98 65.12% Impervious Area Tc Length Slope Velocity Capacity Description (feet) (ft/ft) (ft/sec) (cfs) 5.0 Direct Entr Subcatchment 21: E Hydrograph	127,492 80 34.88% Pervious Area 237,972 98 65.12% Impervious Area Tc Length Slope Velocity Capacity Description (feet) (ft/ft) (ft/sec) (cfs) 5.0 Direct Entry, Subcatchment 21: Basin Hydrograph 7,51 cfs 7 6 7 6 7 7 6 7 7 7 6 7 7 7 7 7 7 7 7 7 7 7 7 7	127,492 80 34.88% Pervious Area 237,972 98 65.12% Impervious Area Tc Length Slope Velocity Capacity Description (feet) (ft/ft) (ft/sec) (cfs) 5.0 Direct Entry, Subcatchment 21: Basin 21 Hydrograph 7.51 cfs 25-Year Runoff Ar Runoff Ar Runoff Volur Runoff	127,492 80 34.88% Pervious Area 237,972 98 65.12% Impervious Area Tc Length Slope Velocity Capacity Description (feet) (ft/ft) (ft/sec) (cfs) 5.0 Direct Entry, Subcatchment 21: Basin 21 Hydrograph 7,51 cfs 7,51 c	127,492 80 34.88% Pervious Area 237,972 98 65.12% Impervious Area Tc Length Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs) 5.0 Direct Entry, Subcatchment 21: Basin 21 Hydrograph 7.51 cfs 75 cfs	127,492 80 34.88% Pervious Area 237,972 98 65.12% Impervious Area Tc Length Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs) 5.0 Direct Entry, Subcatchment 21: Basin 21 Hydrograph 7.51 cfs 7 7.51 cfs 7 7 7 7 7 7 7 7 7 7 7 7 7 7	127,492 80 34.88% Pervious Area 237,972 98 65.12% Impervious Area Tc Length Slope Velocity Capacity Description (feet) (ft/ft) (ft/sec) (cfs) 5.0 Direct Entry, Subcatchment 21: Basin 21 Hydrograph 7.51 cfs 7 Type IA 24-h 25-Year Rainfall=4.50 Runoff Area=365,464 s Runoff Depth=3.64 Tc=5.0 min CN=80/98	127,492 80 34.88% Pervious Area 237,972 98 65.12% Impervious Area Tc Length Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs) 5.0 Direct Entry, Subcatchment 21: Basin 21 Hydrograph 7,51 cfs 7,51 cfs

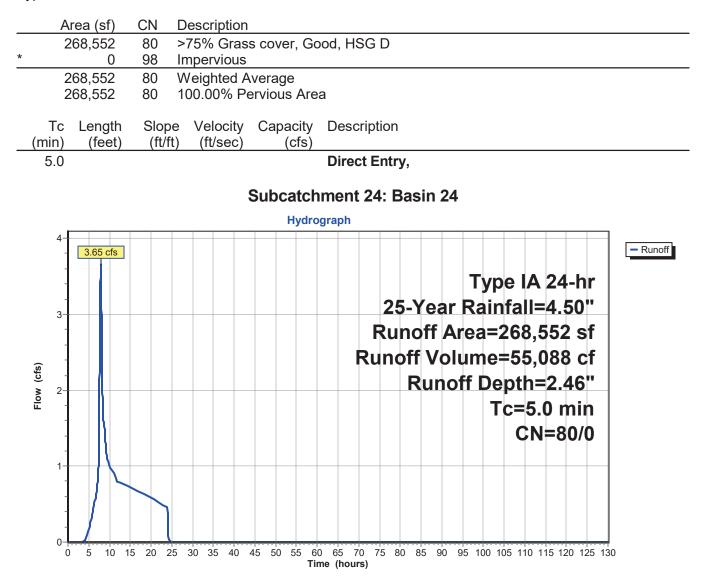
Summary for Subcatchment 21S: Basin 17

Runoff = 2.47 cfs @ 7.88 hrs, Volume= 36,004 cf, Depth= 4.26"



Summary for Subcatchment 24: Basin 24

Runoff = 3.65 cfs @ 7.96 hrs, Volume= 55,088 cf, Depth= 2.46"



Summary for Reach D5: Ditch 5

 Inflow Area =
 33,843 sf, 12.50% Impervious, Inflow Depth >666.51" for 25-Year event

 Inflow =
 4.50 cfs @
 7.94 hrs, Volume=
 1,879,722 cf, Incl. 4.00 cfs Base Flow

 Outflow =
 4.50 cfs @
 7.98 hrs, Volume=
 1,878,891 cf, Atten= 0%, Lag= 2.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 2.05 fps, Min. Travel Time= 3.3 min Avg. Velocity = 1.98 fps, Avg. Travel Time= 3.5 min

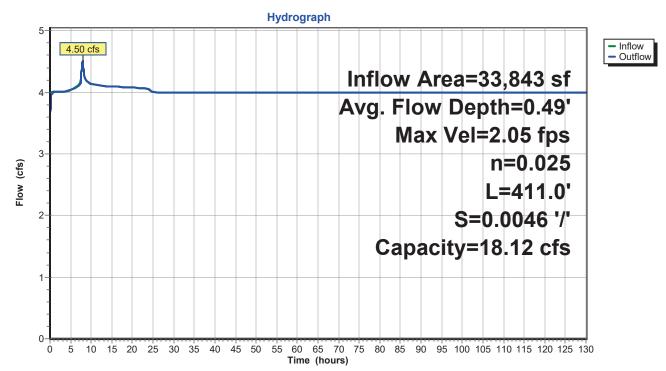
Peak Storage= 903 cf @ 7.98 hrs Average Depth at Peak Storage= 0.49' Bank-Full Depth= 1.00' Flow Area= 6.0 sf, Capacity= 18.12 cfs

Custom cross-section, Length= 411.0' Slope= 0.0046 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 693.07', Outlet Invert= 691.16'

‡

	Offset		Chan.Depth		
_	(feet)	(feet)	(feet)		
	-4.50	1.00	0.00		
	-1.50	0.00	1.00		
	1.50	0.00	1.00		
	4.50	1.00	0.00		
	Depth End	l Area 🛛 Pe	erim.	Storage	Discharge
_	(feet)	(sq-ft) (feet) (cut	pic-feet)	(cfs)

(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	3.0	0	0.00
1.00	6.0	9.3	2,466	18.12



Reach D5: Ditch 5

Summary for Reach D6: Ditch 6

Discharge

 Inflow Area =
 268,552 sf, 0.00% Impervious, Inflow Depth = 2.46" for 25-Year event

 Inflow =
 3.65 cfs @
 7.96 hrs, Volume=
 55,088 cf

 Outflow =
 3.59 cfs @
 8.01 hrs, Volume=
 55,088 cf, Atten= 2%, Lag= 2.9 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Max. Velocity= 3.05 fps, Min. Travel Time= 6.0 min Avg. Velocity = 1.48 fps, Avg. Travel Time= 12.3 min

Peak Storage= 1,290 cf @ 8.01 hrs Average Depth at Peak Storage= 0.38' Bank-Full Depth= 1.00' Flow Area= 5.0 sf, Capacity= 26.07 cfs

Custom cross-section, Length= 1,095.0' Slope= 0.0152 '/' Constant n= 0.025 Earth, clean & straight Inlet Invert= 733.54', Outlet Invert= 716.92'

‡

Offset (feet)	Elevation (feet)	Chan.Depth (feet)	
-4.00	1.00	0.00	
-1.00	0.00	1.00	
1.00	0.00	1.00	
4.00	1.00	0.00	
Depth End			Storage

(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	2.0	0	0.00
1.00	5.0	8.3	5,475	26.07

Hydrograph 4 Inflow 3.59 cfs - Outflow Inflow Area=268,552 sf Avg. Flow Depth=0.38' 3 Max Vel=3.05 fps n=0.025 Flow (cfs) L=1,095.0' 2 S=0.0152 '/' Capacity=26.07 cfs 1 0 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 5 Ó Time (hours)

Reach D6: Ditch 6

Summary for Pond Overflow: Overflow Basins

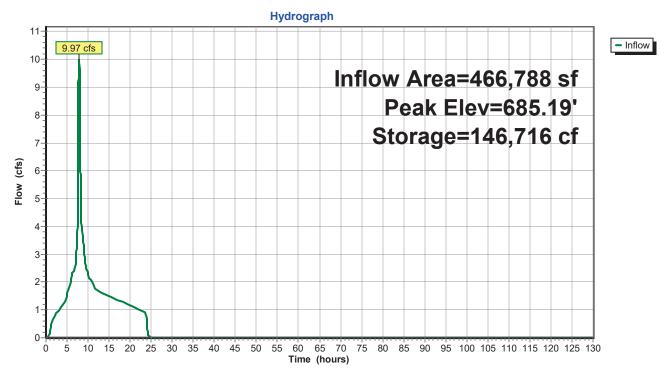
Inflow Are	a =	466,788 sf,	72.69% Impervious,	Inflow Depth = 3.77"	for 25-Year event
Inflow	=	9.97 cfs @	7.89 hrs, Volume=	146,716 cf	
Outflow	=	0.00 cfs @	0.00 hrs, Volume=	0 cf, Atter	n= 100%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 685.19' @ 25.98 hrs Surf.Area= 143,344 sf Storage= 146,716 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	684.00'	760,666 cf	South Basin (Prismatic) Listed below (Recalc)
#2	684.00'	806,919 cf	North Basin (Prismatic) Listed below (Recalc)
		1,567,585 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
684.00	29,903	0	0
685.00	69,069	49,486	49,486
686.00	73,956	71,513	120,999
687.00	78,710	76,333	197,332
688.00	83,799	81,255	278,586
689.00	88,812	86,306	364,892
690.00	93,849	91,331	456,222
691.00	99,018	96,434	552,656
692.00	104,068	101,543	654,199
693.00	108,867	106,468	760,666
Elevation	Surf.Area	Inc.Store	Cum.Store
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
(feet)	(sq-ft)	(cubic-feet)	
(feet) 684.00 685.00 686.00	(sq-ft) 66,768 72,428 77,062	(cubic-feet) 0	(cubic-feet) 0 69,598 144,343
(feet) 684.00 685.00	(sq-ft) 66,768 72,428 77,062 81,957	(cubic-feet) 0 69,598 74,745 79,510	(cubic-feet) 0 69,598 144,343 223,853
(feet) 684.00 685.00 686.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936	(cubic-feet) 0 69,598 74,745 79,510 84,447	(cubic-feet) 0 69,598 144,343 223,853 308,299
(feet) 684.00 685.00 686.00 687.00 688.00 689.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936 91,978	(cubic-feet) 0 69,598 74,745 79,510 84,447 89,457	(cubic-feet) 0 69,598 144,343 223,853 308,299 397,756
(feet) 684.00 685.00 686.00 687.00 688.00 689.00 690.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936 91,978 97,111	(cubic-feet) 0 69,598 74,745 79,510 84,447 89,457 94,545	(cubic-feet) 0 69,598 144,343 223,853 308,299 397,756 492,301
(feet) 684.00 685.00 686.00 687.00 688.00 689.00 690.00 691.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936 91,978 97,111 102,276	(cubic-feet) 0 69,598 74,745 79,510 84,447 89,457 94,545 99,694	(cubic-feet) 0 69,598 144,343 223,853 308,299 397,756 492,301 591,994
(feet) 684.00 685.00 686.00 687.00 688.00 689.00 690.00	(sq-ft) 66,768 72,428 77,062 81,957 86,936 91,978 97,111	(cubic-feet) 0 69,598 74,745 79,510 84,447 89,457 94,545	(cubic-feet) 0 69,598 144,343 223,853 308,299 397,756 492,301



Pond Overflow: Overflow Basins

Summary for Pond Pond E: Pond E

Inflow Area	ı =	580,537 sf,	6.24% Impervious,	Inflow Depth = 2.57" for 25-Year event	
Inflow	=	8.18 cfs @	7.98 hrs, Volume=	124,523 cf	
Outflow	=	3.44 cfs @	8.76 hrs, Volume=	124,456 cf, Atten= 58%, Lag= 46.4 mir	n
Primary	=	3.44 cfs @	8.76 hrs, Volume=	124,456 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs Peak Elev= 714.97' @ 8.76 hrs Surf.Area= 11,215 sf Storage= 18,803 cf

Plug-Flow detention time= 101.2 min calculated for 124,446 cf (100% of inflow) Center-of-Mass det. time= 101.2 min (878.1 - 776.9)

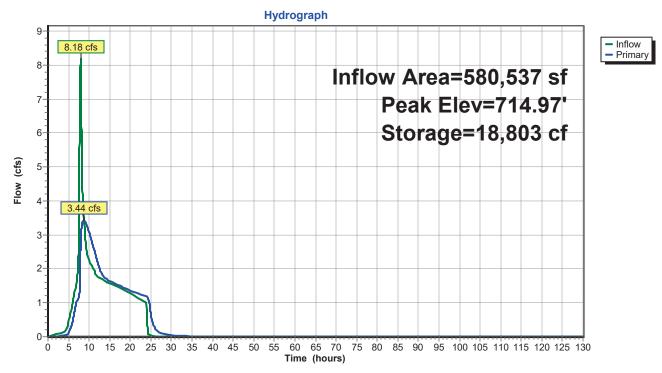
Volume	Inve	ert Avail.Sto	rage Storage	Description		
#1	713.0	0' 31,30	08 cf Custom	Stage Data (Pr	rismatic) Listed below (Recalc)	
Elevatio (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
713.0	00	7,916	0	0		
714.(00	9,540	8,728	8,728		
715.0	00	11,265	10,403	19,131		
716.0	00	13,090	12,178	31,308		
Device	Routing	Invert	Outlet Device	S		
#1	Primary	713.00'	15.0" Round	Culvert		
	y		Inlet / Outlet I		headwall, Ke= 0.500 / 712.80' S= 0.0045 '/' Cc= 0.900 f	
#2	Device 1	711.00'	7.0" Horiz. O	rifice/Grate C:	= 0.620 Limited to weir flow at low he	eads
#3	Device 1	713.95'	16.0" W x 3.0	" H Vert. Orifice	e/Grate C= 0.620	
Drimary		Max-3 11 ofe (@ 8.76 bre ∐\\	1-71/ 07' (Eroc	e Discharge)	

Primary OutFlow Max=3.44 cfs @ 8.76 hrs HW=714.97' (Free Discharge)

-1=Culvert (Passes 3.44 cfs of 5.95 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.87 cfs @ 6.98 fps)

-3=Orifice/Grate (Orifice Controls 1.57 cfs @ 4.71 fps)

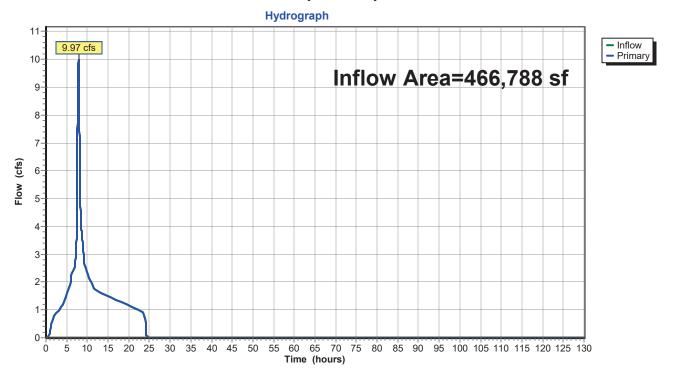


Pond Pond E: Pond E

Summary for Link Pipe 27: Pipe 27

Inflow Are	a =	466,788 sf,	72.69% Impervious,	Inflow Depth = 3.77"	for 25-Year event
Inflow	=	9.97 cfs @	7.89 hrs, Volume=	146,716 cf	
Primary	=	9.97 cfs @	7.89 hrs, Volume=	146,716 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-130.00 hrs, dt= 0.01 hrs



Link Pipe 27: Pipe 27

															-
		Project	Bull Pup	Filtratio	on Fac	ility Piped C	onvova	nco Anal	veic						
		<u> </u>	1/15/20			inty i ipeu c	Jonveya		y 515						
				24											
		Calc'd By:	Hankins												
											Pipe Inforr				
Pipe	Tributary Basins	Description	Q	Pipe	Pipe	Manning's	Slope	Slope	Area	Wetted	Hydrauli	Velocity	Flow	% Pipe	Velocity
Number			(Calc'd)	Dia.	Dia.	number	"S" %	"S"	Full	Perimeter	c Radius	Full	Rate	Capacity	@ Q/Qf
			"Q"	(inch)	(ft)	"n"			(Calc'd)	(Calc'd)	(Calc'd)	(Calc'd)	Full	Used	(Calc'd)
				"D"	"D"				"Af"	"WPf"	"Rf"	"Vf"	(Calc'd)	(Calc'd)	"V"
													"Of"	"0/0f"	
								Analysi							
1	1-6, 23	From Pond A (Detained)	2.09	18	1.50	0.013	0.95	0.0095		4.712	0.375	5.809	10.266	20.4%	1.18
2	1-6, 18, 23		2.27	18	1.50	0.013	0.39	0.0039	1.767	4.712	0.375	3.722	6.578	34.5%	1.28
3	23		0.88	12	1.00	0.013	0.50	0.0050	0.785	3.142	0.250	3.216	2.526	34.8%	1.12
4	1, 2		1.74	12	1.00	0.013	0.53	0.0053	0.785	3.142	0.250	3.311	2.601	66.9%	2.22
5	3		0.86	10	0.83	0.013	0.63	0.0063	0.545	2.618	0.208	3.197	1.744	49.3%	1.58
6	1-3, 23		3.48	18	1.50	0.013	0.26	0.0026		4.712	0.375	3.039	5.371	64.8%	1.97
7	1-4, 23		3.88	18	1.50	0.013	0.26	0.0026	1.767	4.712	0.375	3.039	5.371	72.2%	2.20
8	5		0.32	10	0.83	0.013	0.56	0.0056		2.618	0.208	3.014	1.644	19.5%	0.59
9	1-5, 23		4.20	18	1.50	0.013	0.26	0.0026	1.767	4.712	0.375	3.039	5.371	78.2%	2.38
10	7		0.76	10	0.83	0.013	0.56	0.0056	0.545	2.618	0.208	3.014	1.644	46.2%	1.39
11	8		2.81	15	1.25	0.013	0.51	0.0051	1.227	3.927	0.313	3.769	4.626	60.7%	2.29
12	8, 9	From Pond B (Detained)	2.22	18	1.50	0.013	0.49	0.0049	1.767	4.712	0.375	4.172	7.373	30.1%	1.26
13	Pump		0.80	12	1.00	0.013	0.44	0.0044	0.785	3.142	0.250	3.017	2.370	33.8%	1.02
14	10		9.05	15	1.25	0.013	10.70	0.1070	1.227	3.927	0.313	17.265	21.187	42.7%	7.37
15	10, 11	From Pond C (Detained)	3.45	18	1.50	0.013	0.51	0.0051	1.767	4.712	0.375	4.256	7.522	45.9%	1.95
16	7, 12		1.86	10	0.83	0.013	1.00	0.0100	0.545	2.618	0.208	4.028	2.197	84.7%	3.41
17	13		2.46	12	1.00	0.013	0.50	0.0050	0.785	3.142	0.250	3.216	2.526	97.4%	3.13
18	7, 12-14	From Pond D (Detained)	2.79	12	1.00	0.013	1.69	0.0169	0.785	3.142	0.250	5.913	4.644	60.1%	3.55
19	1-7, 12-15, 18, 23		5.11	18	1.50	0.013	0.50	0.0050	1.767	4.712	0.375	4.215	7.448	68.6%	2.89
20	15		1.42	12	1.00	0.013	0.52	0.0052	0.785	3.142	0.250	3.280	2.576	55.1%	1.81
21	7, 12-15	From Flow Splitter	0.66	8	0.67	0.013	0.76	0.0076	0.349	2.094	0.167	3.026	1.056	62.5%	1.89
22	7, 12-15	From Flow Splitter	3.18	18	1.50	0.013	0.51	0.0051	1.767	4.712	0.375	4.256	7.522	42.3%	1.80
23	16		1.54	10	0.83	0.013	2.20	0.0220	0.545	2.618	0.208	5.974	3.259	47.3%	2.82
24	1-7, 12-16, 18, 23		6.49	18	1.50	0.013	0.50	0.0050	1.767	4.712	0.375	4.215	7.448	87.1%	3.67
25	8-11, 22		5.83	18	1.50	0.013	15.00	0.1500	1.767	4.712	0.375	23.084	40.793	14.3%	3.30
26	20, 24	From Pond E (Detained)	3.44	15	1.25	0.013	0.45	0.0045	1.227	3.927	0.313	3.541	4.345	79.2%	2.80
27	17, 21	From Pump Station	4.00	15	1.25	0.013	2.00	0.0200	1.227	3.927	0.313	7.464	9.160	43.7%	3.26
28	17, 19, 21		4.50	15	1.25	0.013	7.88	0.0788	1.227	3.927	0.313	14.816	18.182	24.7%	3.67
29	1-7, 12-19, 21, 23		10.99	24	2.00	0.013	0.41	0.0041	3.142	6.283	0.500	4.623	14.524	75.7%	3.50
30	1-19, 21-23		16.82	30	2.50	0.013	0.41	0.0041	4.909	7.854	0.625	5.365	26.334	63.9%	3.43
31	24		3.65	12	1.00	0.013	1.30	0.0130	0.785	3.142	0.250	5.186	4.073	89.6%	4.65
EX1	7, 12-15	Existing 8" Culvert	0.66	8	0.67	0.013	10.11	0.1011	0.349	2.094	0.167	11.037	3.853	17.1%	1.89

Attachment H: Outfall Flow Spreader Calculations and Details

Project Description		
Solve For	Crest Elevation	
Input Data		
Discharge	16.82 cfs	
Headwater Elevation	100.00 ft	
Tailwater Elevation	99.90 ft	
Crest Surface Type	Paved	
Crest Breadth	1.00 ft	
Crest Length	175.0 ft	
Results		
Crest Elevation	99.90 ft	
Headwater Height Above Crest	0.10 ft	
Tailwater Height Above Crest	0.00 ft	
Weir Coefficient	2.93 ft^(1/2)/s	
Submergence Factor	1.000	
Adjusted Weir Coefficient	2.93 ft^(1/2)/s	
Flow Area	17.9 ft ²	
Velocity	0.94 ft/s	
Wetted Perimeter	175.2 ft	
Top Width	175.00 ft	

Attachment H - Flow Spreader

Attachment I: Stormwater Pond Cross Sections, Basins, Planters, Grassy Swale and Flow Control Manholes

See Land Use drawings under separate cover.